



**LYCÉE LOUIS  
MASSIGNON**  
ليسيه لوي مسينيون

ÉTABLISSEMENT  
EN GESTION DIRECTE



**aefe**  
Agence pour  
l'enseignement français  
à l'étranger

# **RENOVATION OF BLOCK B' FOR FRENCH SCHOOL (LYCEE LOUIS MASSIGNON)**

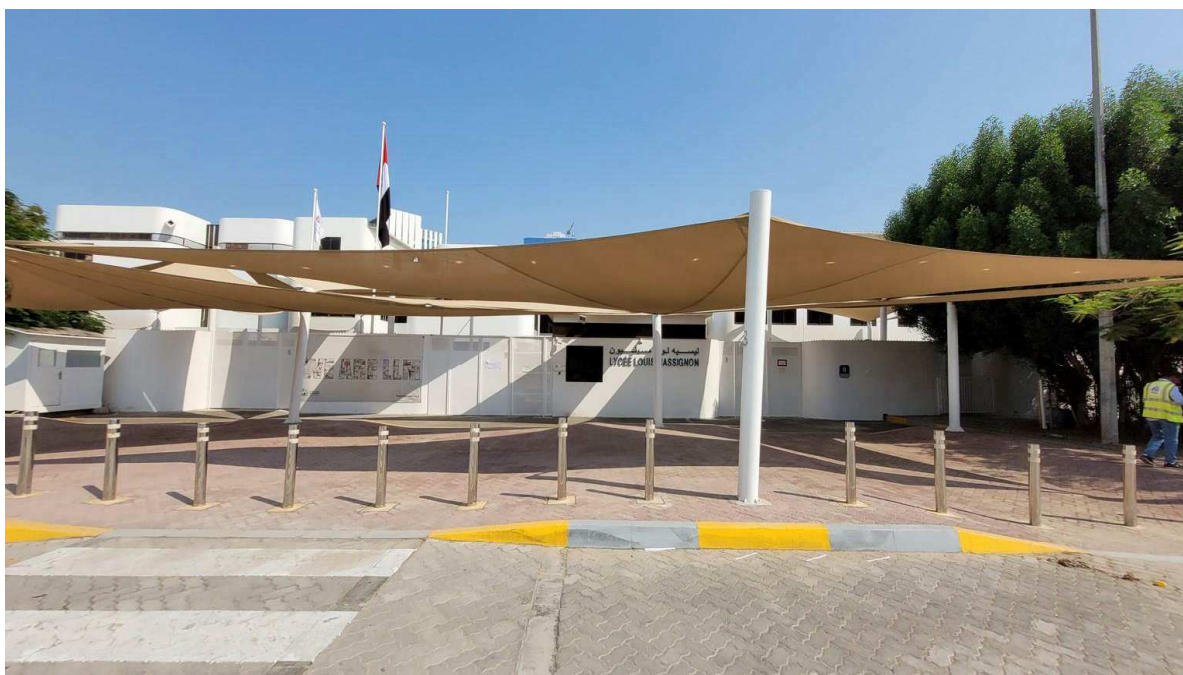
## **CONCRETE REPAIRS AND STRUCTURAL REINFORCEMENT**

**PLOT (26\_27), SECTOR (E40)  
ABU DHABI ISLAND  
EMIRATE OF ABU DHABI  
UNITED ARAB EMIRATES**

**VOLUME 5 – Part 6 of 7  
M/s eForce Inspection Consultancies  
Report eForce/1104/24-R1 dated January 07<sup>th</sup>, 2025**

**HP Project No. 1782**

**Revision 1  
April 18<sup>th</sup>, 2025**



**Existing Lycee Louis Massignon School, Al Sa'ada Street, Abu Dhabi, UAE - Structural Testing Works for [Gymnasium Building & Auditorium Building]- Structural Assessment & Structural Strengthening Design**

<b>Submitted to</b>	<b>Al Hilal Engineering Consultants</b>
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**07<sup>th</sup> Jan 2025 -R1**



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### Preliminary note

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## 1 Structural Analysis

### 1.1 Design Brief

eFORCE has carried out the design check of the existing building using the compressive strength obtained from core testing.

### 1.2 Design Standards

Design check was carried out to meet the requirements of the following standards and design guidelines:

ACI 318-14	Building Code Requirements for Structural Concrete and Commentary
ASCE 7-05	Minimum Design Loads for buildings and other Structures
IEBC 2018	International Existing Building Code

### 1.3 Material Properties

#### I. Concrete Compressive Strength

Floor Level	Columns $f'_c$	Slabs/Beams $f'_c$
Ground	19.6	16.6
Roof	19.6	16.6

#### II. Reinforcement Steel Yield Strength

$F_y = 420$  MPa for flexure bars

## 2 Design Loads

### 2.1.1 Design loads

The below loads were considered in the review of the structural elements analysis and checking.

#### I. Gravity Loads, refer to the below table

Location	SDL	LL
Roof (Inaccessible)	3.0 kPa	1.0 kPa
Roof (Accessible)	3.0 kPa	1.5 kPa
Block Works (200mm thk.)	10.0 kPa	-
Ribbed and Waffle slab filler	1.0 kPa	-

Self-weight is automatically calculated by the design software.  
Refer to below screen shots from ETABS for Loads allocation.

#### II. Wind Load

Automatically computed by ETABS software according to ASCE 7-05.  
Basic Wind Speed = 100 mph  
Exposure Category = C  
Importance Factor = 1.0  
Gust Factor = 0.85  
 $e_1, e_2 = 0.15$

### III. LOADS RUN DOWN GYMNASIUM

At this point, the investigations to determine the mechanical characteristics of the crossbeams are not planned yet, the technical solution for reinforcing the beams has not been determined and validated.

In order to take into account, the additional load due to the reinforcement, the dead weight is increased by 30%. However, in a conservative posture, this coefficient does not have to be applied in combination with a negative pressure wind.

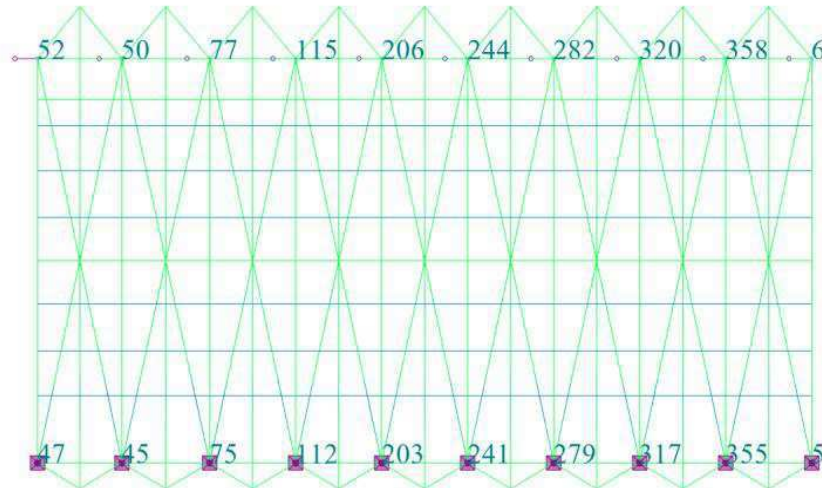


Figure 2-1 reactions from Robot structural refer to **Appendix-A**

### IV. LOADS RUN DOWN AUDITORIUM

At this point, the investigations to determine the mechanical characteristics of the crossbeams are not planned yet, the technical solution for reinforcing the beams has not been determined and validated.

In order to take into account, the additional load due to the reinforcement, the dead weight is increased by 30%. However, in a conservative posture, this coefficient does not have to be applied in combination with a negative pressure wind.

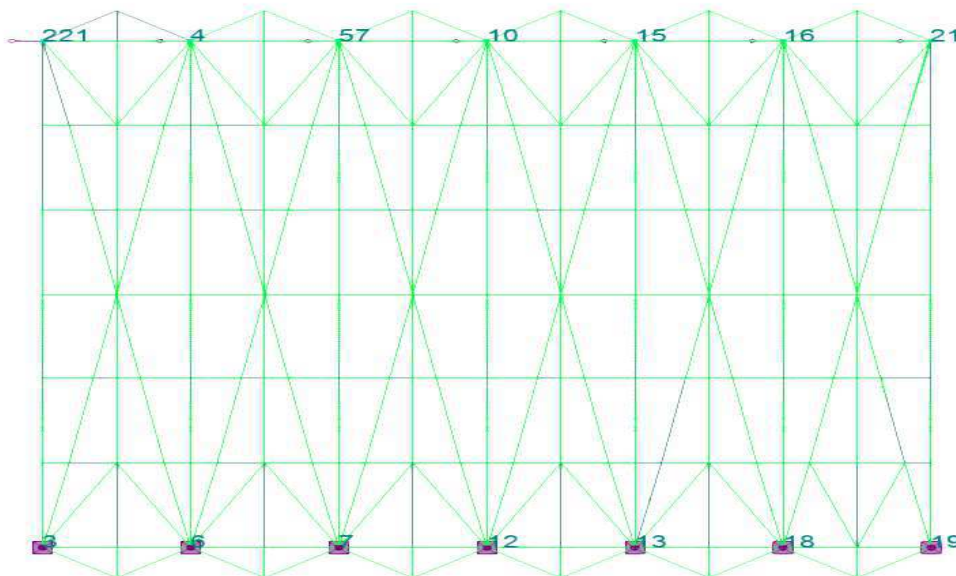


Figure 2-2 reactions from Robot structural refer to **Appendix-A**

### 3 Design Load Combination

#### 3.1.1 Load combination for Vertical Elements

The structure shall be checked for limit state load combination in accordance with ASCE 7-05

- I.  $1.4 D$
- II.  $1.2 D + 1.6 L_n + 1.6 L_h + 1.6 L_p$
- III.  $1.2 D + 0.5 L_n + 1.0 L_h + 1.0 L_p \pm 1.6 W$
- IV.  $1.2 D + 0.5 L_n + 1.0 L_h + 1.0 L_p \pm 1.6 W$
- V.  $0.9 D \pm 1.6 W$
- VI.  $0.9 D \pm 1.6 W$

#### 3.1.2 Load Combinations for Slab Design

The following load combinations will be used in the design of the PT slab

- I. Serviceability Limit state  
 $1.0 D + 1.0 L_n + 1.0 L_h + 1.0 L_p$
- II. Ultimate Limit State  
 $1.2 D + 1.6 L_n + 1.6 L_h + 1.6 L_p$
- III. Crack-section analysis  
Immediate All =  $1.0 D + 1.0 L_n + 1.0 L_h + 1.0 L_p$   
Immediate Sustained =  $1.0 D + 0.25 L_n + 0.25 L_h + 1.0 L_p$   
Long Term Sustained =  $1.0 D + 0.25 L_n + 0.25 L_h + 1.0 L_p$
- IV. Total Long-Term Deflection:  
Immediate All – Immediate Sustained + Long Term Sustained

### 4 Design Software

The software ETABS 2016 is used for the analysis of the structural frame of the building and the design of vertical elements and beams.

The software SAFE 2016 is being used for the design check of the slab and beams.

### 5 Analysis and Design Procedure

#### 5.1 ETABS Model

The structure is modeled using structural programs ETABS for analysis using a Finite Element Method in order to study the stability, deformation and forces acting on each member of the structure due to vertical and wind loads.

In general, the slab and beams will transmit the loads to the vertical support going down to the foundation.



In ETABS analysis, the lateral stability shall be satisfied through RCC Shear Walls, Columns and Beams with sufficient stiffness that can satisfy all the requirements in strength.

For the ultimate strength analysis and design, the following stiffness modification factor for each element shall be applied considering the crack section.

Solid Slab	-	0.25
Rib/Waffle Slab	-	0.25
Beams	-	0.35
Columns	-	0.70
Walls	-	0.70

### 5.1.1 GYMNASIUM PART

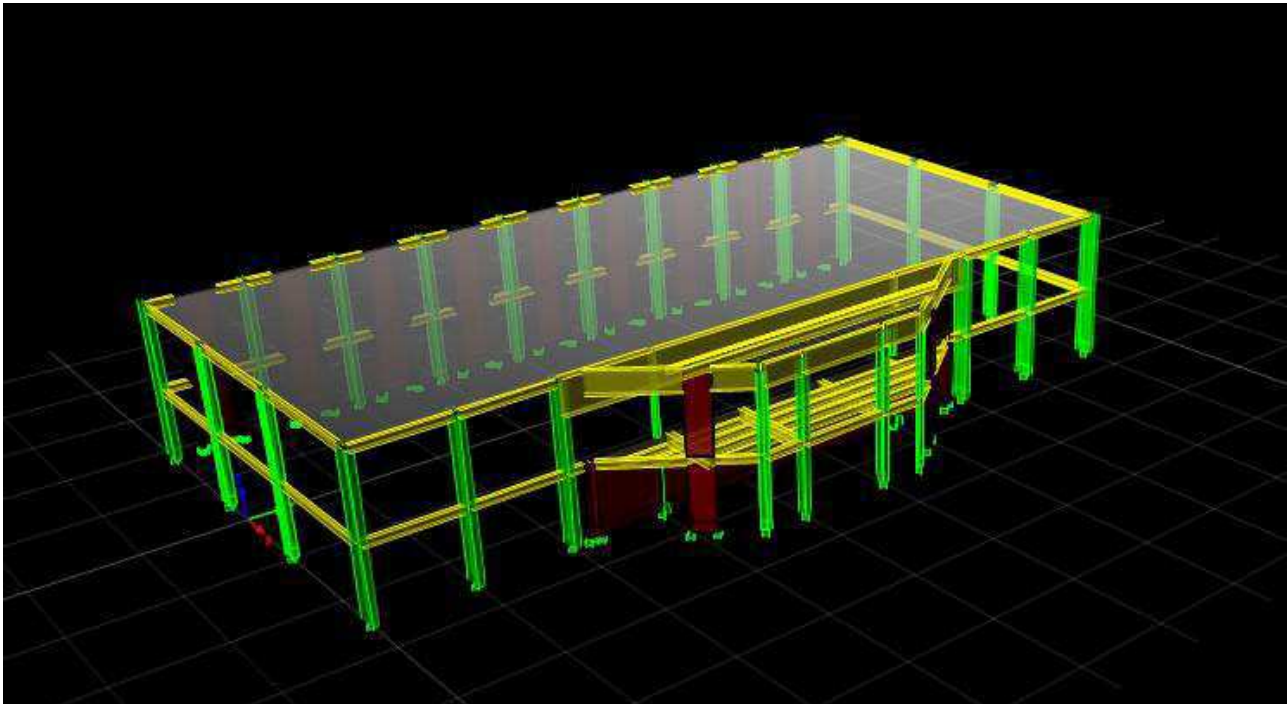


Figure 5-1 ETABS 3D Model for gymnasium part

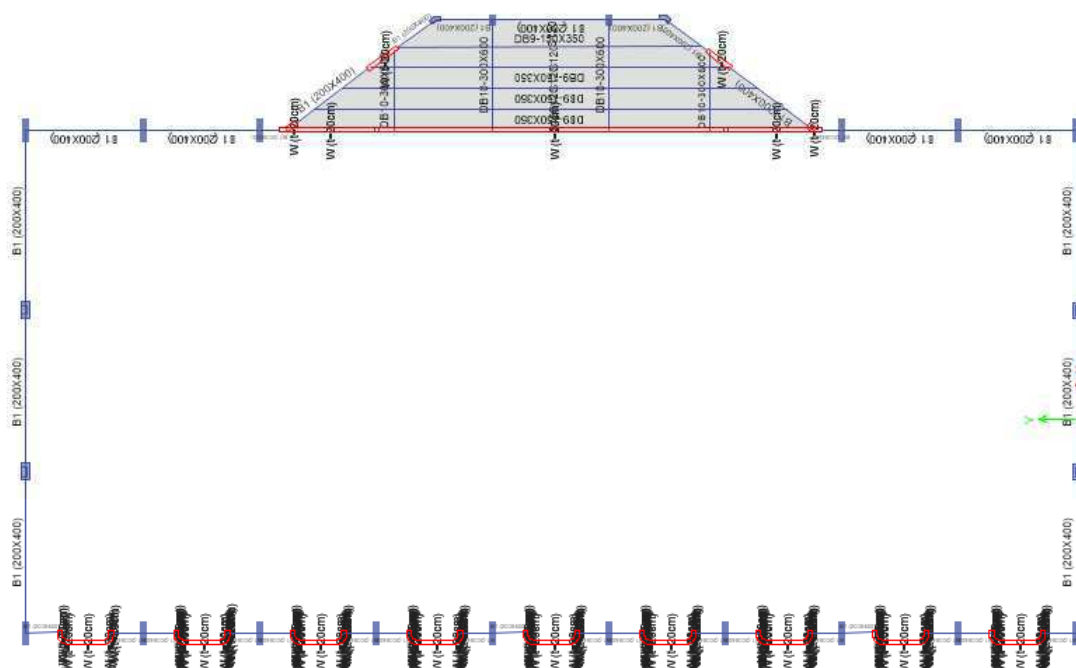


Figure 5-2 Walls, Slabs and Beams Section Assignment at ground Floor Level

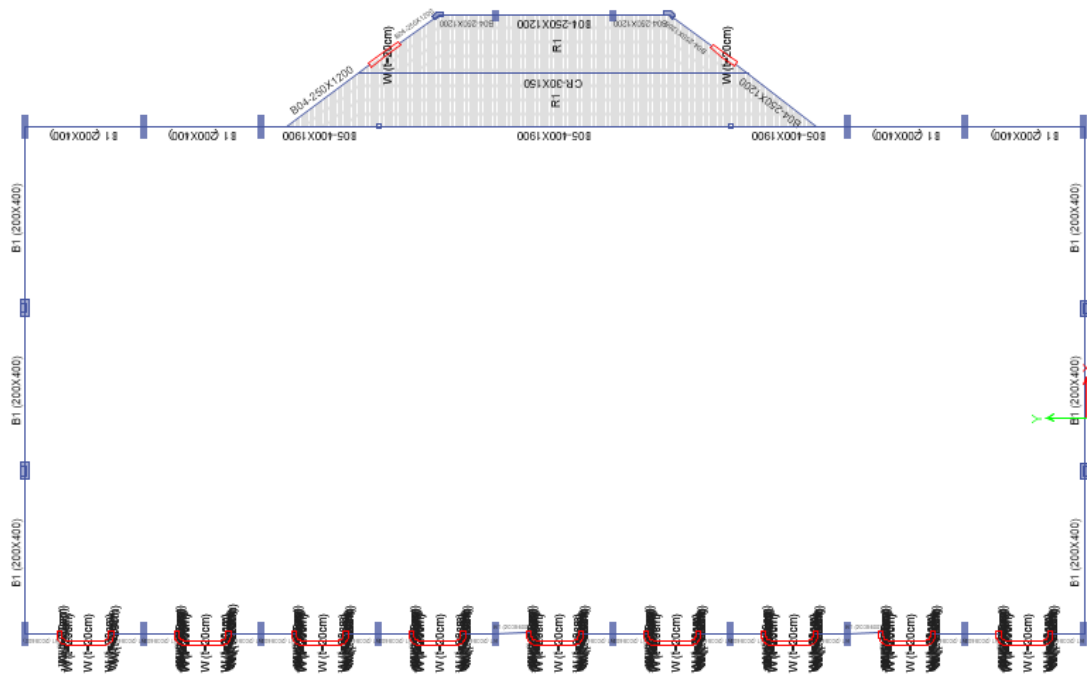


Figure 5-3 Walls, Slabs and Beams Section Assignment at Roof Floor Level

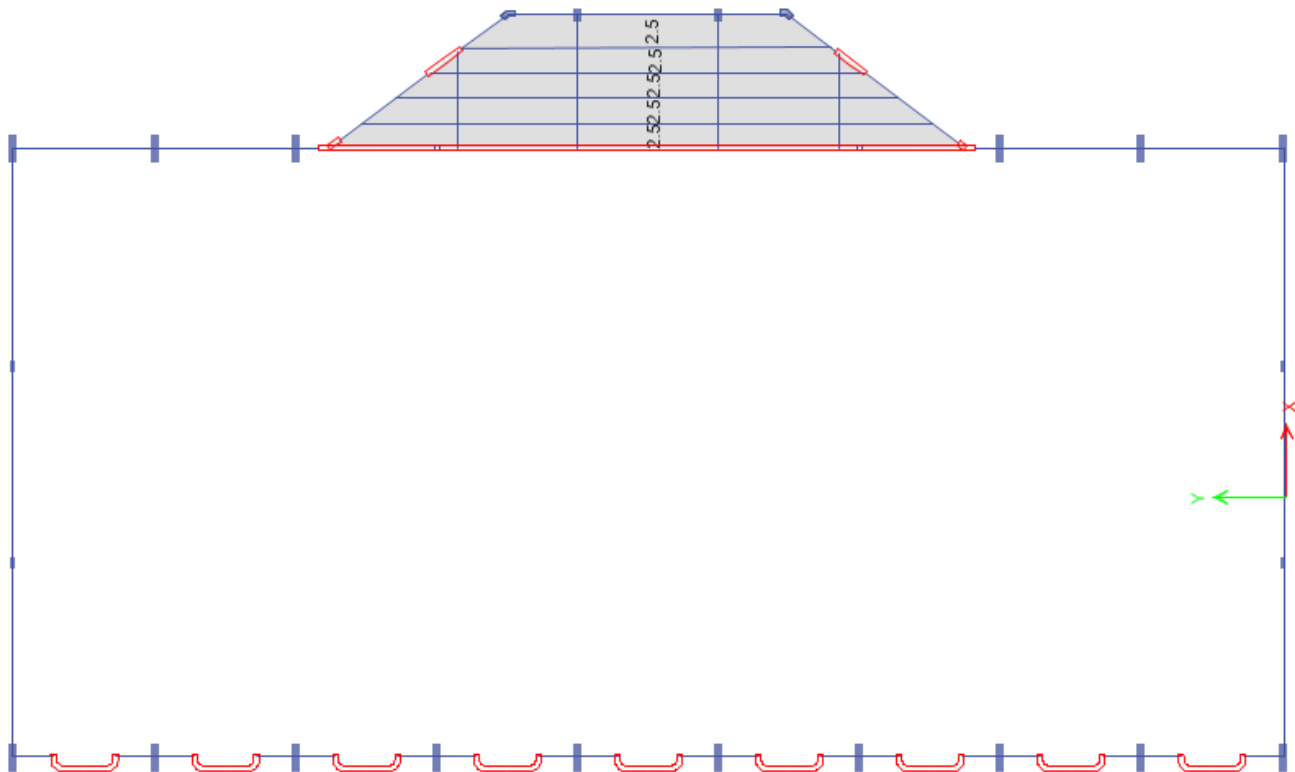


Figure 5-4 Super Imposed Dead Load at GROUND Floor Slab (SID)

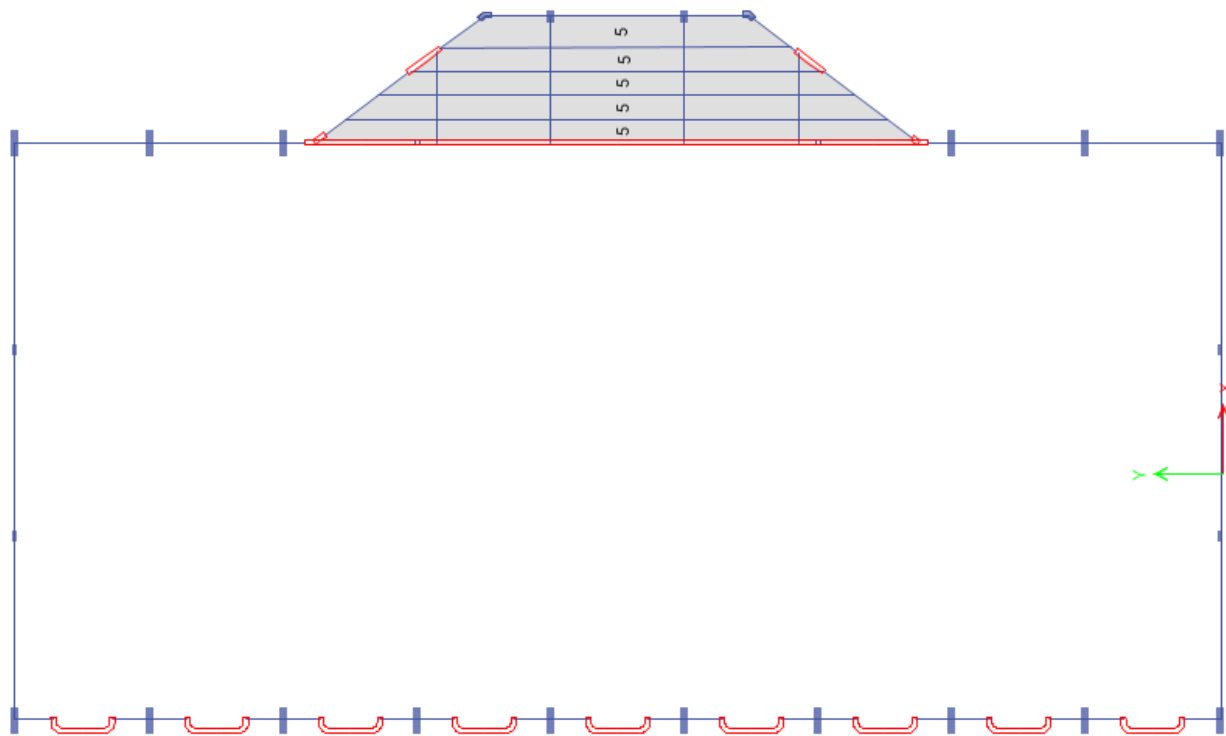


Figure 5-5 Live Load Dead Load at GROUND Floor Slab (LL)

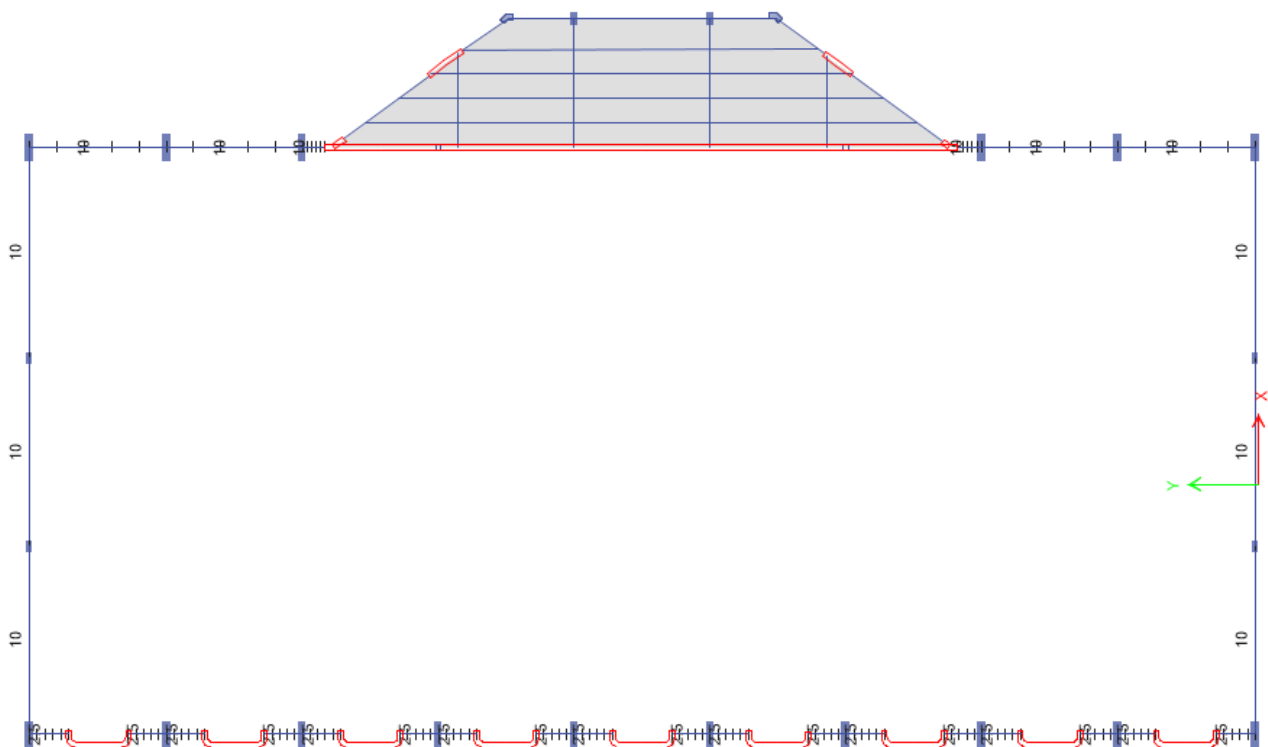


Figure 5-6 Blockworks Load at GROUND Floor Slab (Block)

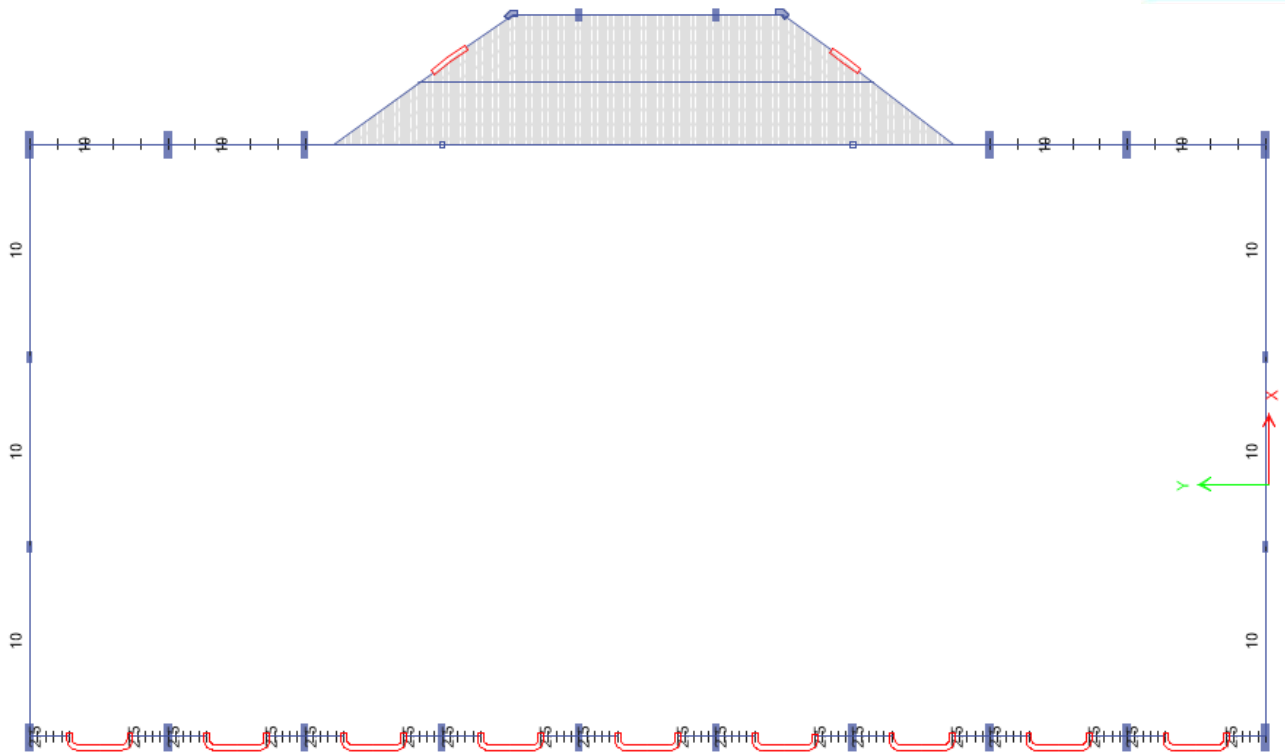


Figure 5-7 Blockworks Load at ROOF Floor Slab (Block)

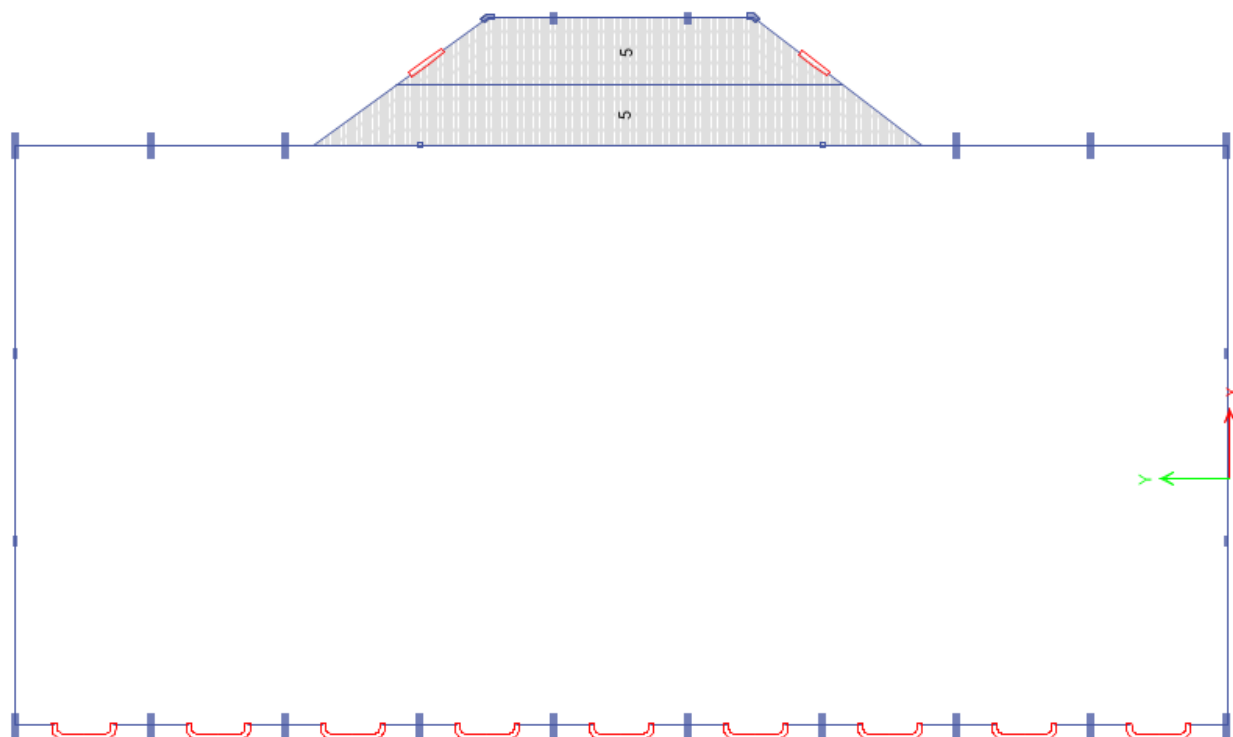


Figure 5-8 Super Imposed Load at Roof Floor Slab (SID)



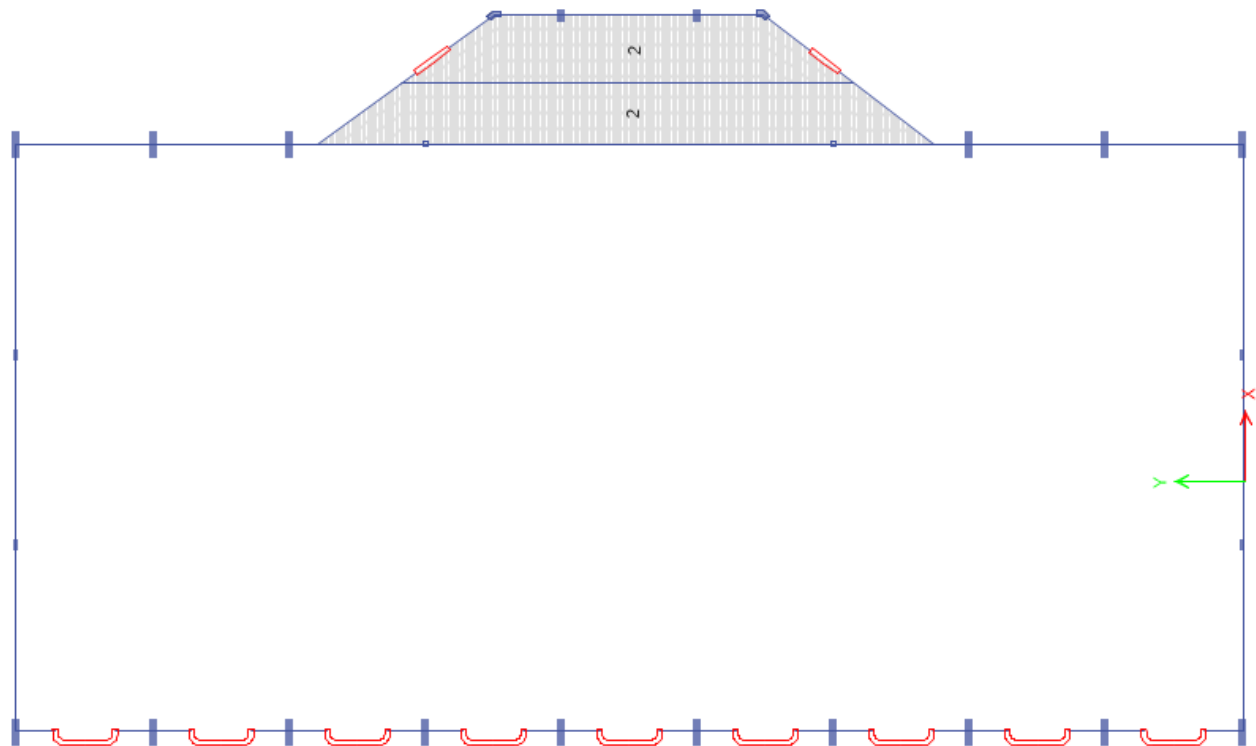


Figure 5-9 Live Load at Roof Floor Slab (LL Roof)

### 5.1.2 AUDITORIUM PART

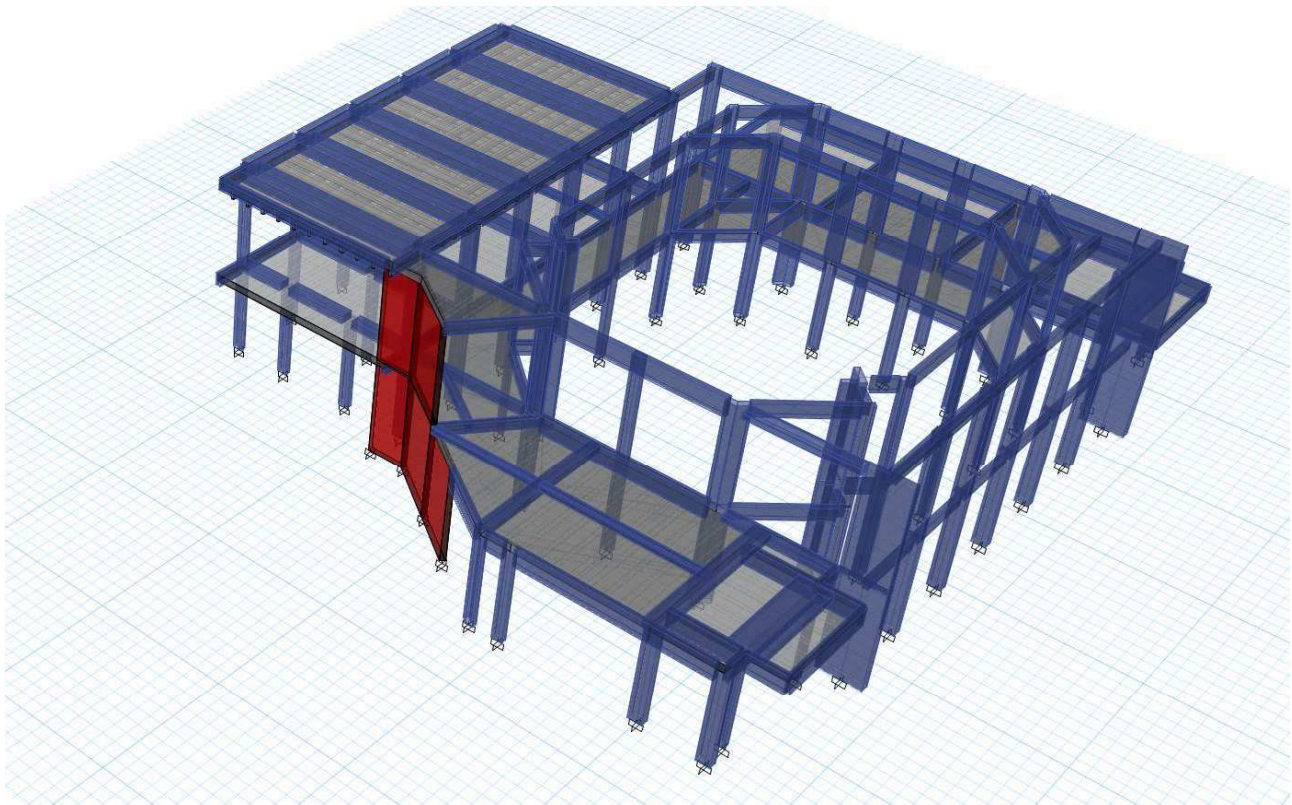


Figure 5-10 ETABS 3D Model for Auditorium part

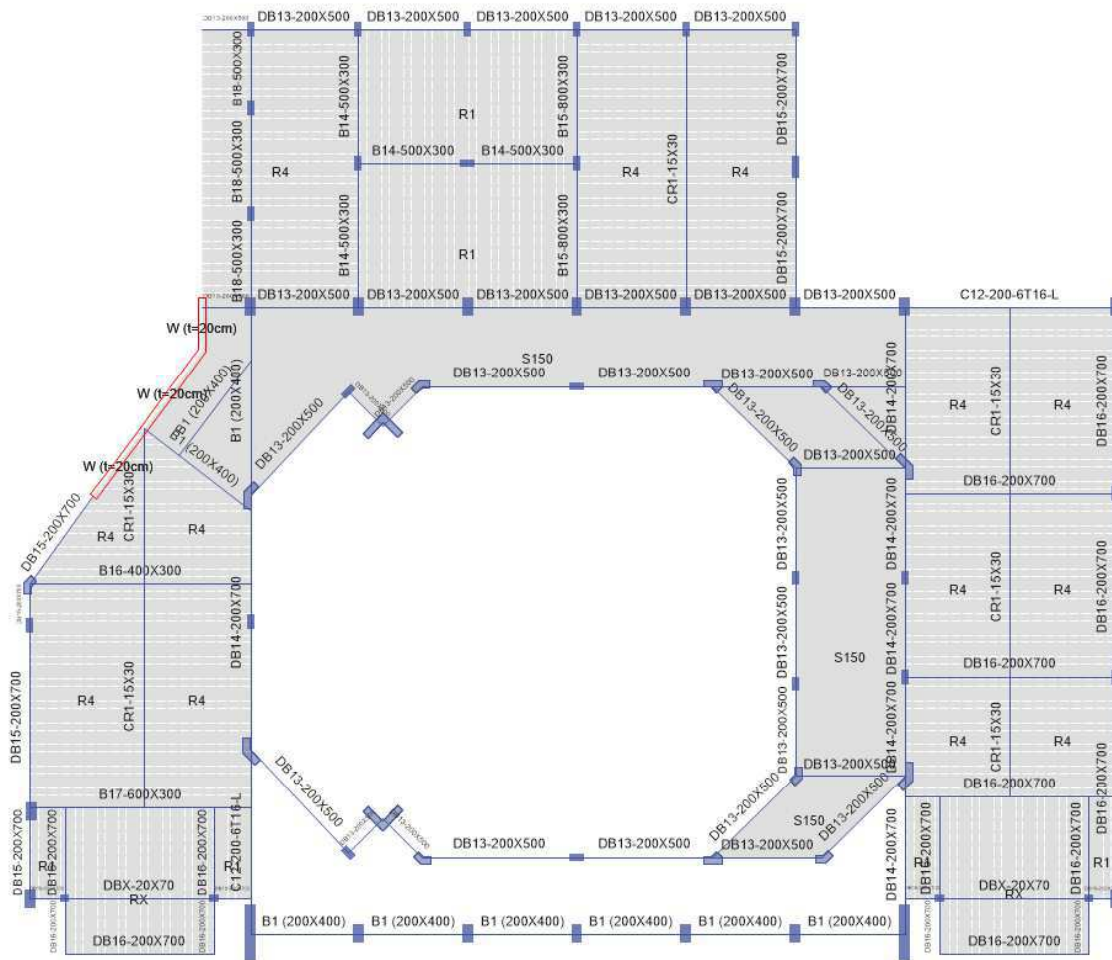


Figure 5-11 Walls, Slabs and Beams Section Assignment at ground Floor Level

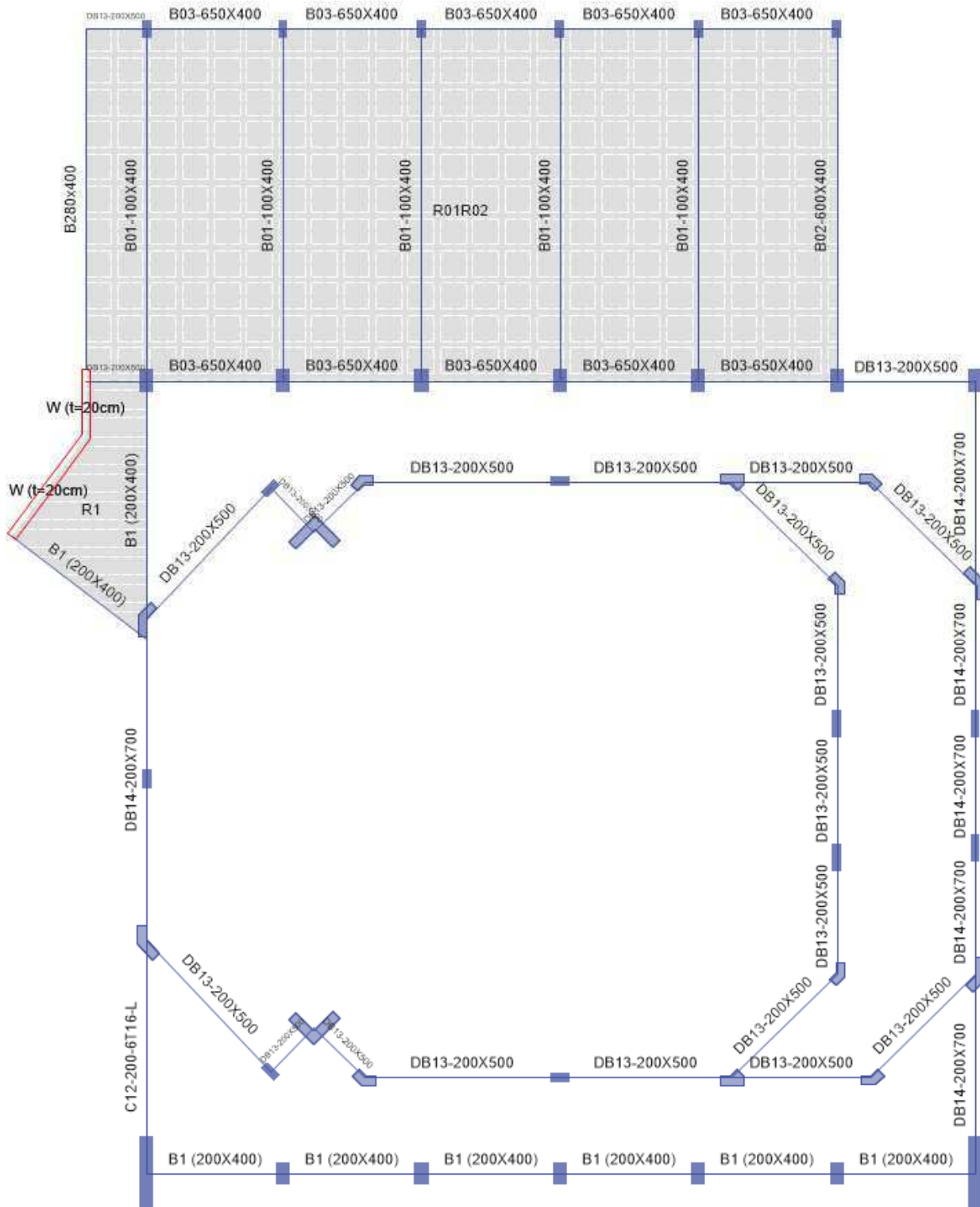


Figure 5-12 Walls, Slabs and Beams Section Assignment at Roof Floor Level

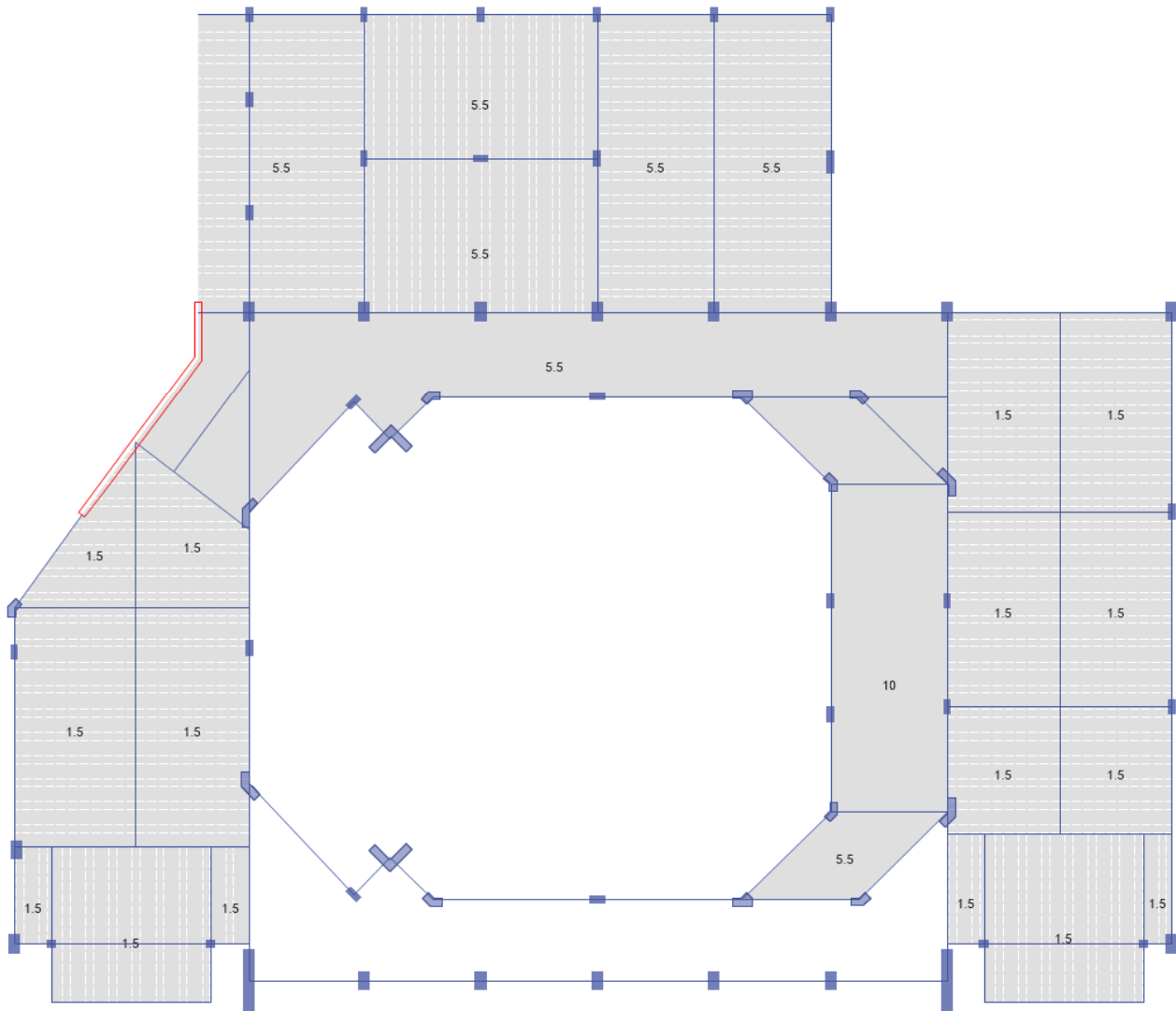


Figure 5-13 Super Imposed Dead Load at GROUND Floor Slab (SID)

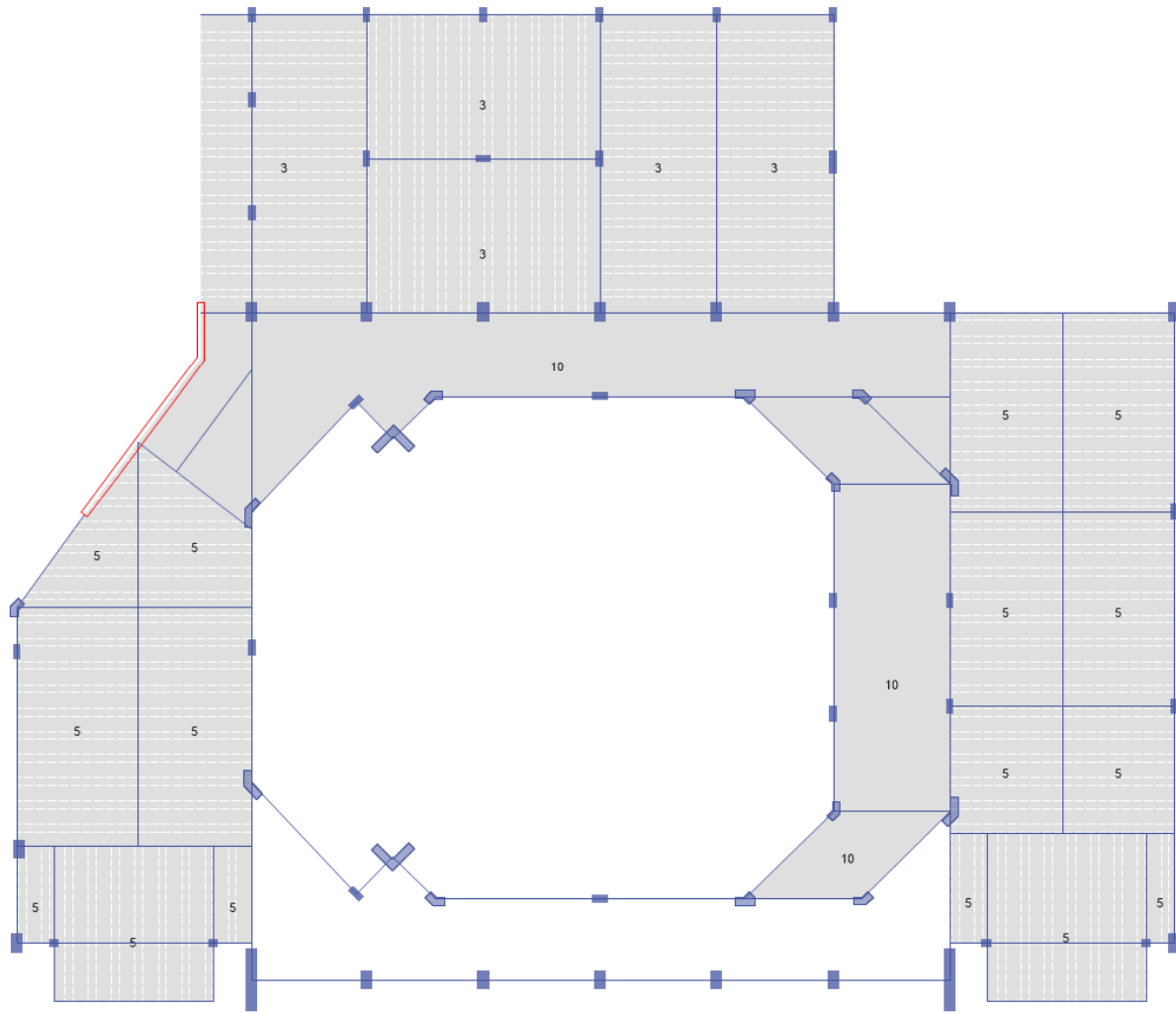


Figure 5-14 Live Load Dead Load at GROUND Floor Slab (LL)



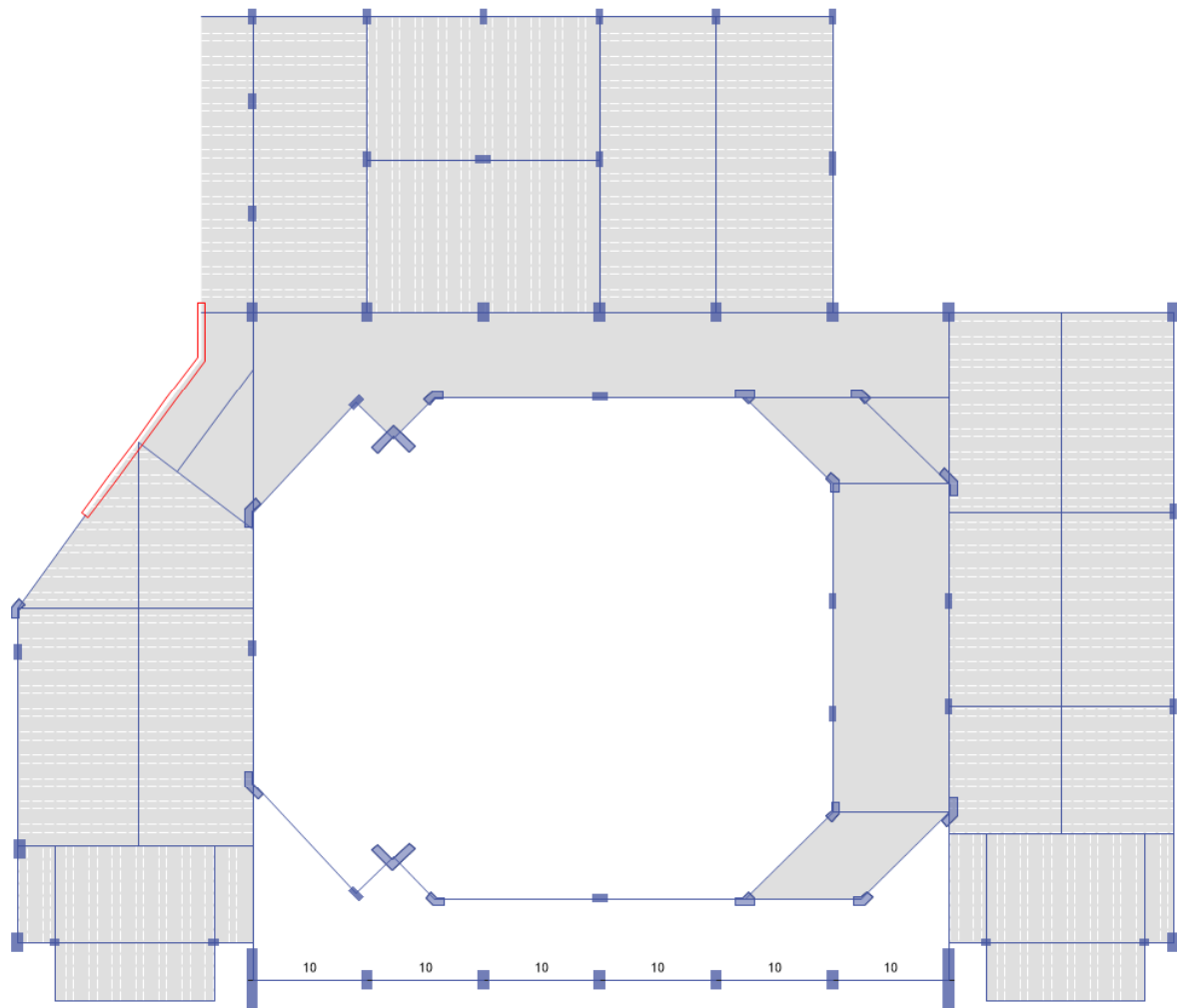


Figure 5-15 Blockworks Load at GROUND Floor Slab (Block)

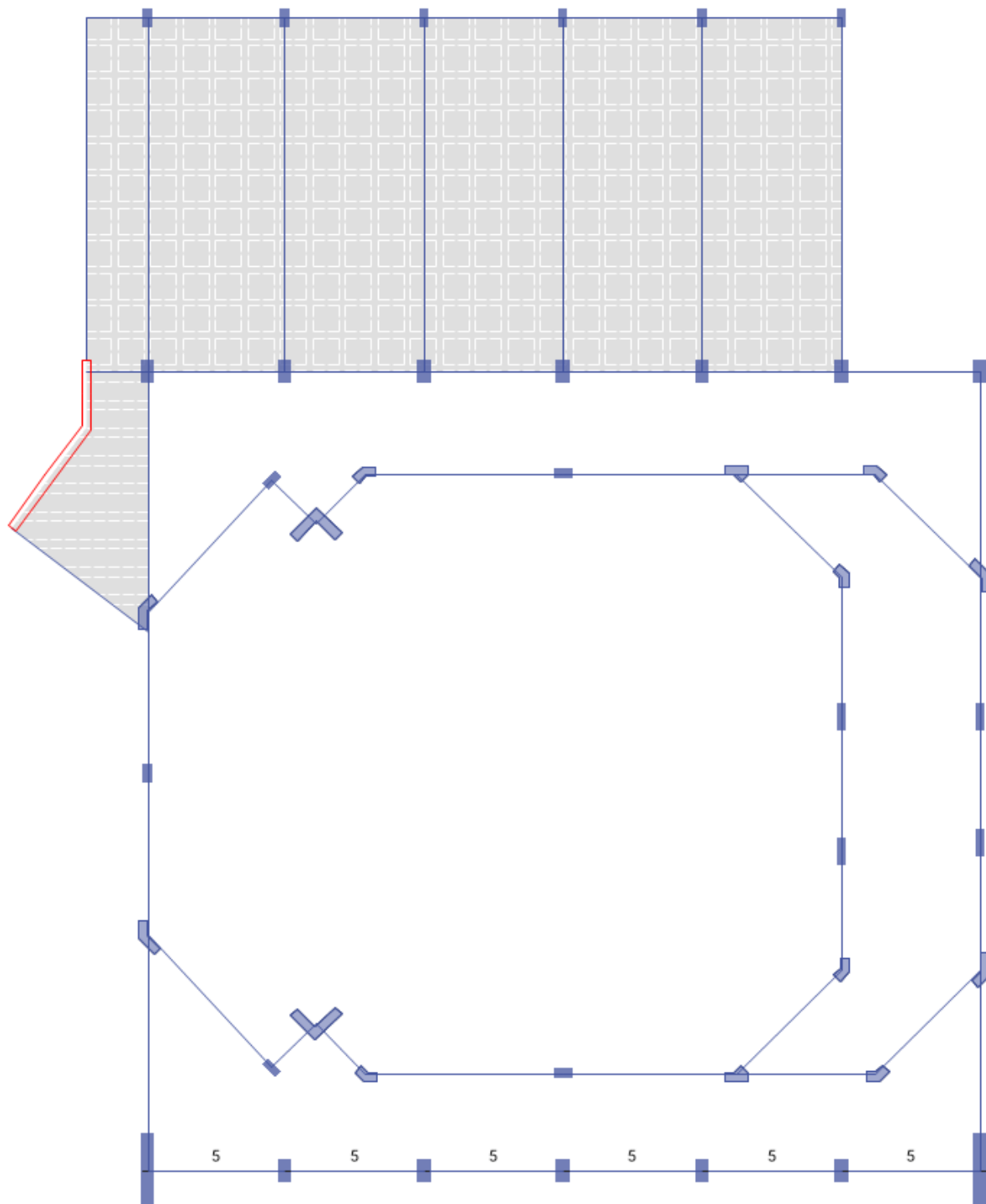


Figure 5-16 Blockworks Load at ROOF Floor Slab (Block)

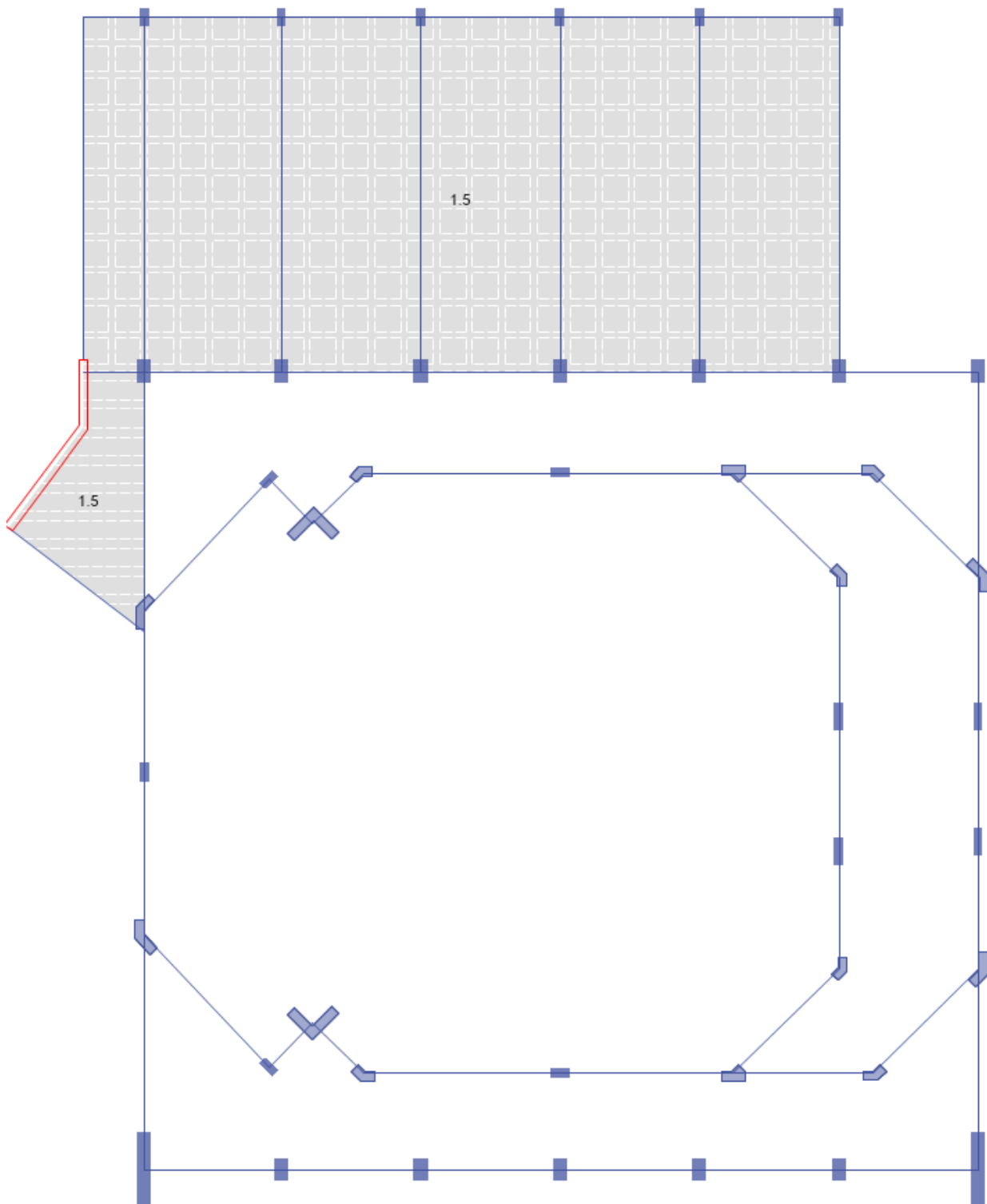


Figure 5-17 Super Imposed Load at Roof Floor Slab (SID)

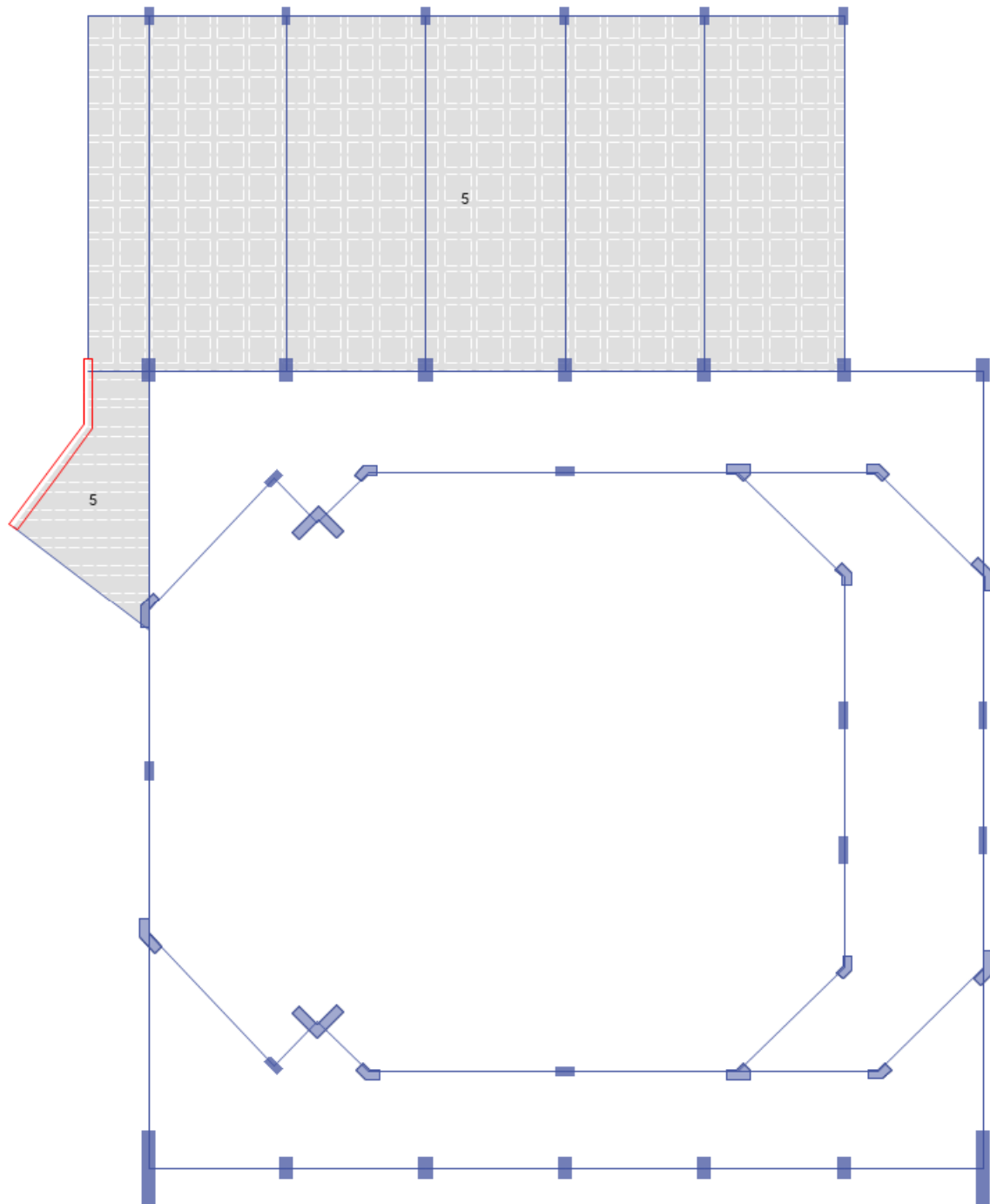


Figure 5-18 Live Load at Roof Floor Slab (LL Roof)

### 5.1.3 SWIMMING PART

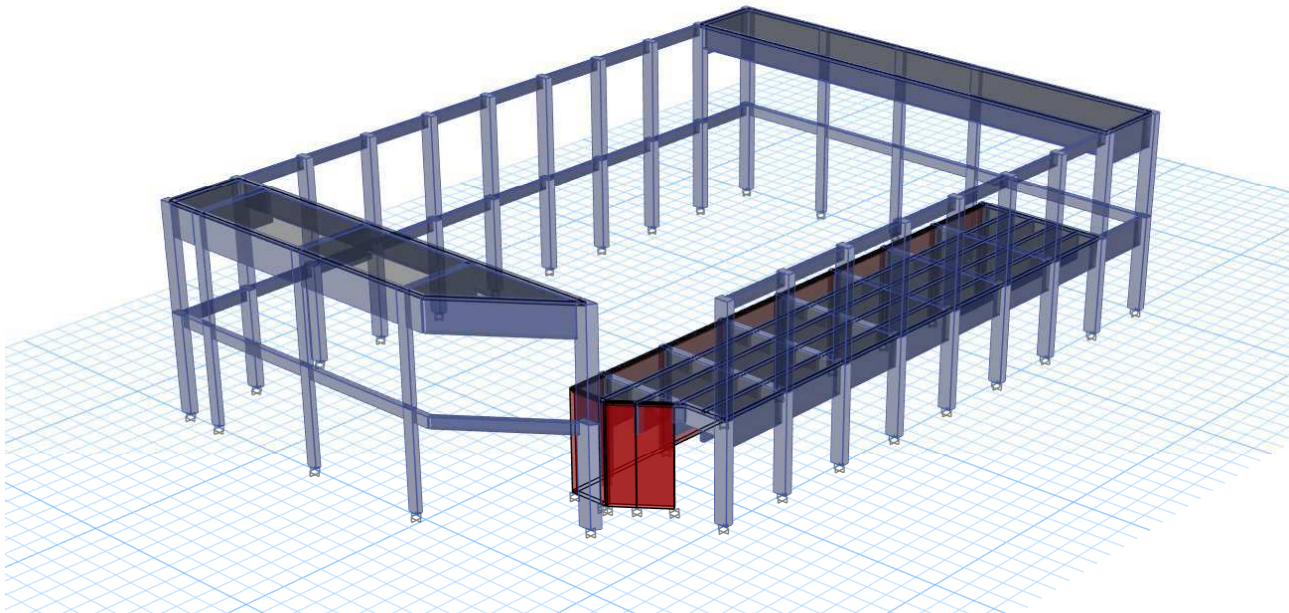


Figure 5-19 ETABS 3D Model for swimming part

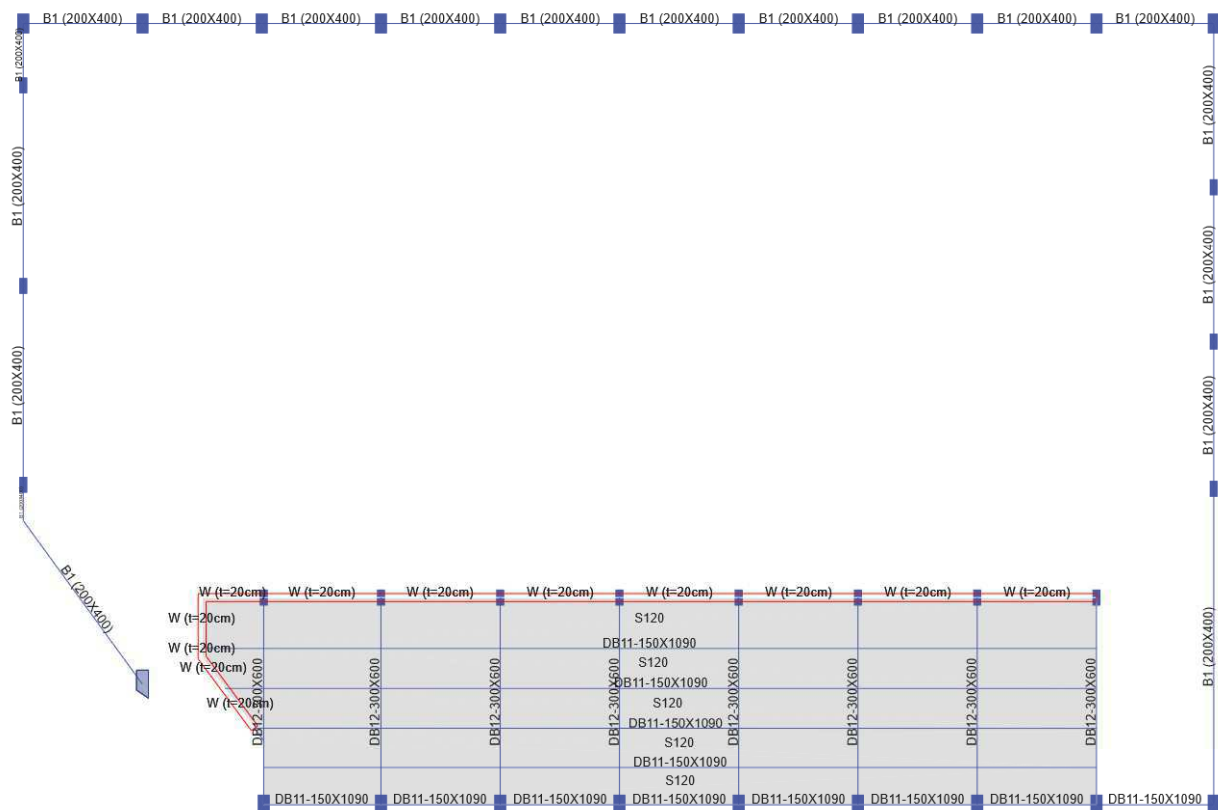


Figure 5-20 Walls, Slabs and Beams Section Assignment at ground Floor Level



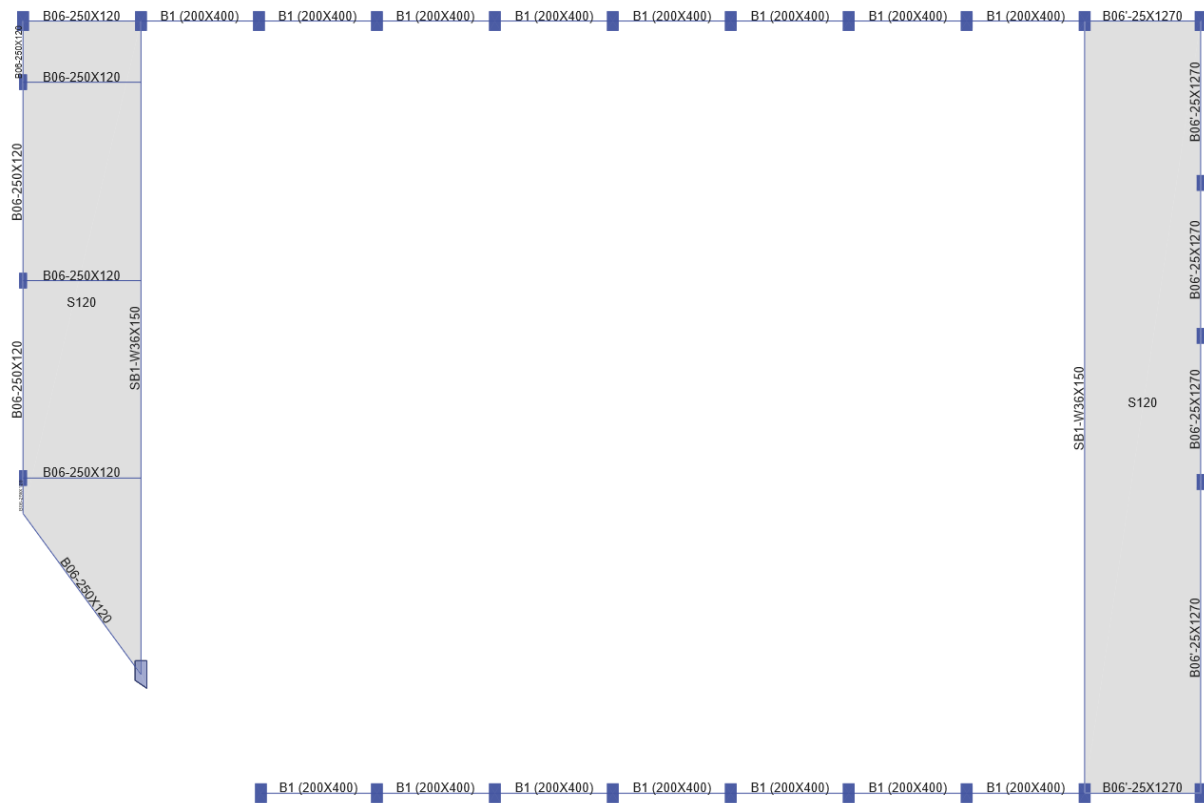


Figure 5-21 Walls, Slabs and Beams Section Assignment at Roof Floor Level

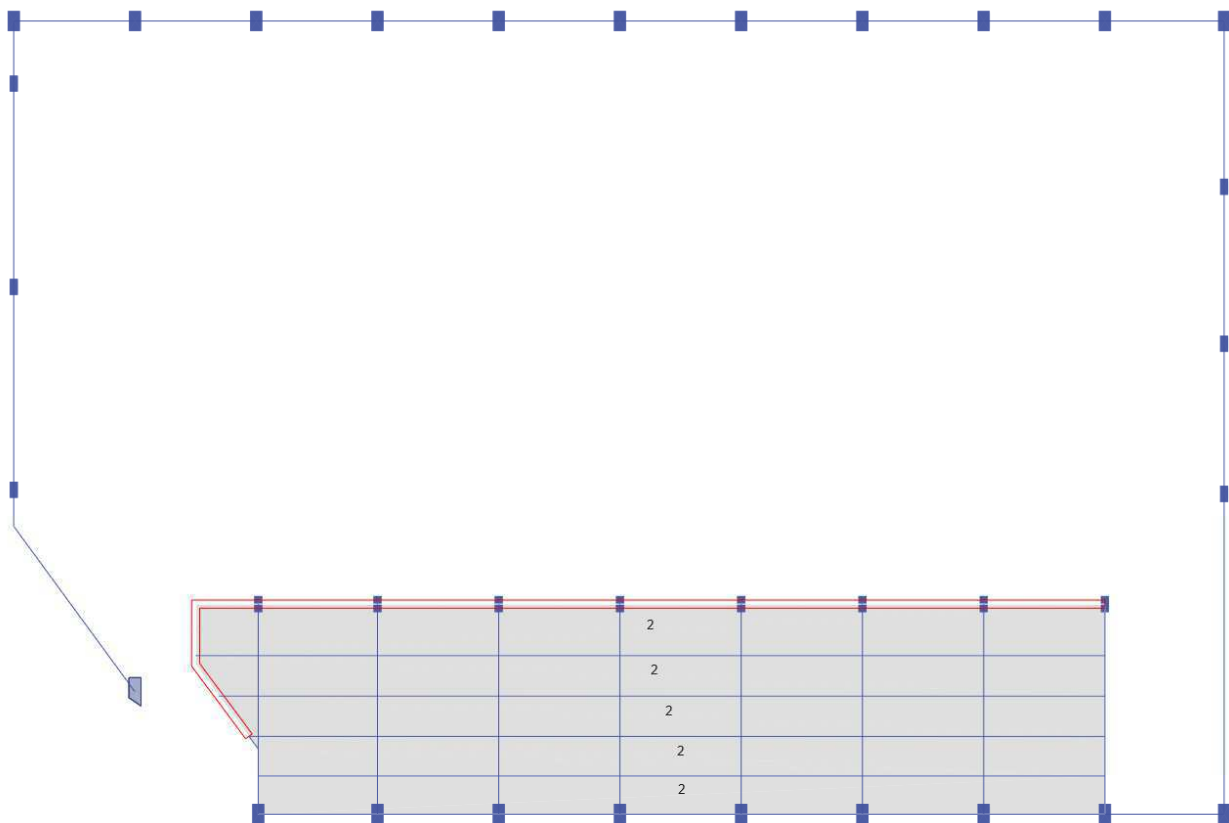


Figure 5-22 Super Imposed Dead Load at GROUND Floor Slab (SID)

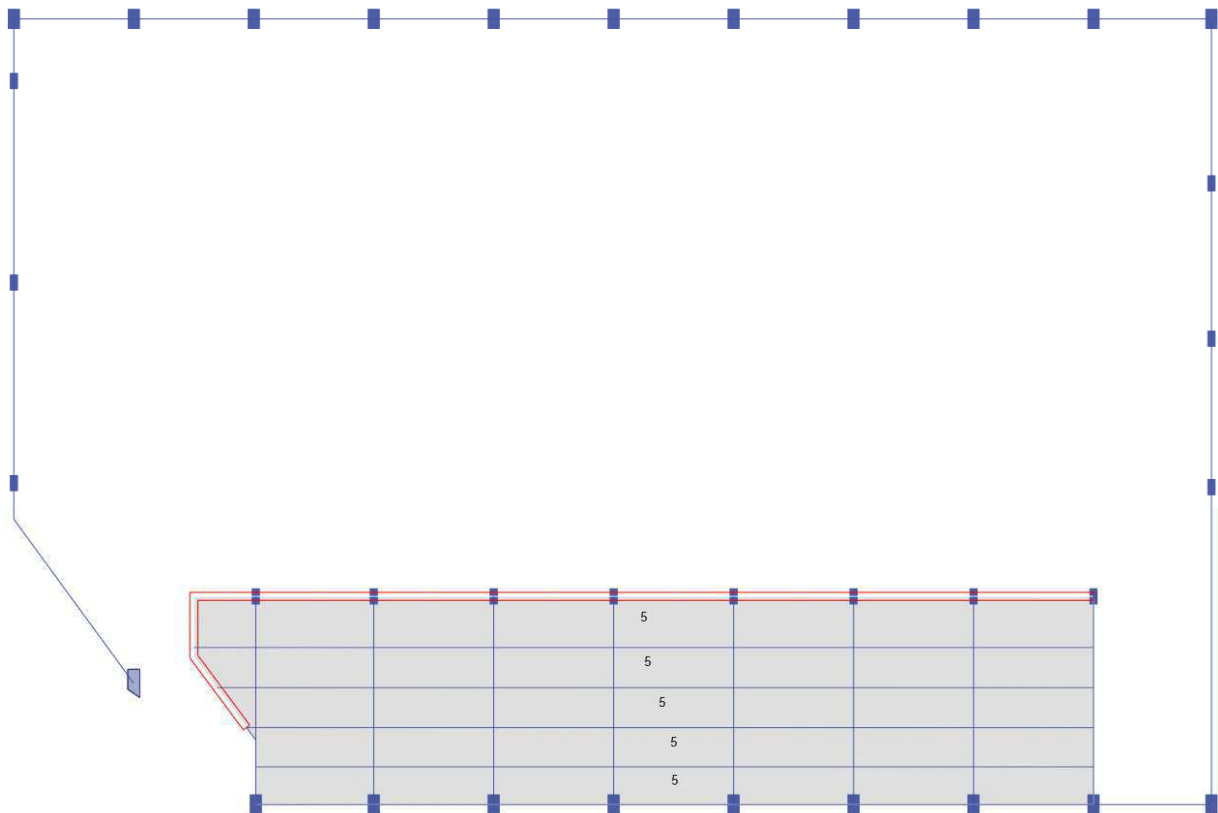


Figure 5-23 Live Load Dead Load at GROUND Floor Slab (LL)

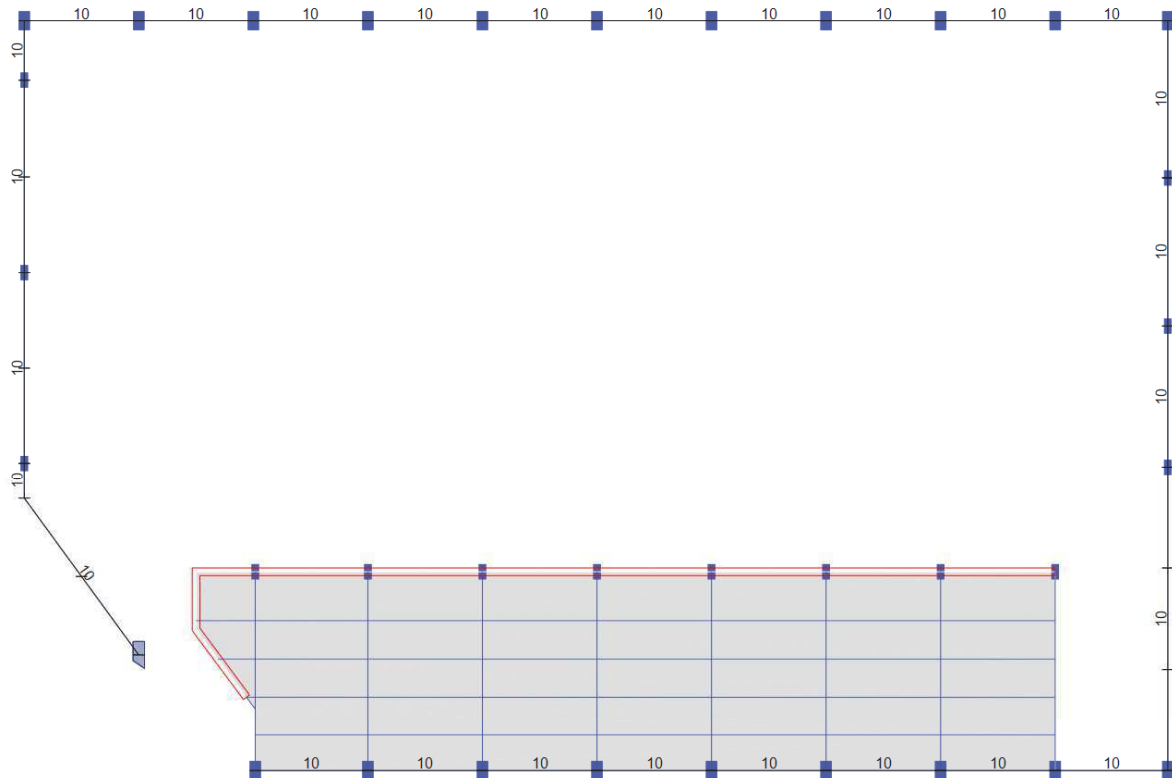


Figure 5-24 Blockworks Load at GROUND Floor Slab (Block)

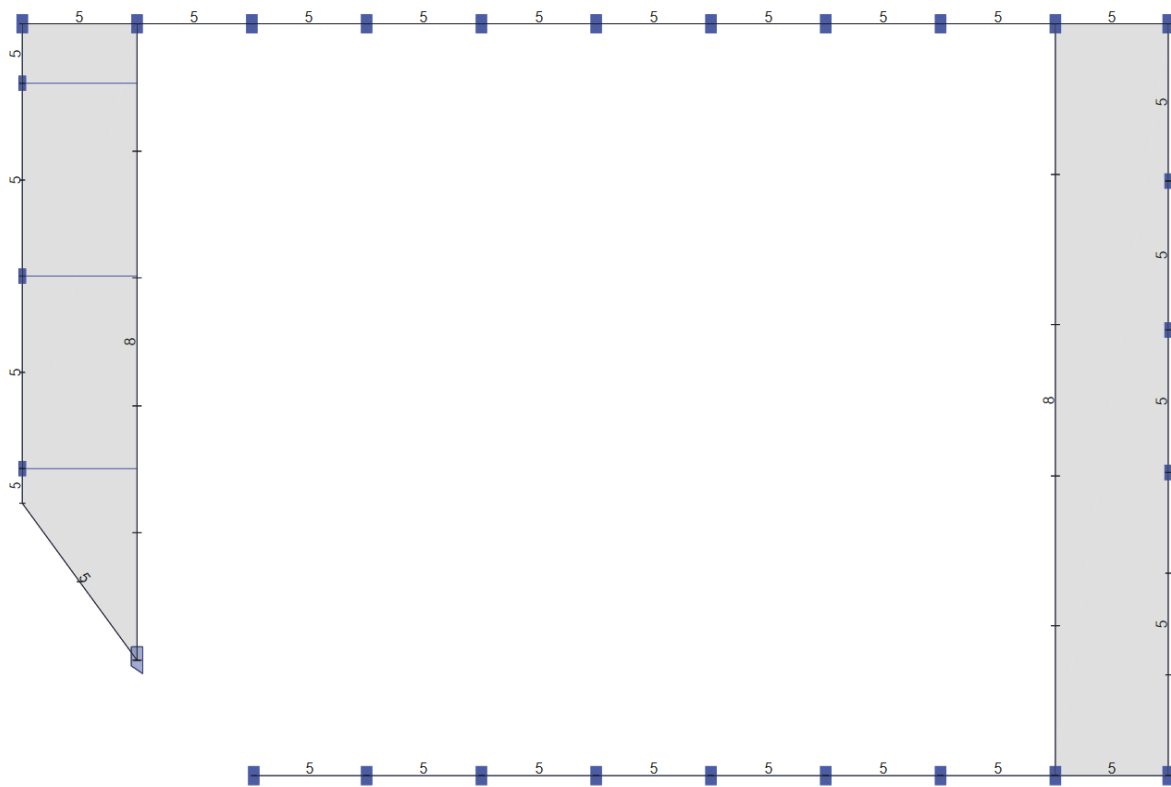


Figure 5-25 Blockworks Load at ROOF Floor Slab (Block)

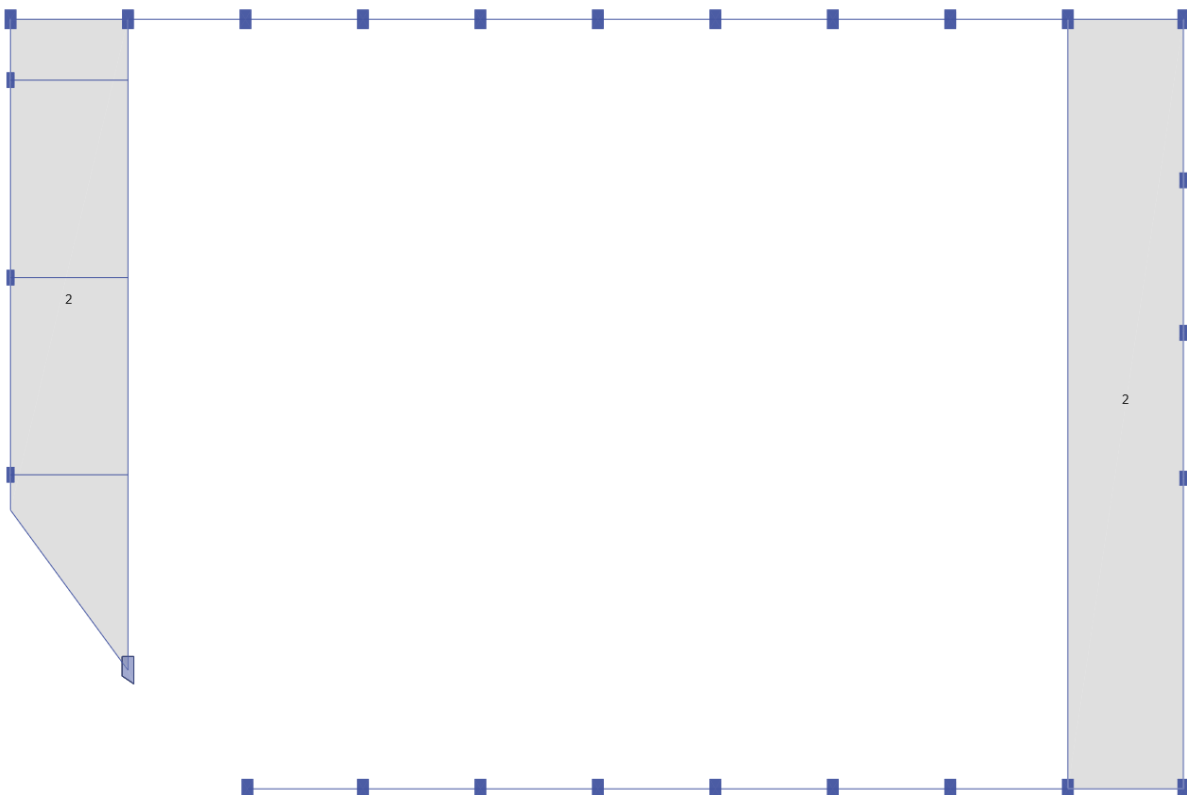


Figure 5-26 Super Imposed Load at Roof Floor Slab (SID)

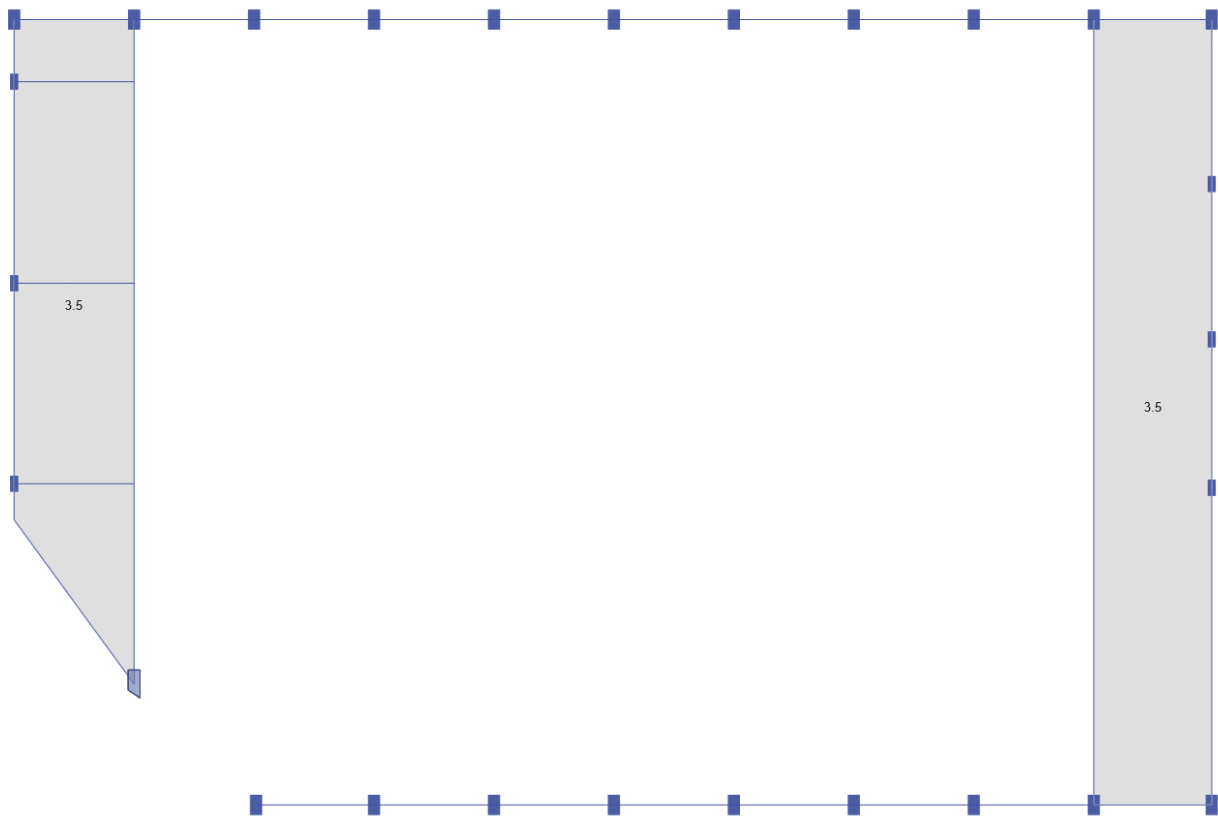


Figure 5-27 Live Load at Roof Floor Slab (LL Roof)

## 5.2 SAFE Model

The slab and beam analysis and design shall be performed using SAFE software. Ground Floor, Typical Floor and Roof floor in ETABS model are exported to SAFE model to check the design of slab and beams.

### 5.2.1 GYMNASIUM PART

#### I. Ground floor

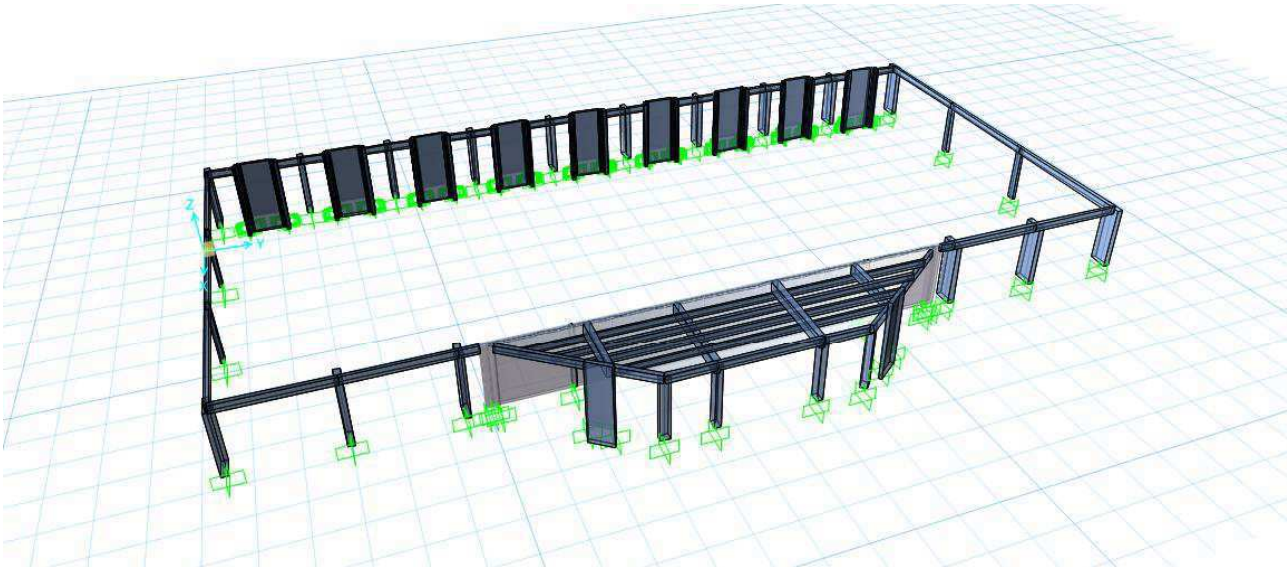


Figure 5-28 3D View at GROUND Floor

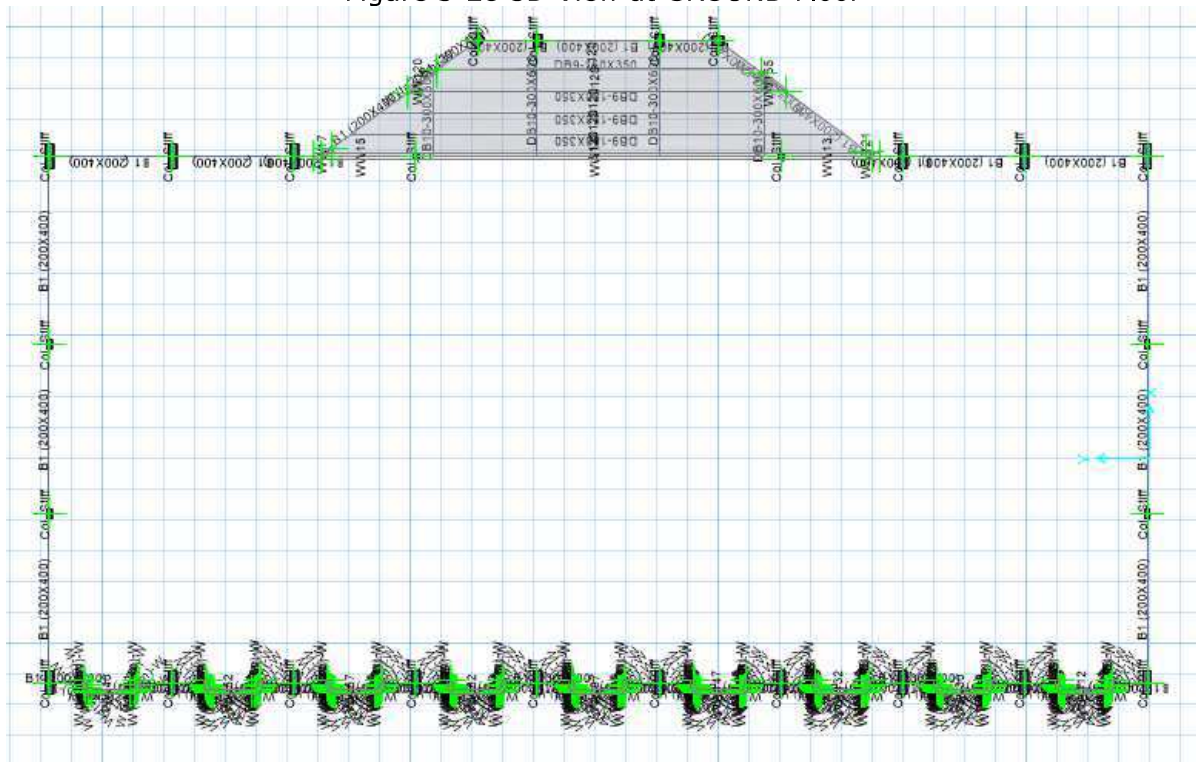


Figure 5-29 Section Assignment at GROUND Floor

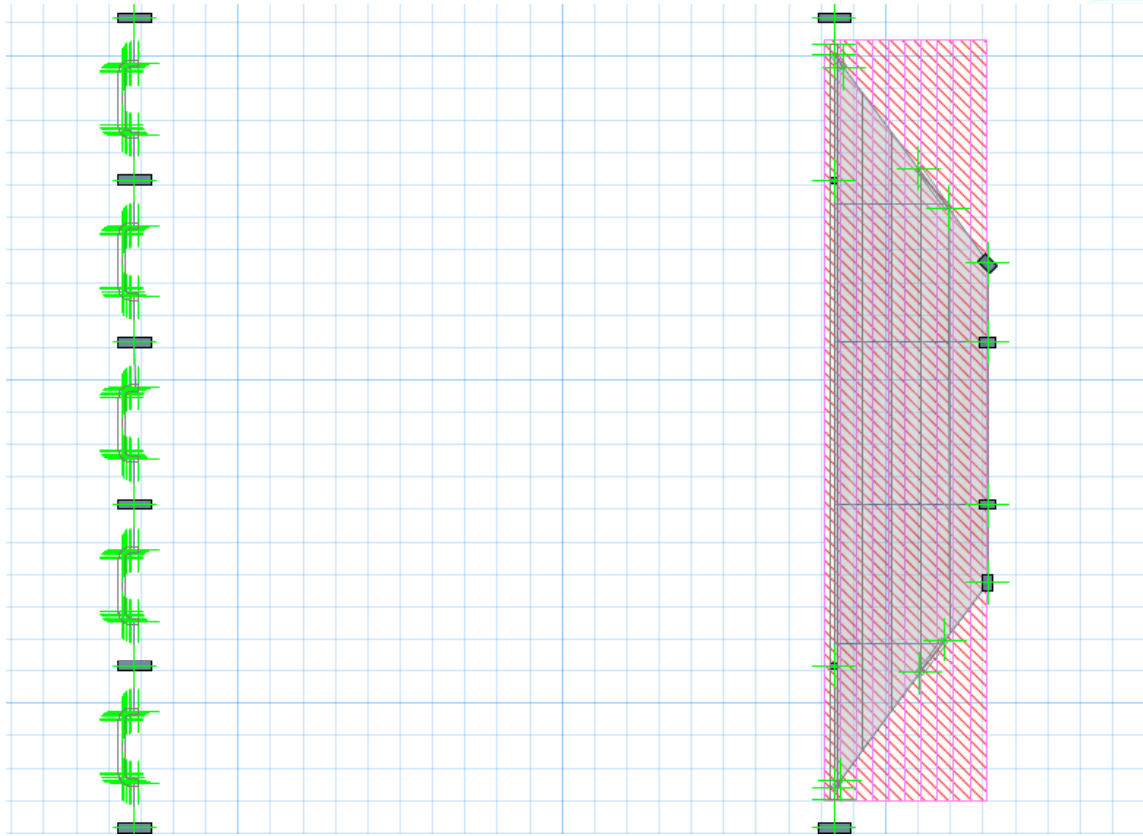


Figure 5-30 Design Strip Layer A at GROUND Floor

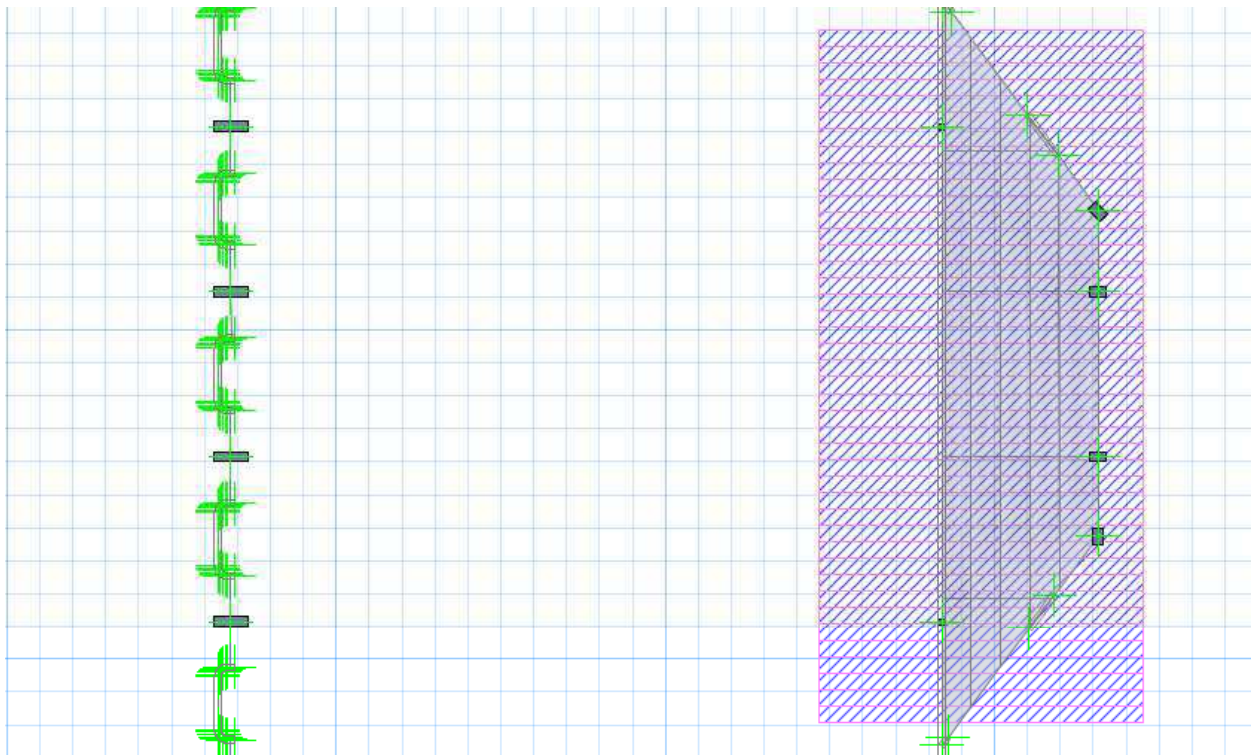


Figure 5-31 Design Strip Layer B at GROUND Floor



## I. Roof Floor

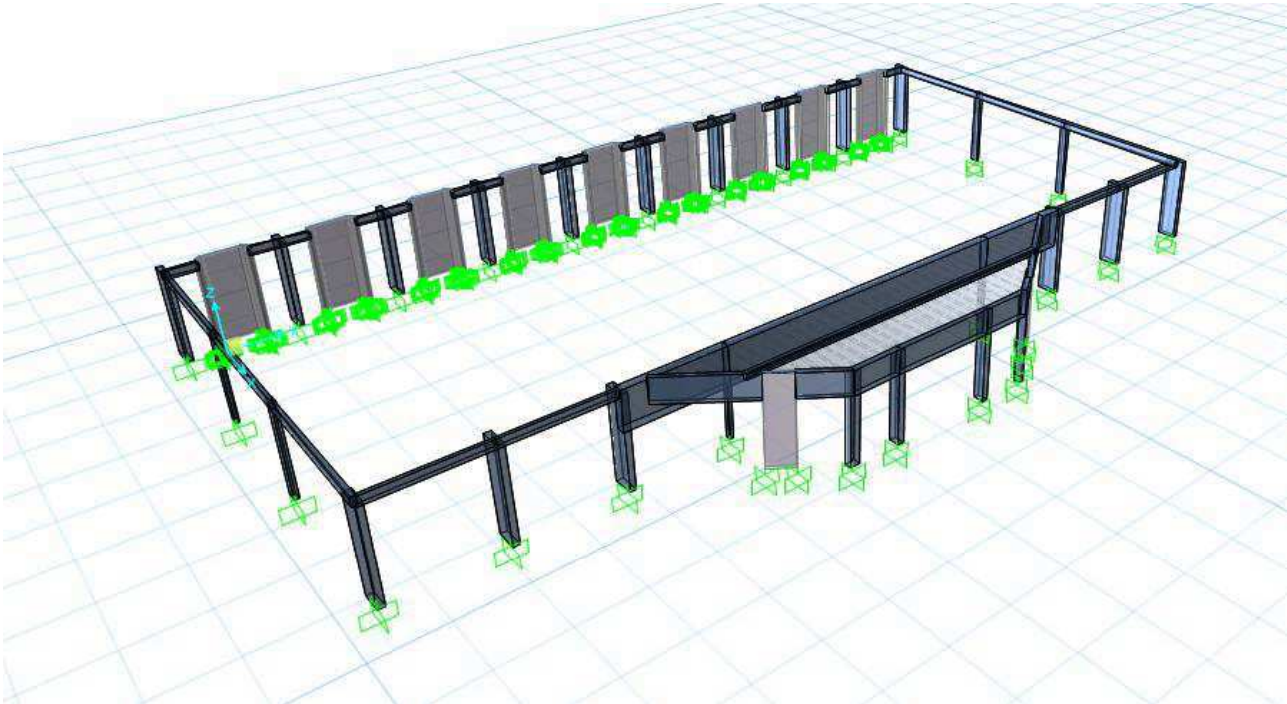


Figure 5-32 3D View at Roof Floor

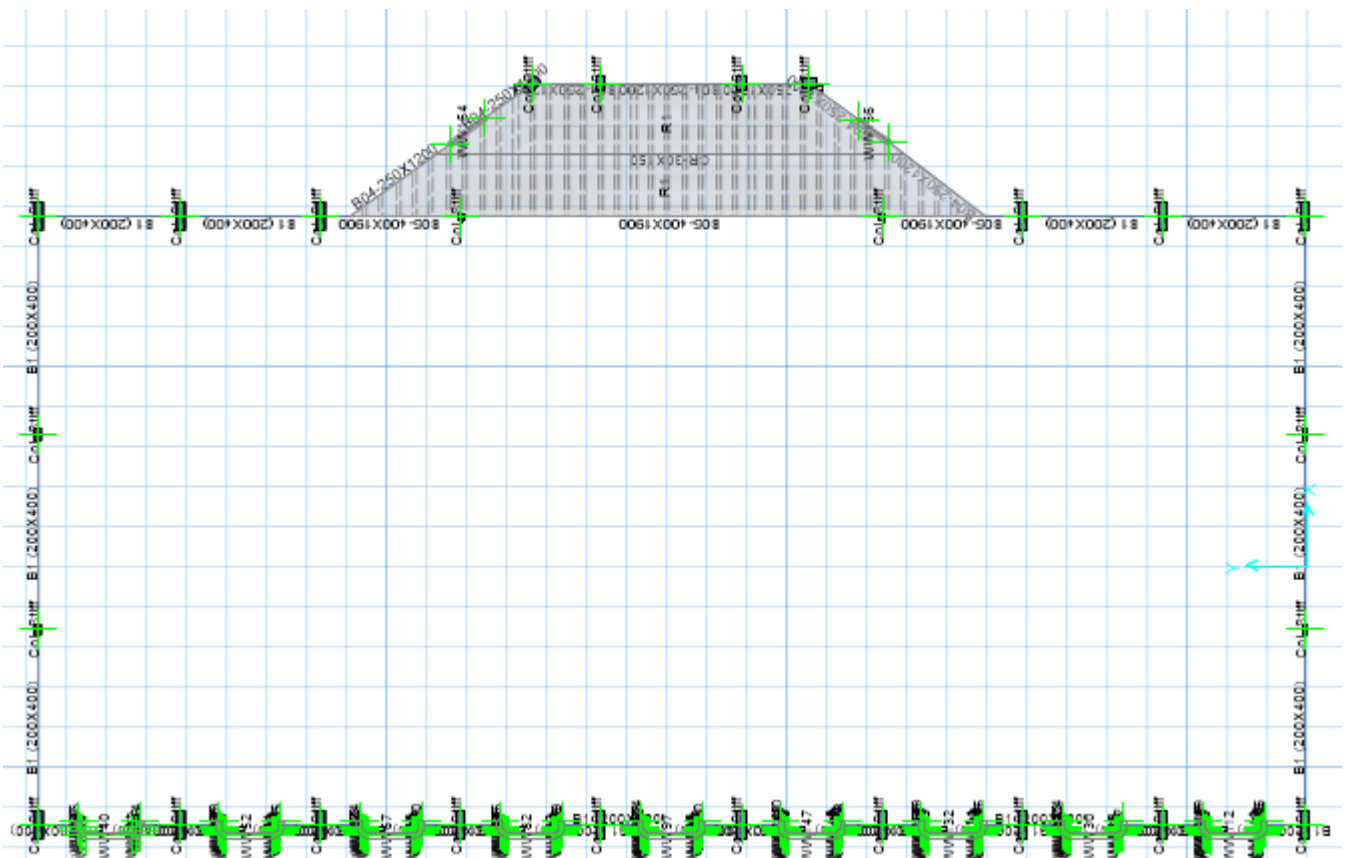


Figure 5-33 Section Assignment Roof Floor



## 5.2.2 ADITORIUM PART

### I. Ground floor

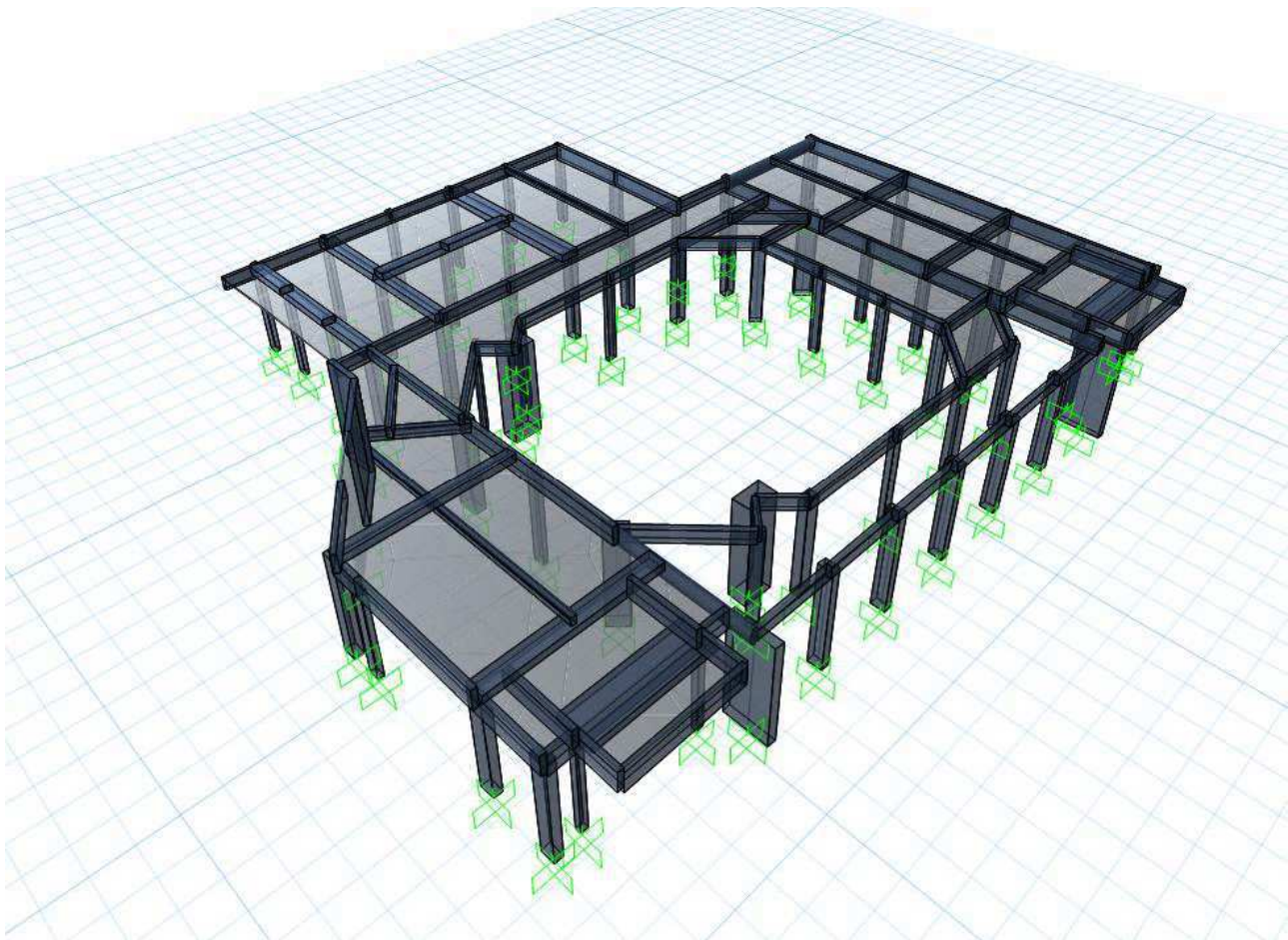


Figure 5-35 3D View at GROUND Floor



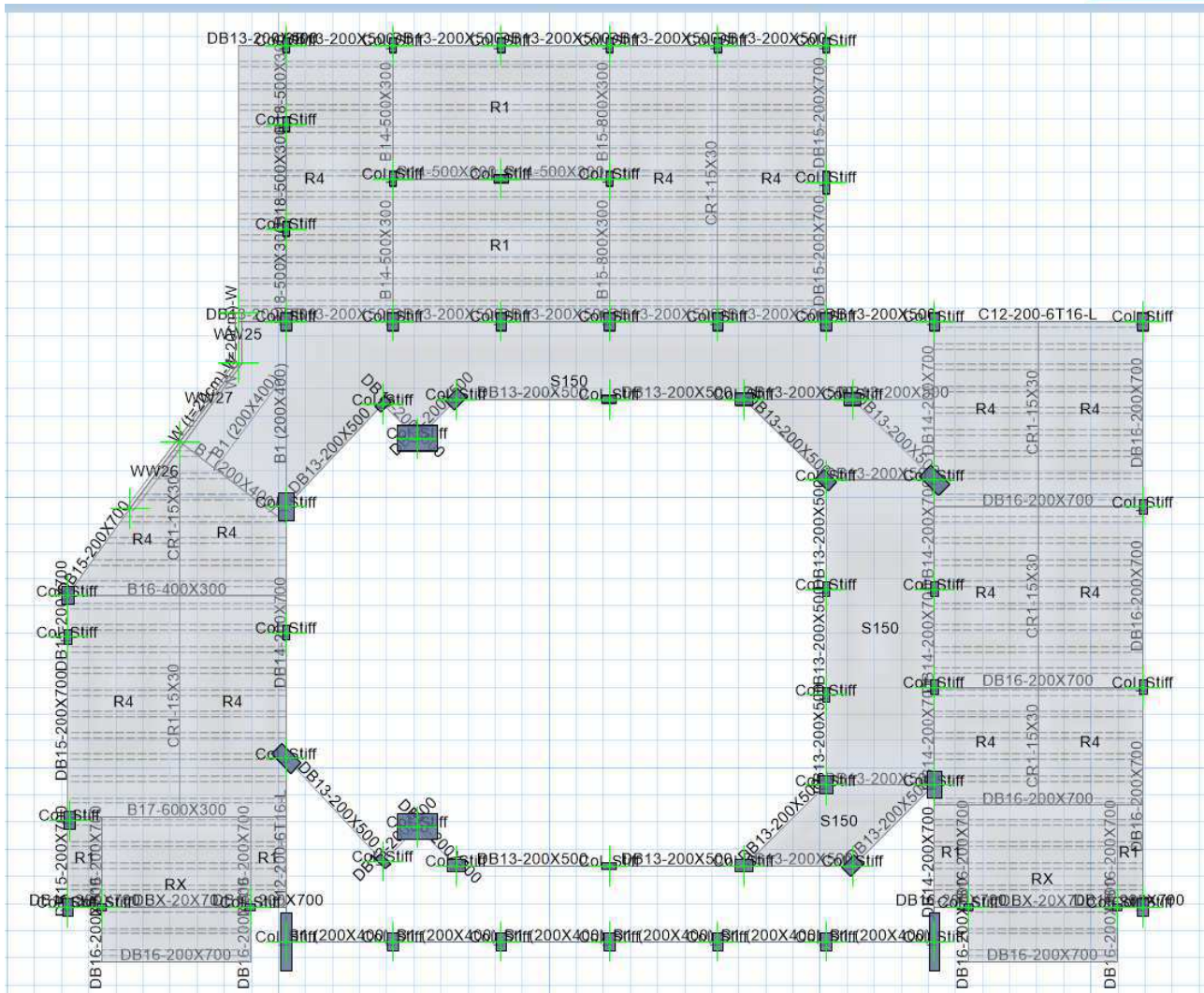


Figure 5-36 Section Assignment at GROUND Floor

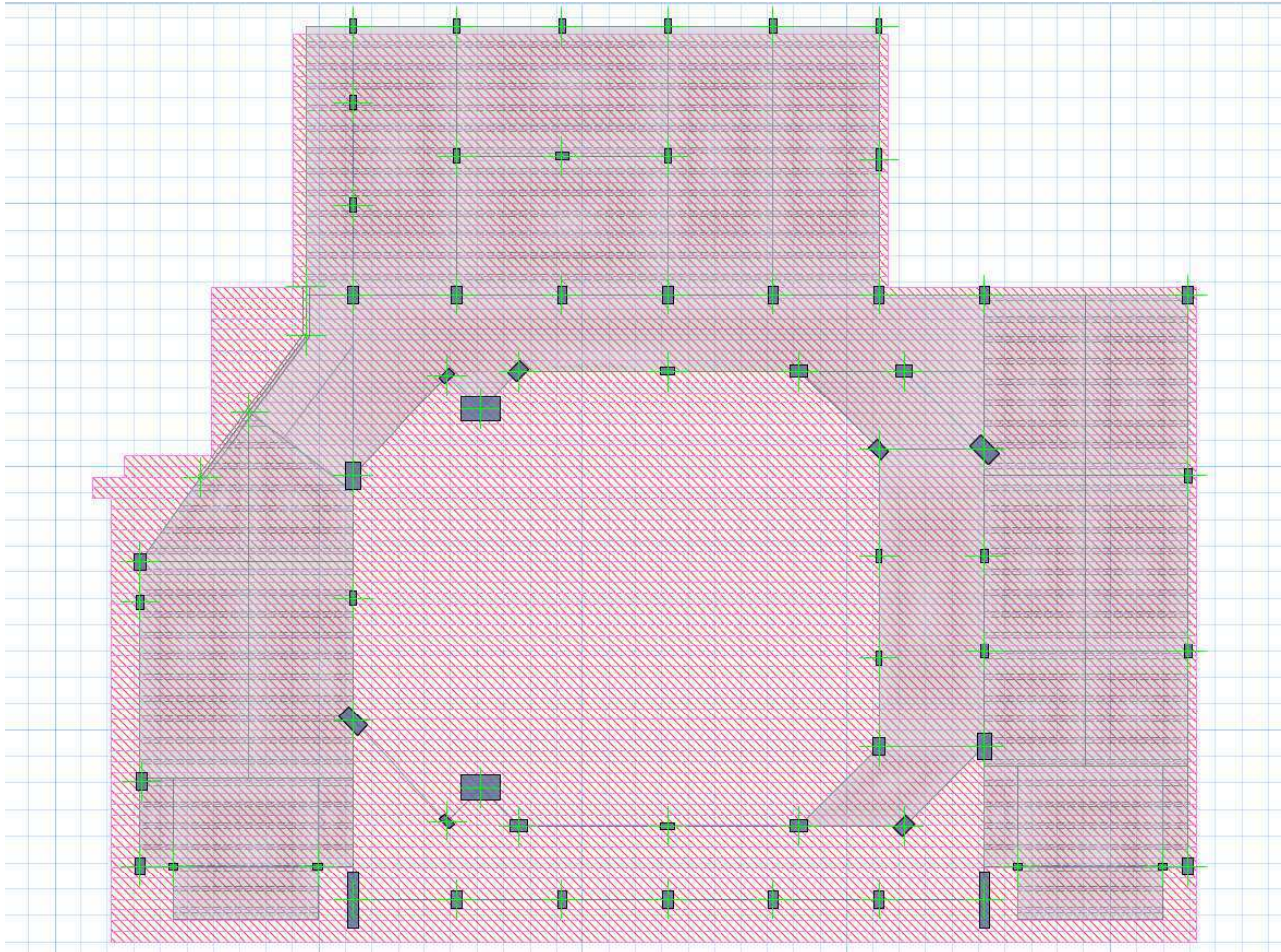


Figure 5-37 Design Strip Layer A at GROUND Floor



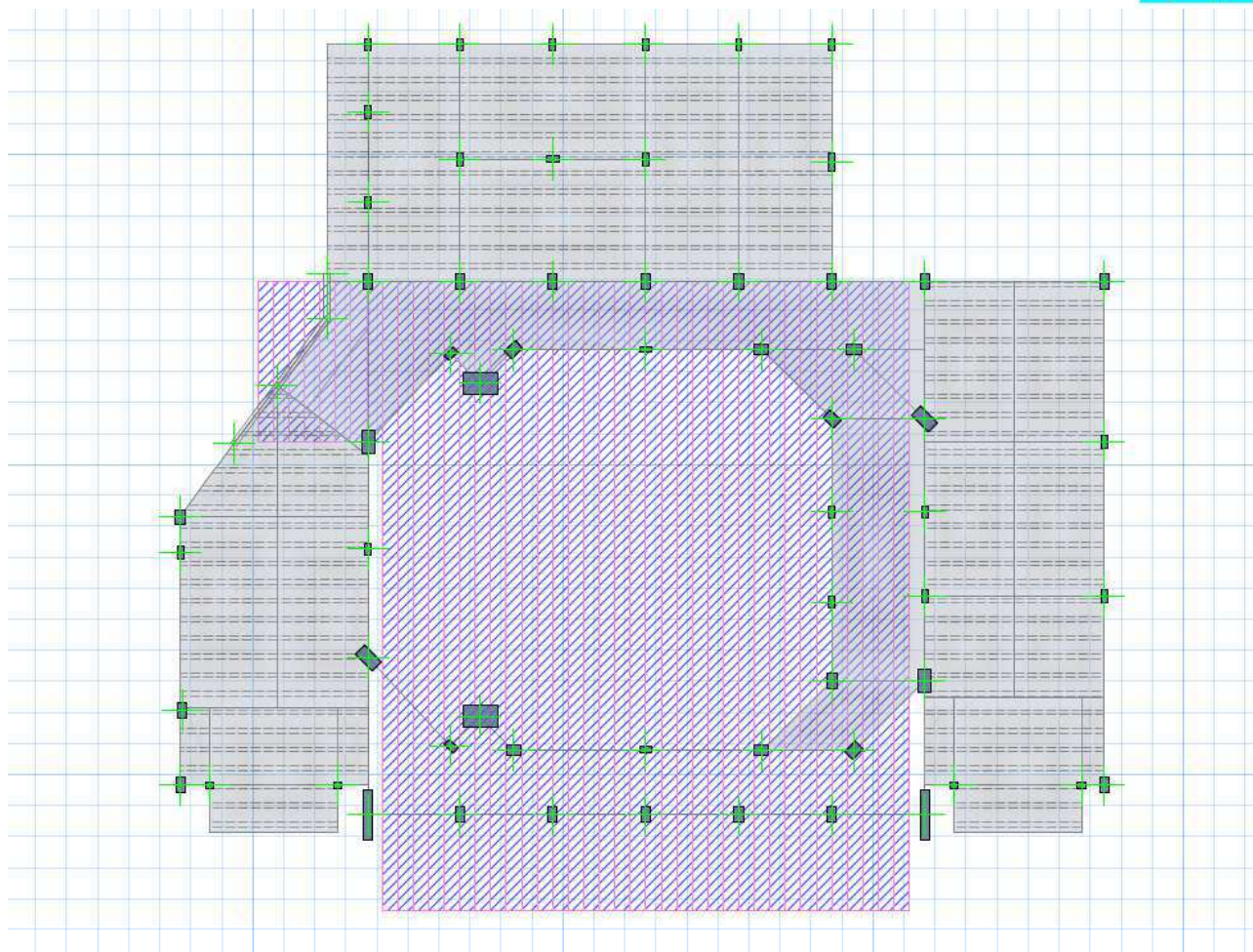


Figure 5-38 Design Strip Layer B at GROUND Floor

## II. Roof Floor

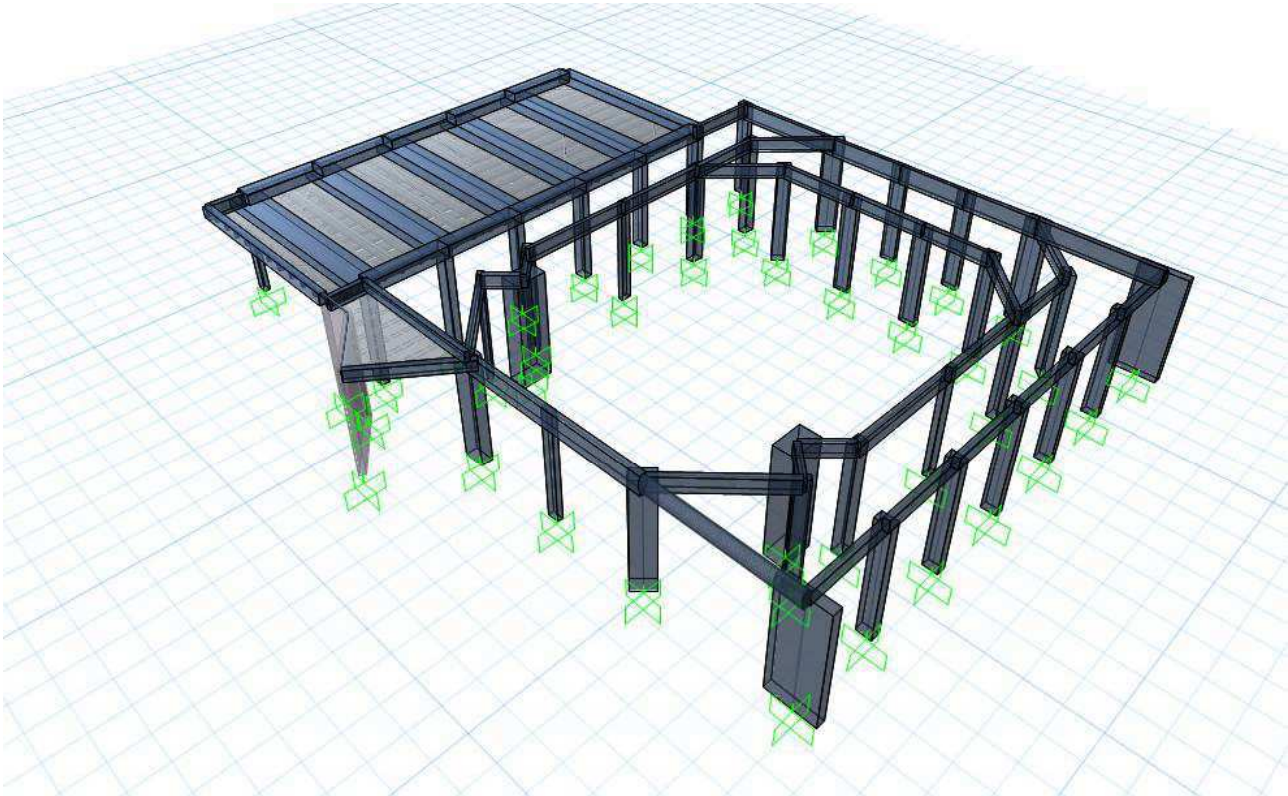


Figure 5-39 3D View at Roof Floor



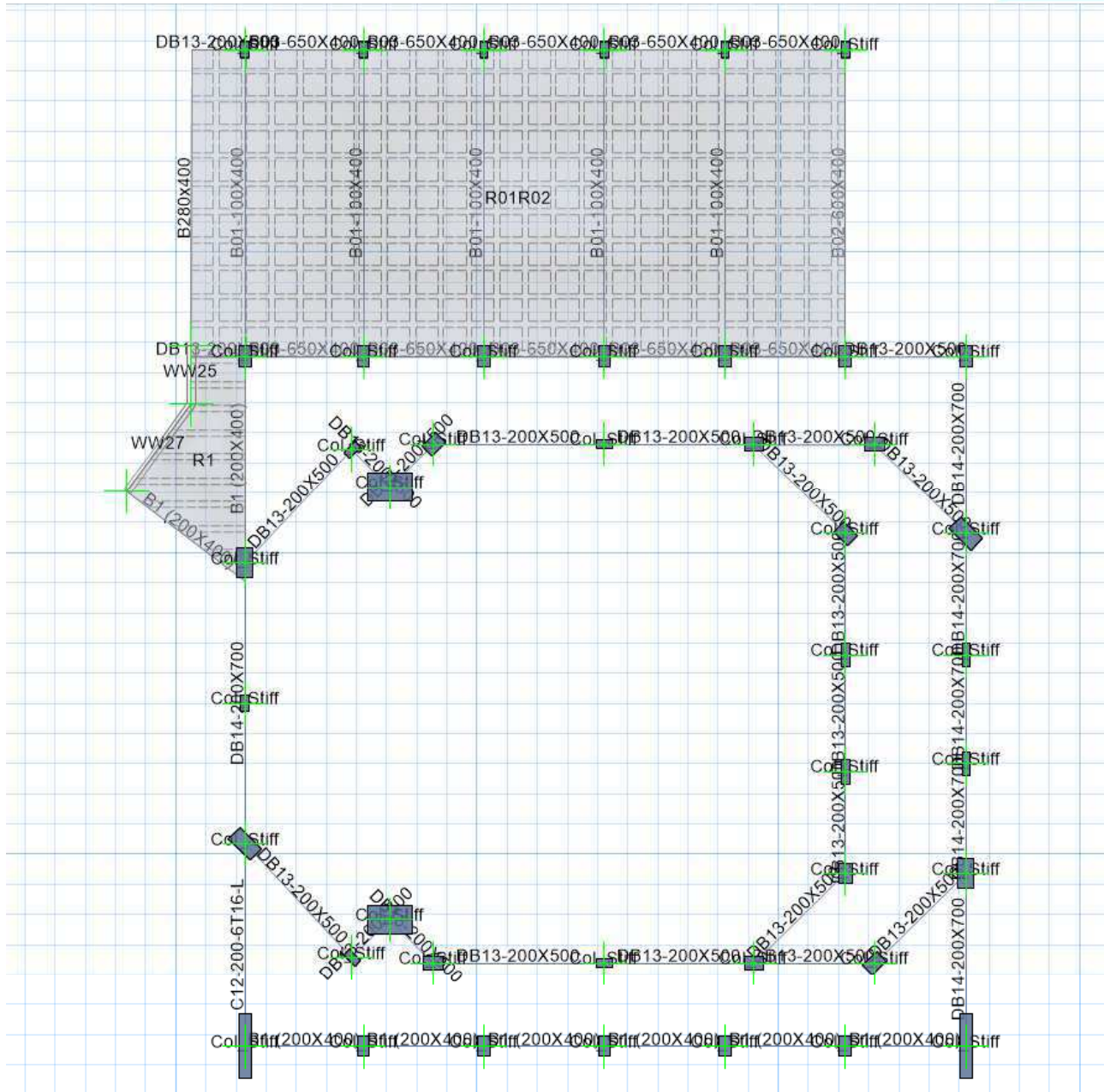


Figure 5-40 Section Assignment Roof Floor

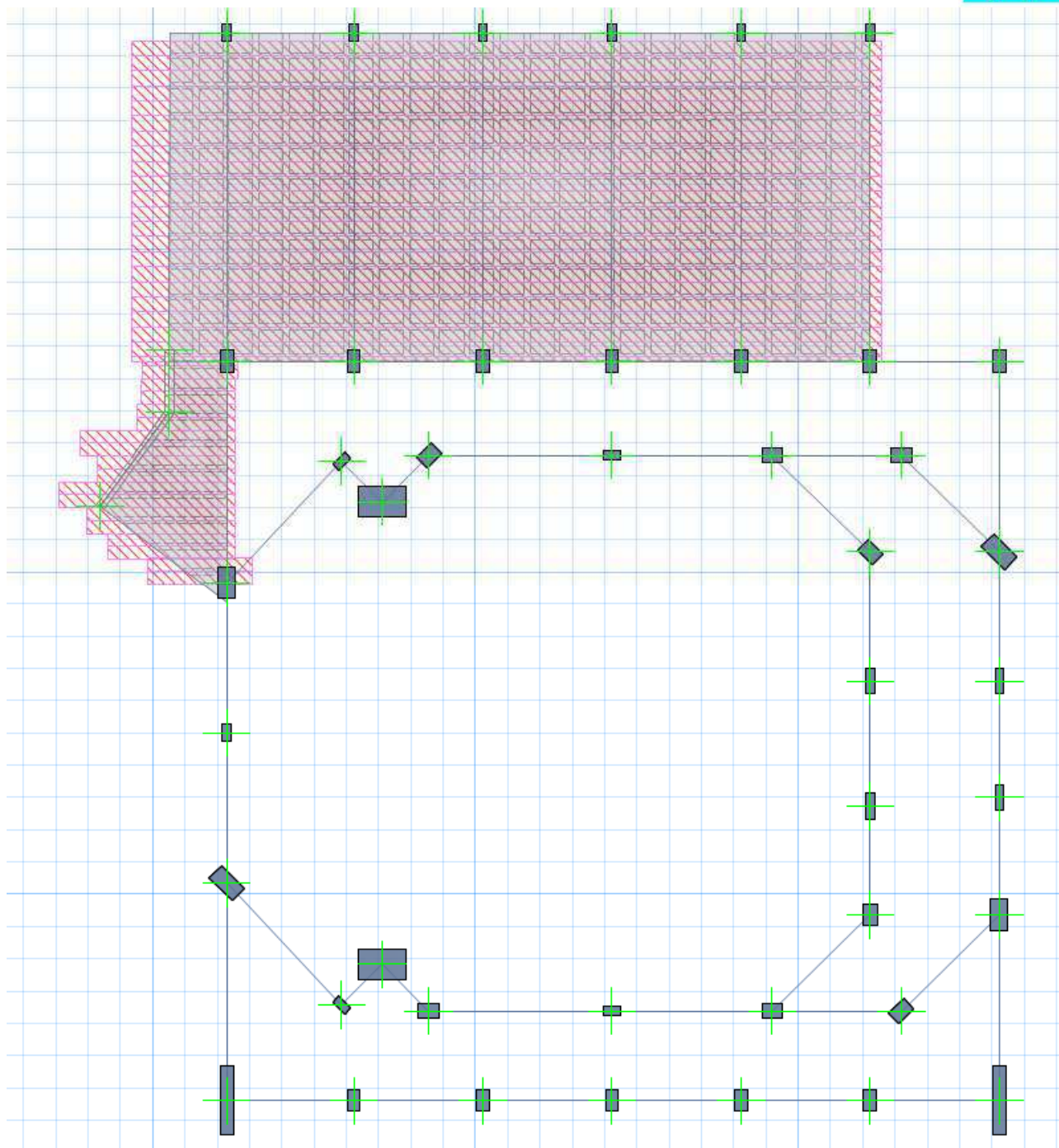


Figure 5-41 Design Strip Layer A at Roof Floor

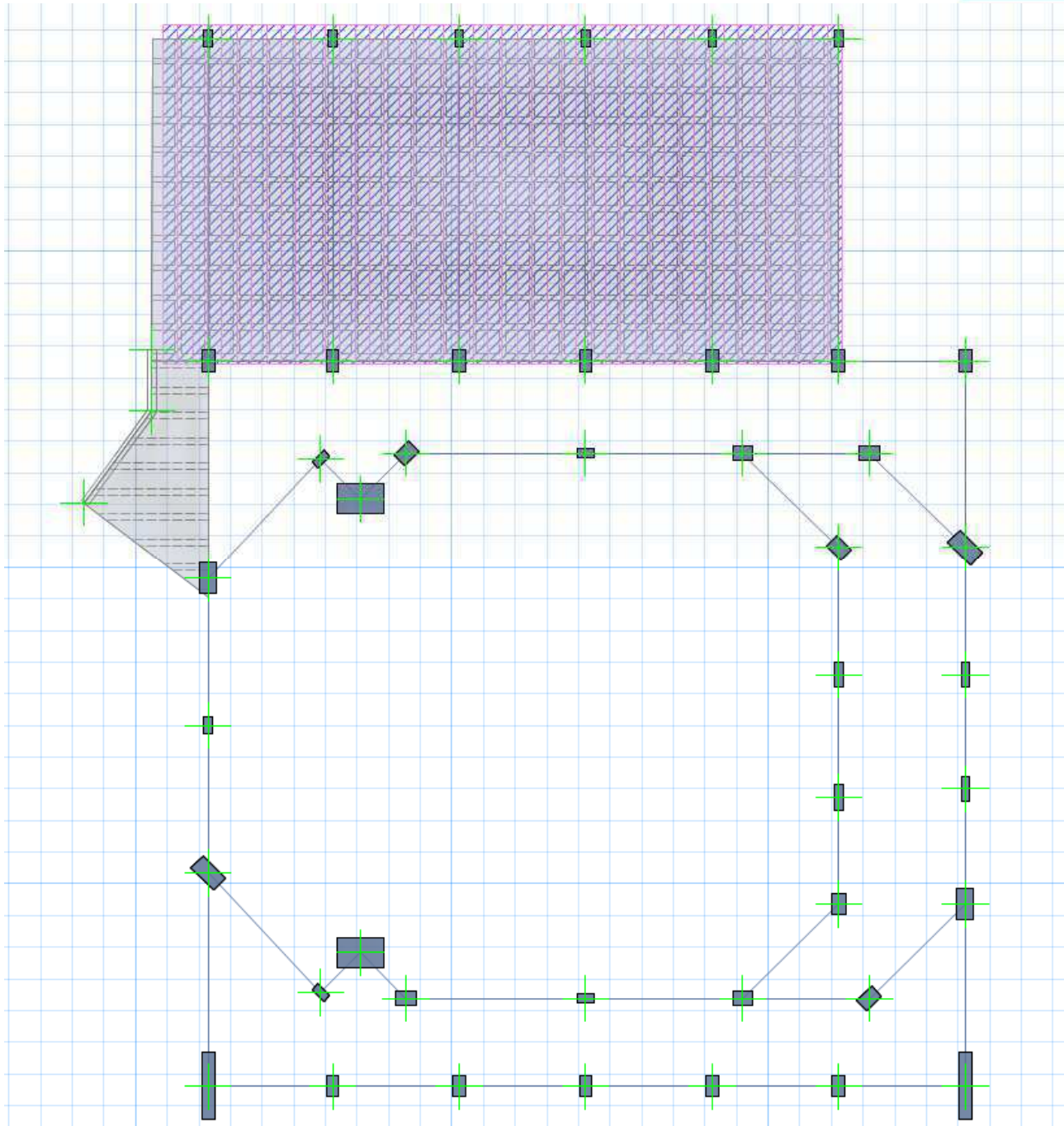


Figure 5-42 Design Strip Layer B at Roof Floor



## 5.2.3 SWIMMING PART

### I. GROUND

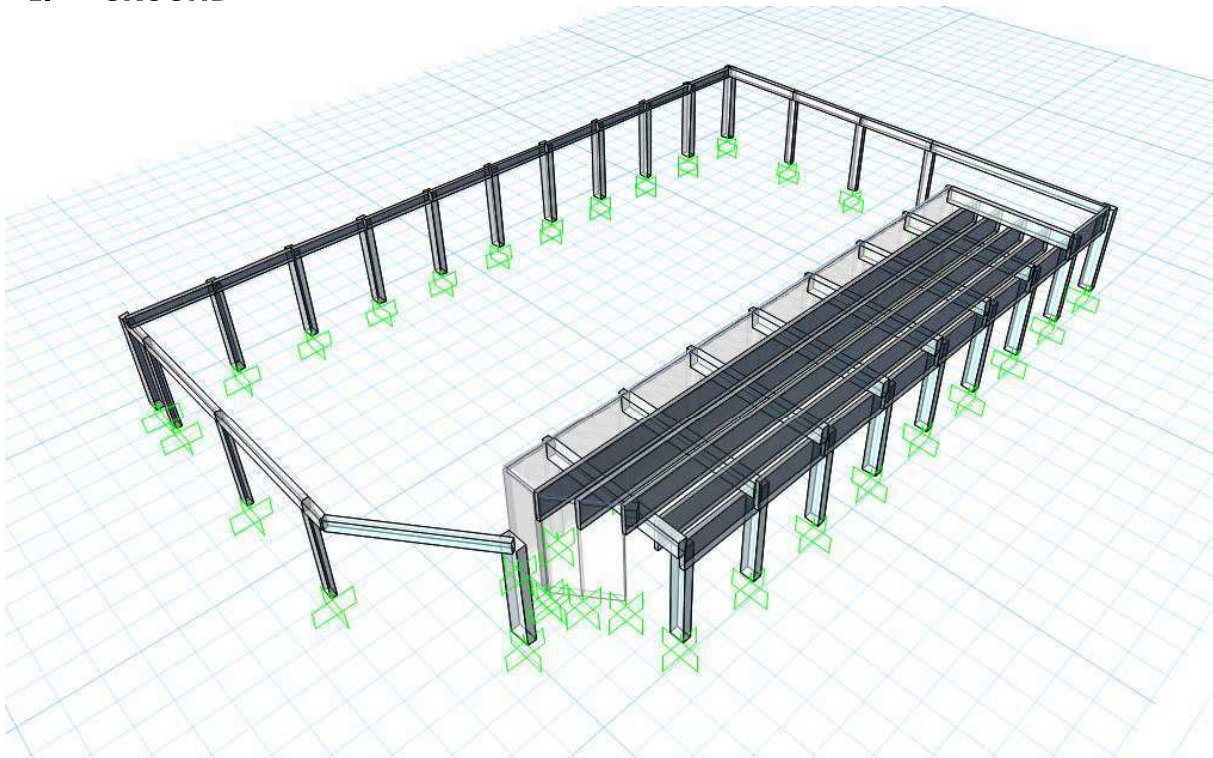


Figure 5-43 3D View at Roof Floor

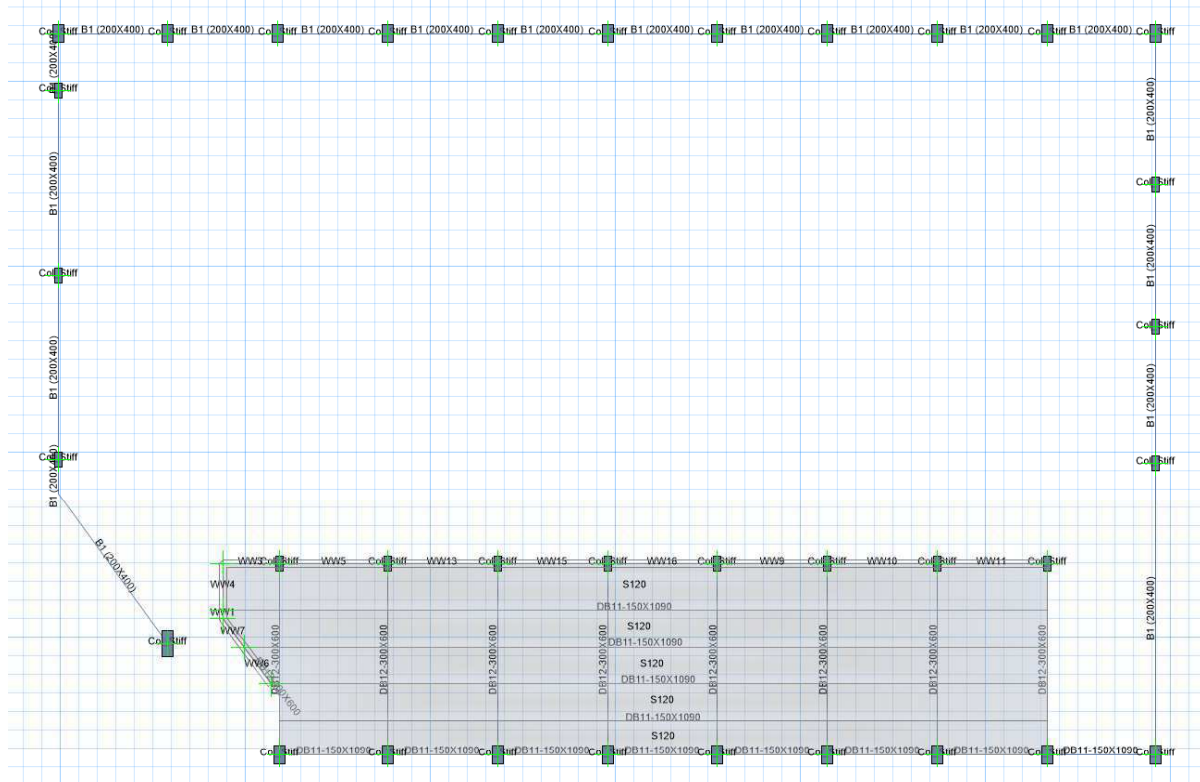


Figure 5-44 Section Assignment Roof Floor

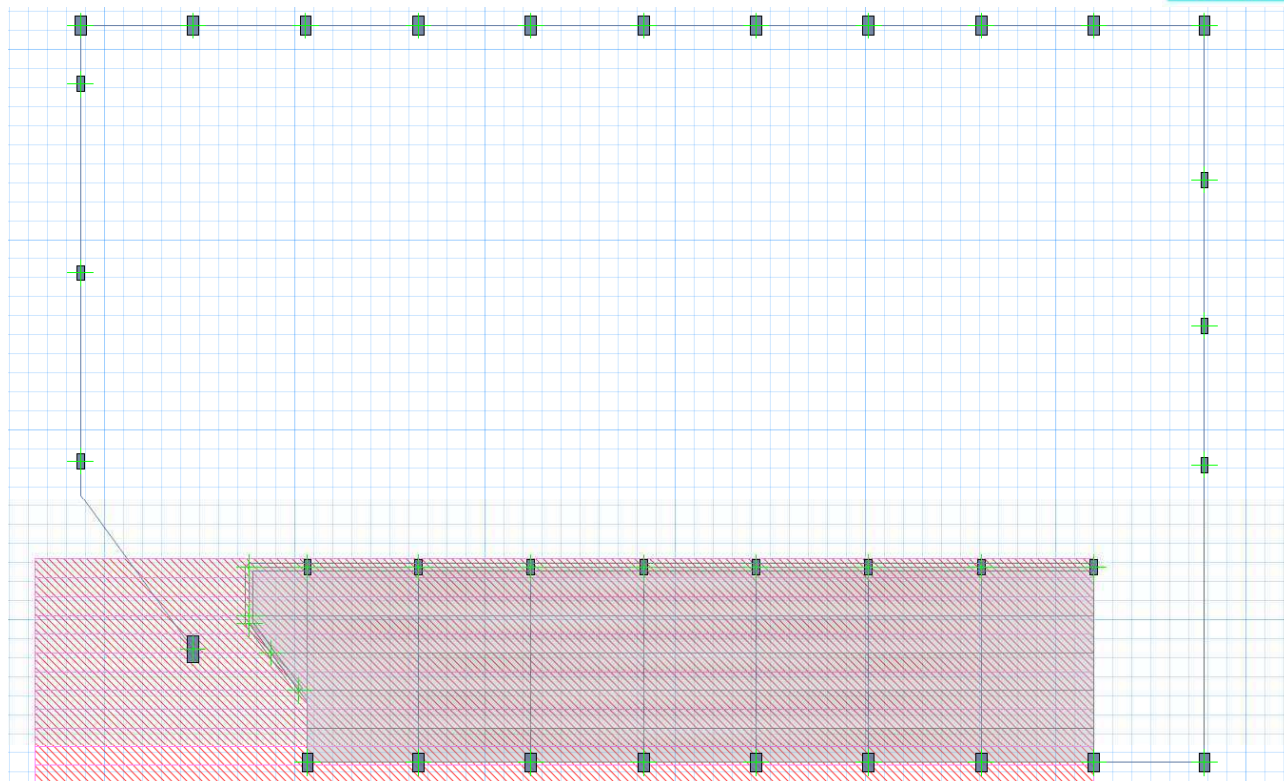


Figure 5-45 Design Strip Layer A at Roof Floor

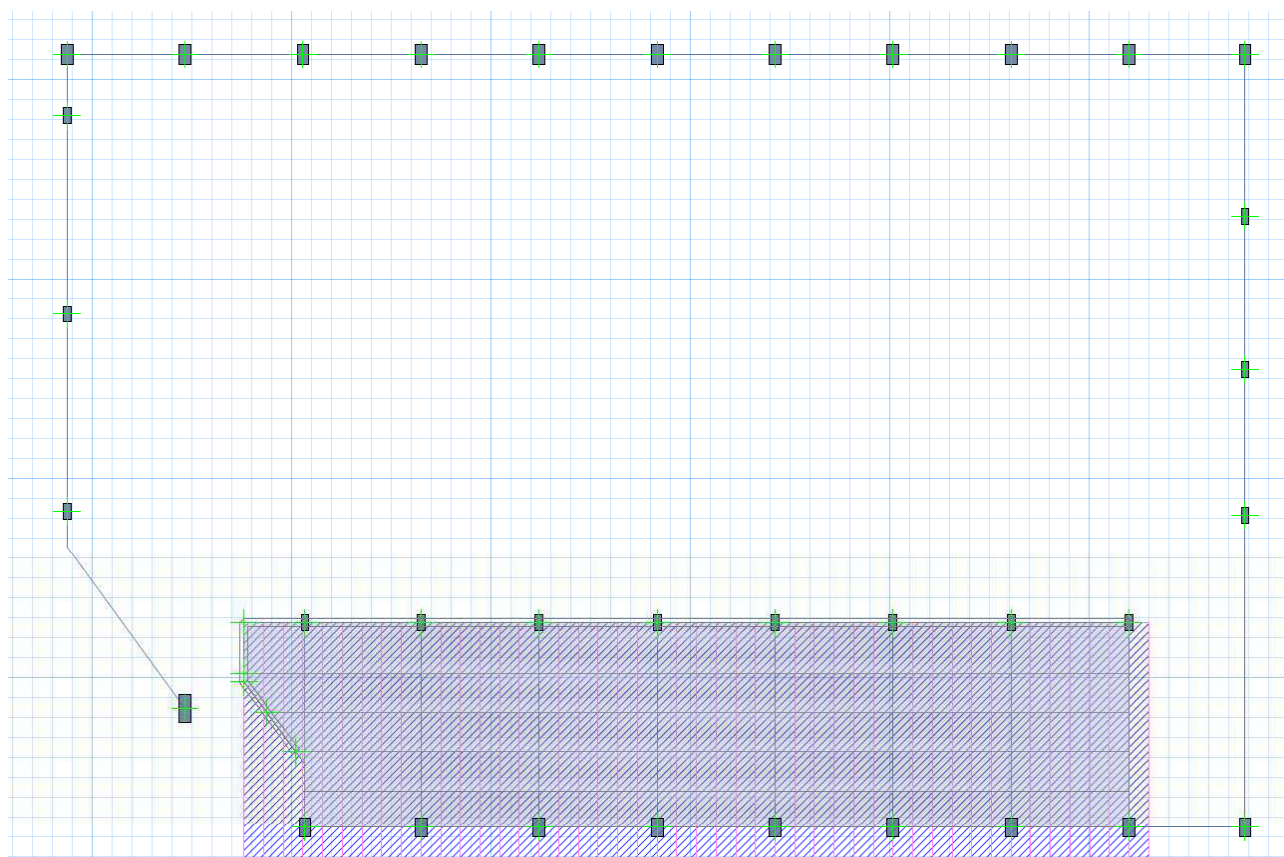


Figure 5-46 Design Strip Layer B at Roof Floor



## II. Roof Floor

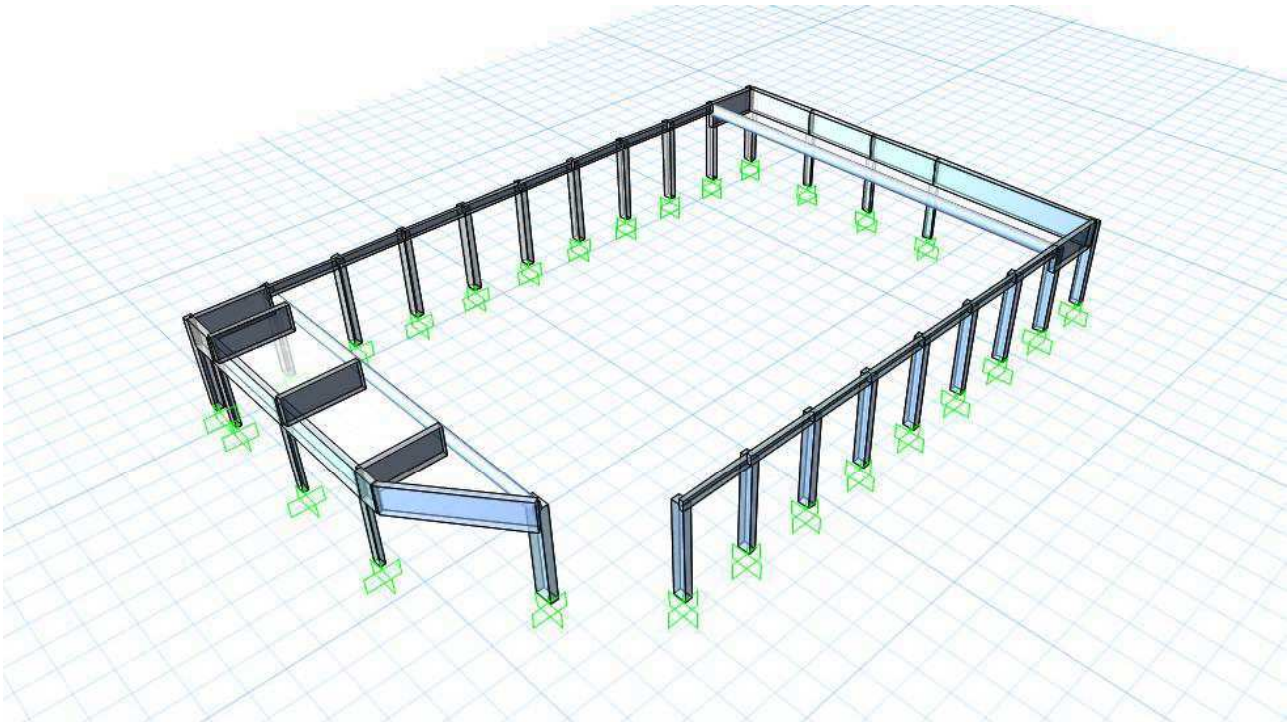


Figure 5-47 3D View at Roof Floor

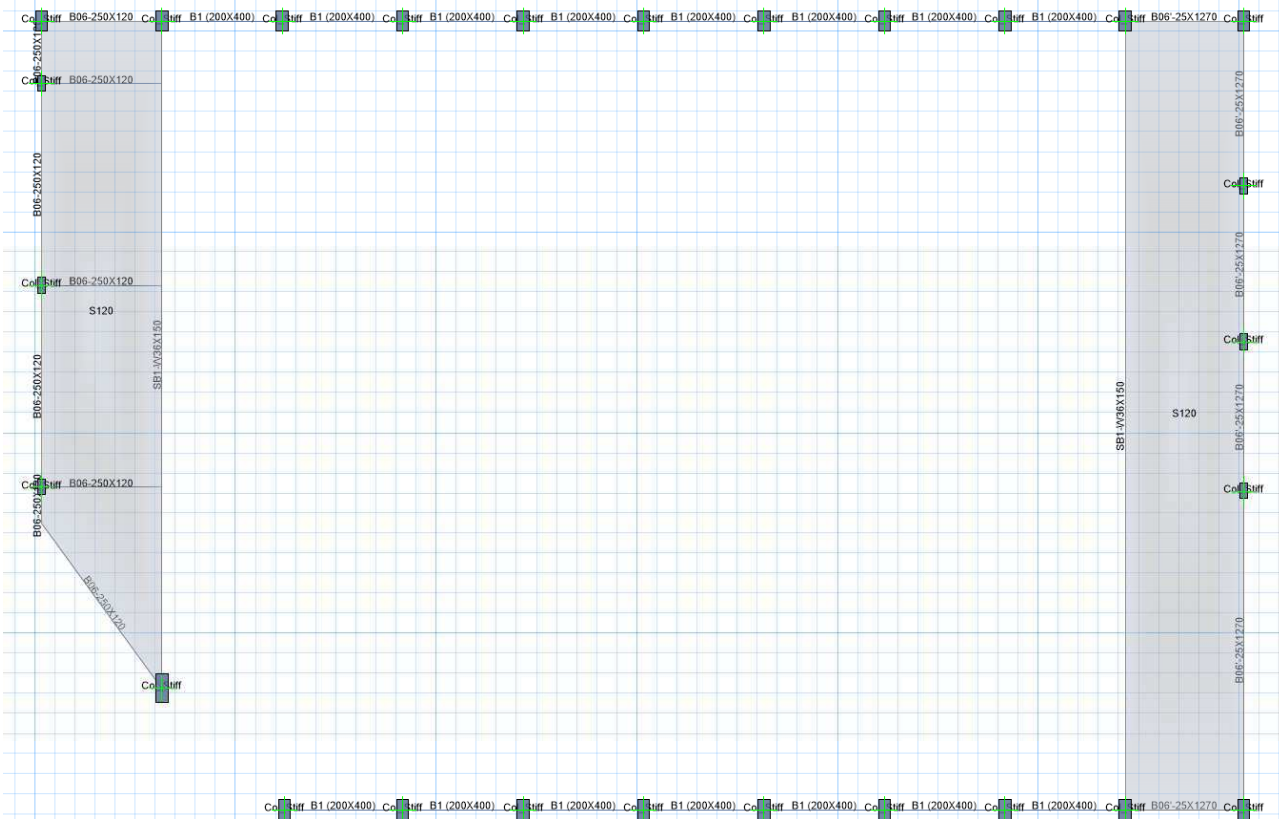


Figure 5-48 Section Assignment Roof Floor

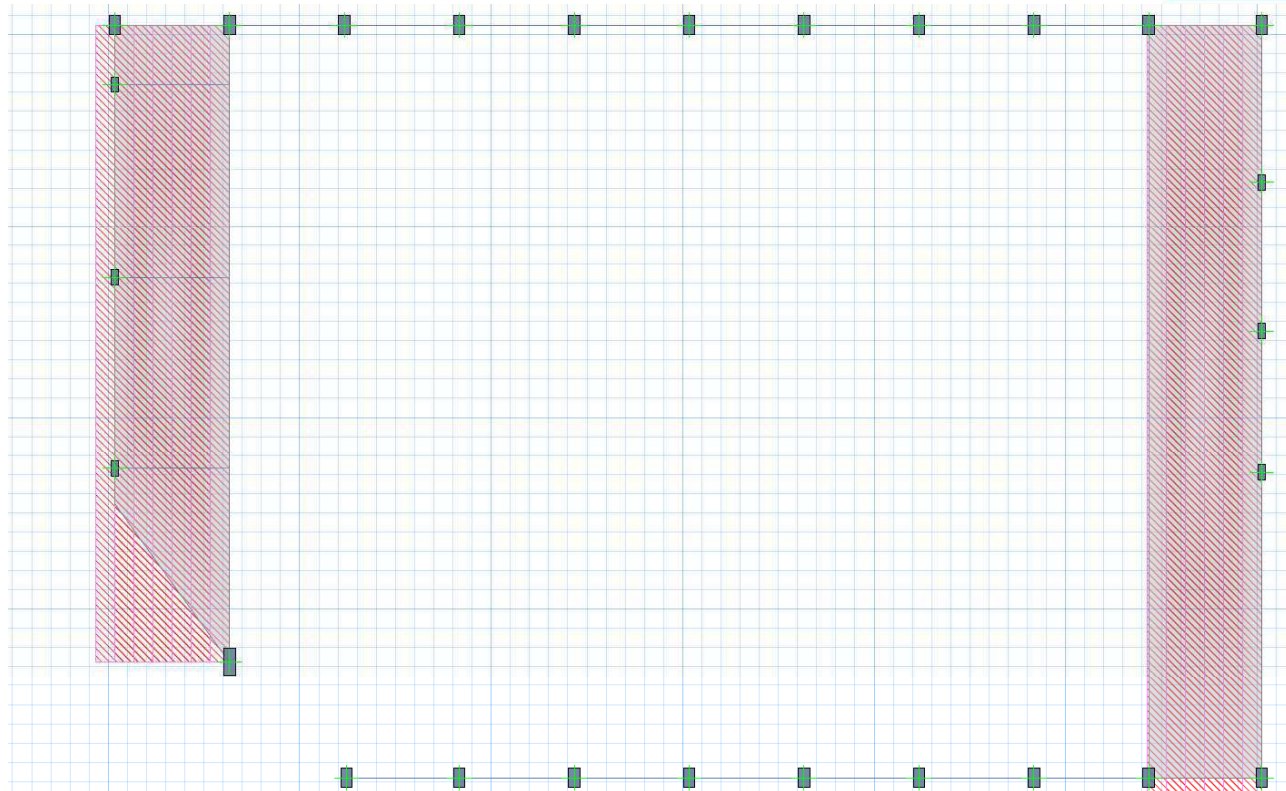


Figure 5-49 Design Strip Layer A at Roof Floor

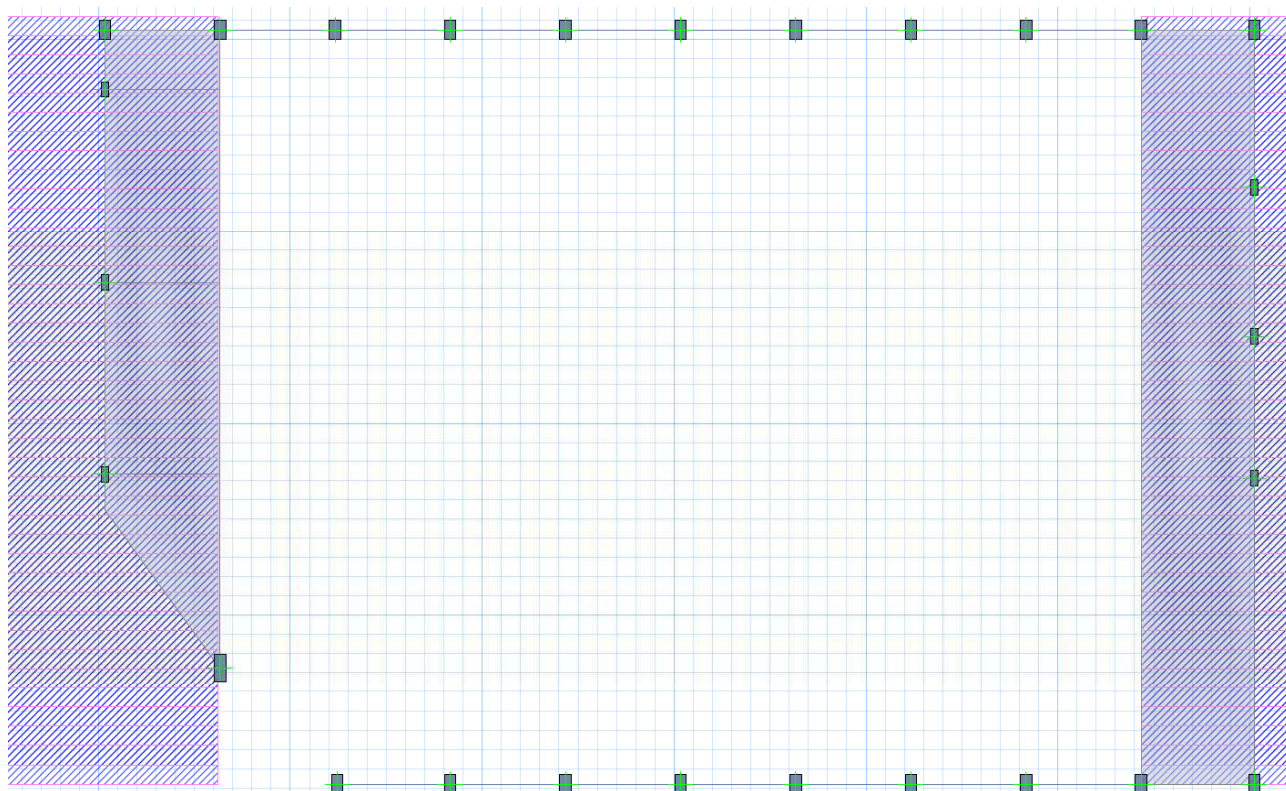


Figure 5-50 Design Strip Layer B at Roof Floor



## 6 Design Result

### 6.1 ETABS Design Result (Walls and Columns)

#### I. Column Design

In the design of concrete columns, longitudinal steel is specified. Then, the program calculates the column stress in terms of a column capacity ratio, which is a factor that gives an indication of the stress condition of the column with respect to the capacity of the column.

The factored axial force and bending moments obtained from each loading combination was used to check the capacity of the column. PMM D/C ratio which is greater than 1.0 indicates that the column is not safe. Based on the design results, all columns at Ground Floor are found to be inadequate wherein PMM ratio exceeded 1.0. due to lower concrete compressive strength and its double height length. In addition, a few columns at Roof floor are also found to be under design. The unsafe columns are highlighted in red shown in Figure 6-1.

For shear reinforcement, the programs determine the required reinforcement considering the effect of the axial force on the concrete shear capacity. The maximum required shear reinforcement is checked with the existing shear reinforcement ( $2\text{LT}8-200 = 500\text{mm}^2/\text{m}$ ) and found to be sufficient. The required shear reinforcement for both minor and major shear.

### 6.1.1 GYMNASIUM PART

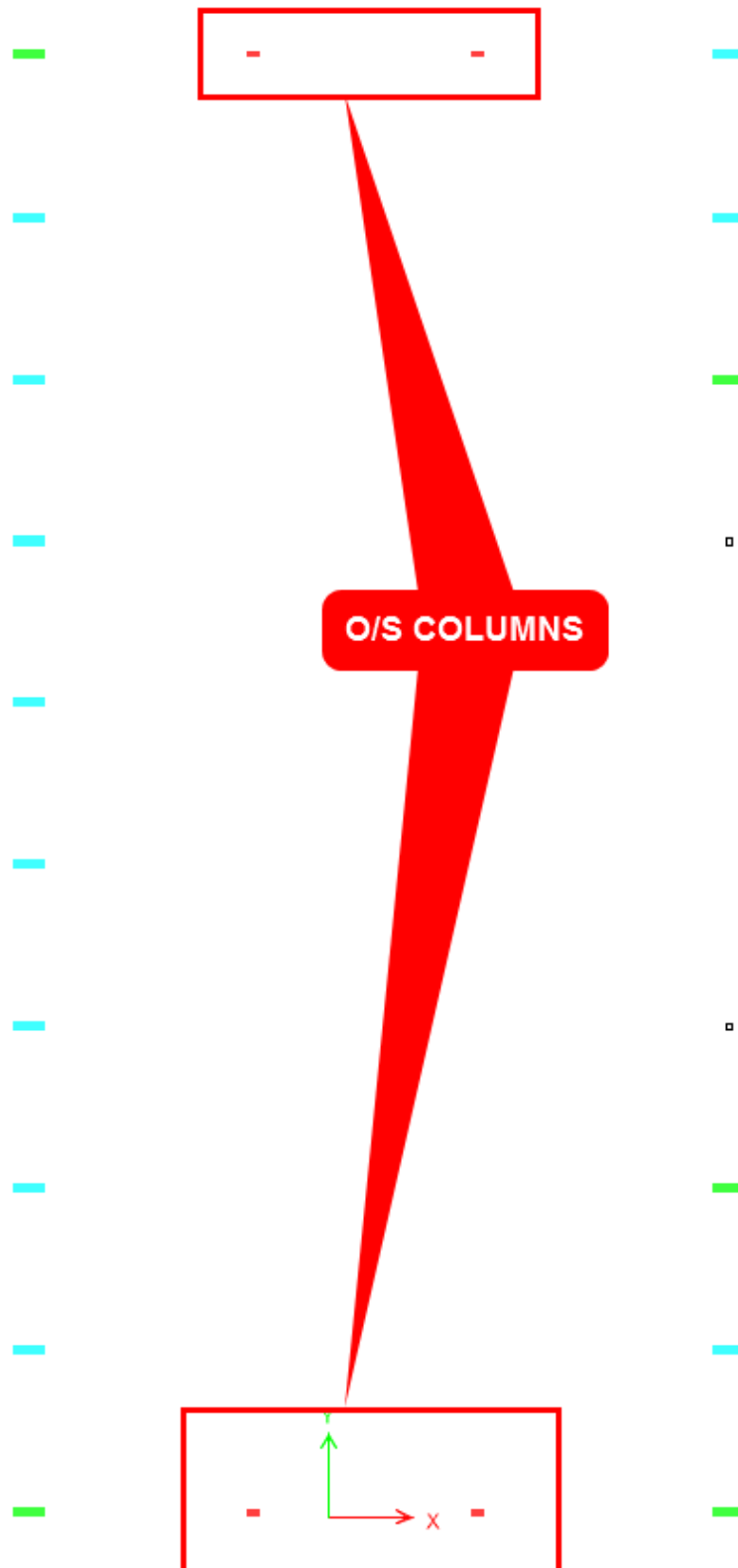


Figure 6-1 Column PMM Ratio at Ground Floor

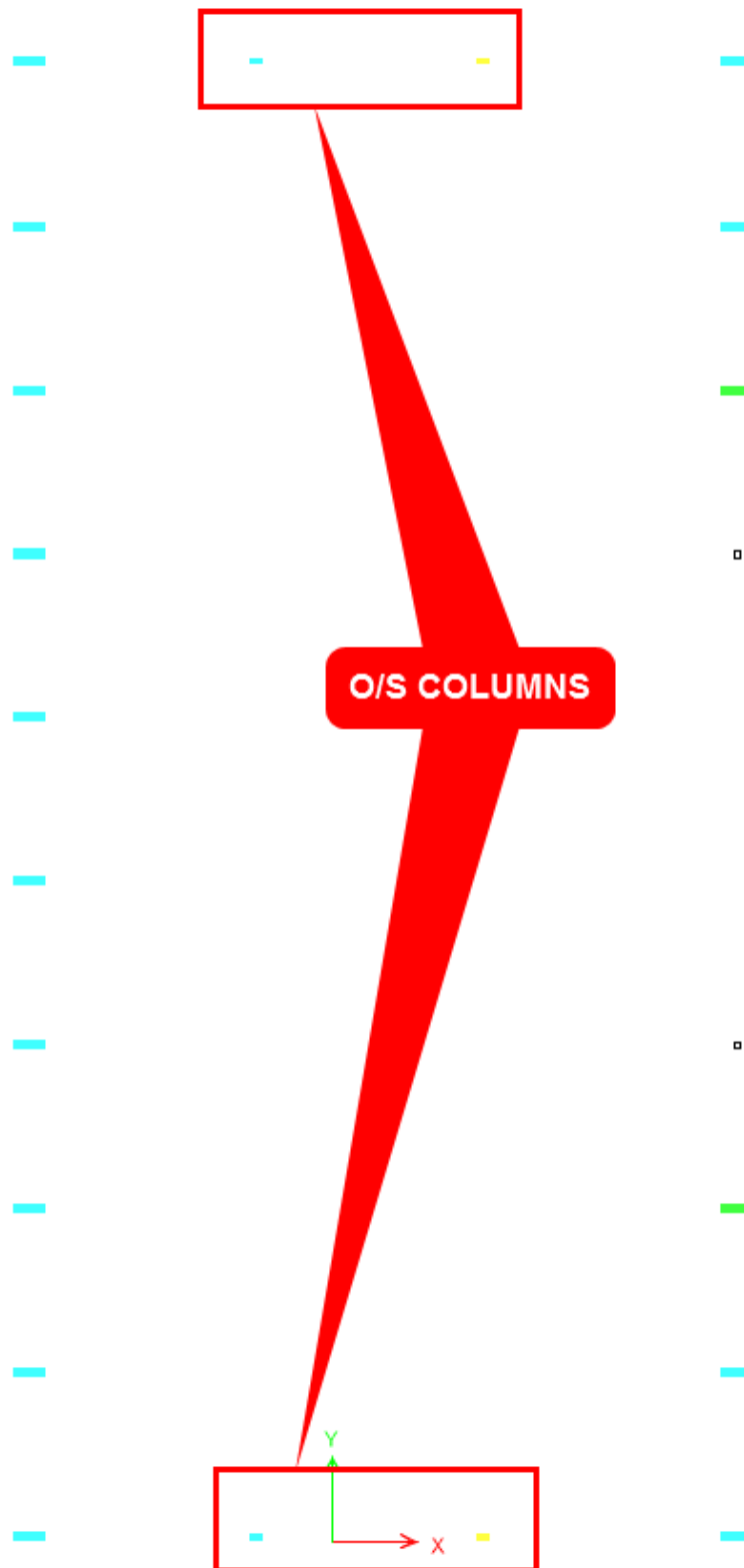


Figure 6-2 Column PMM Ratio at ROOF Floor

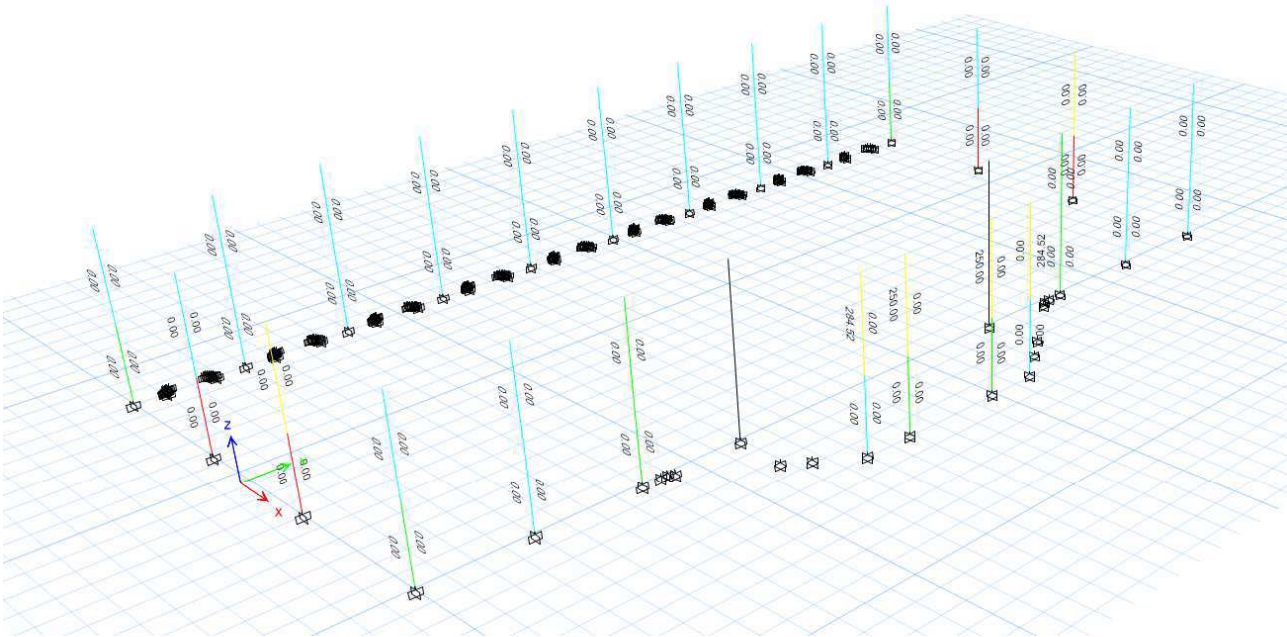


Figure 6-3 Shear Reinforcing

## 6.1.2 AUDTORIUM PART

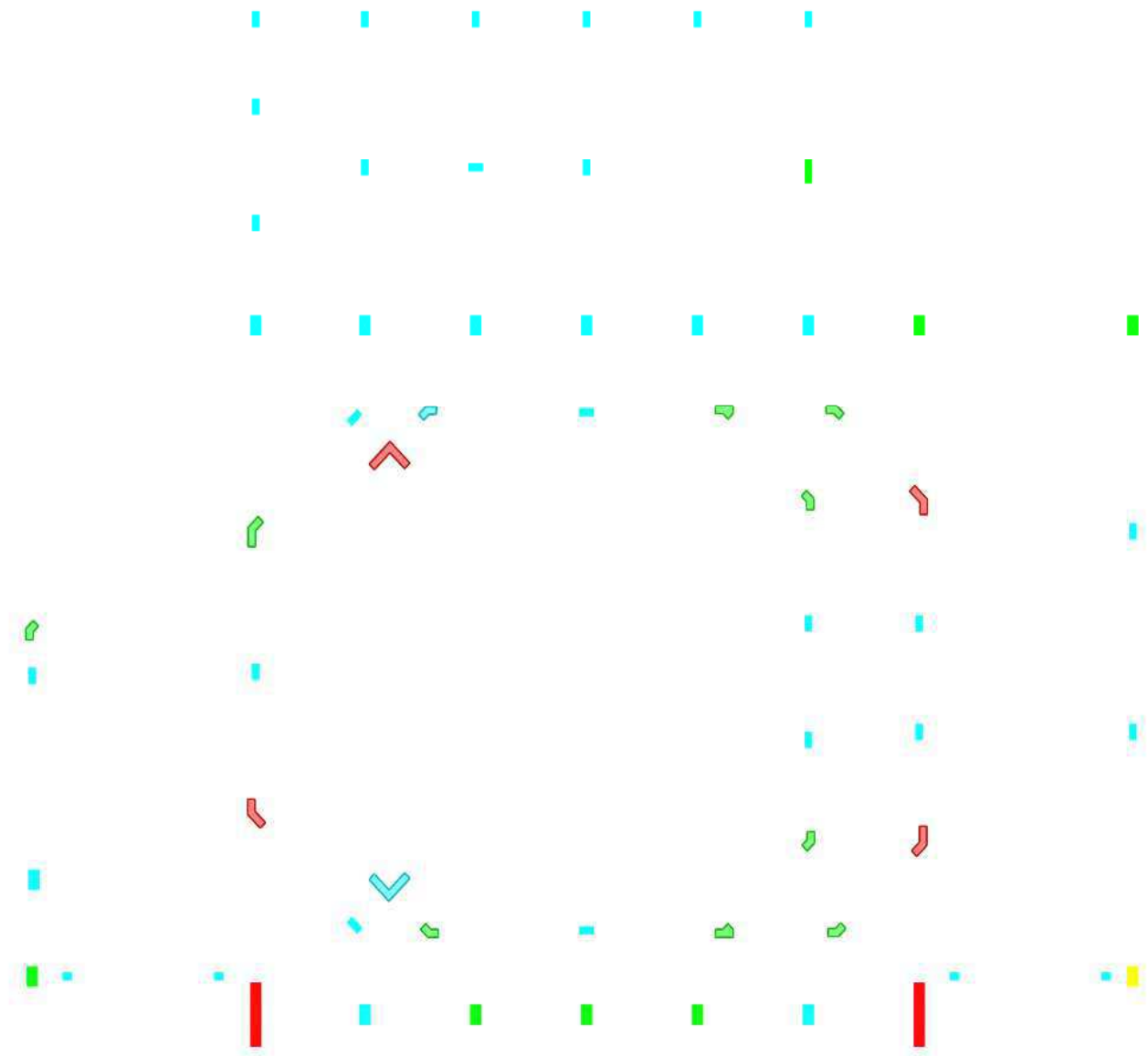


Figure 6-4 Column PMM Ratio at Ground Floor

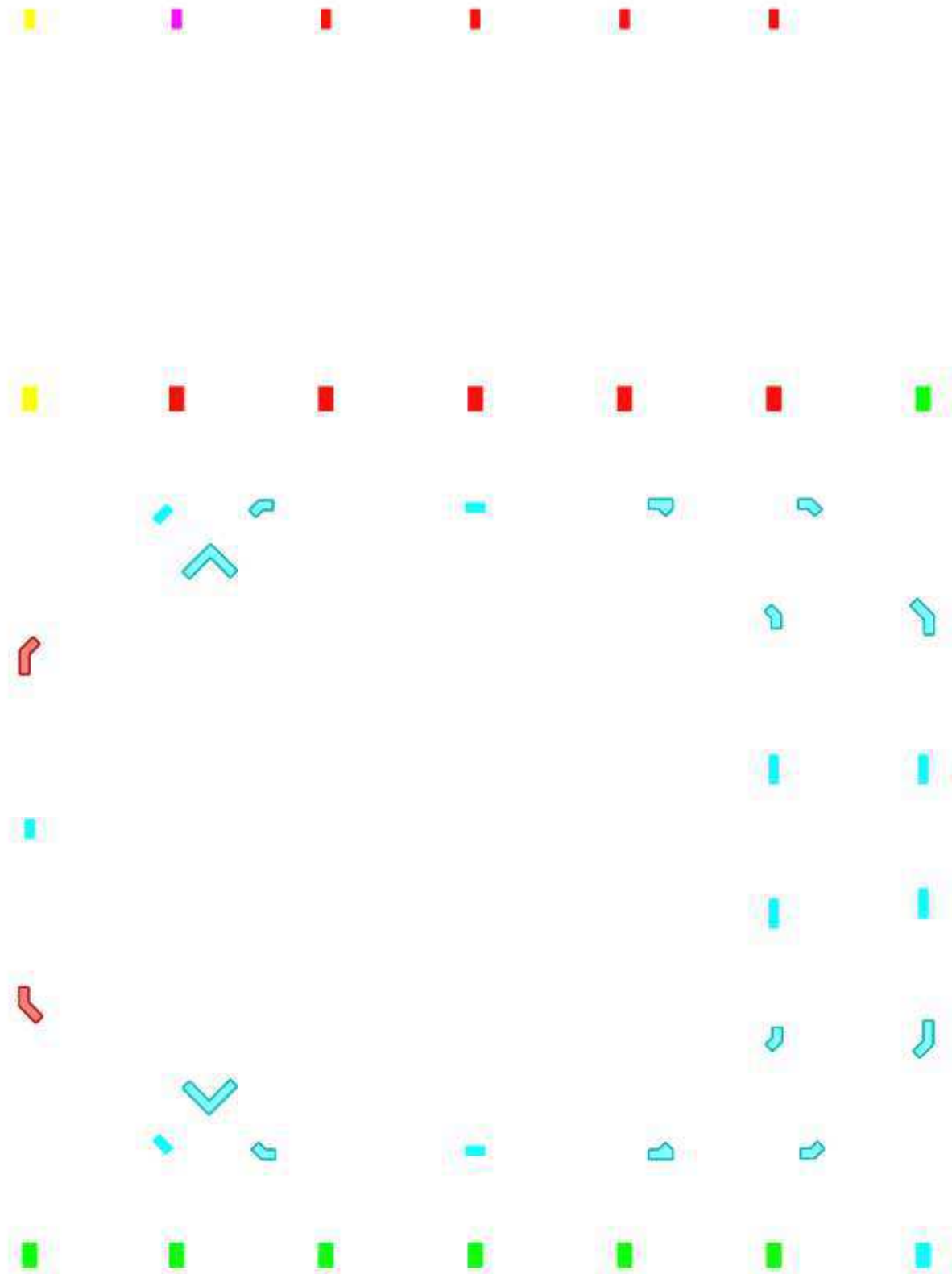


Figure 6-5 Column PMM Ratio at ROOF Floor



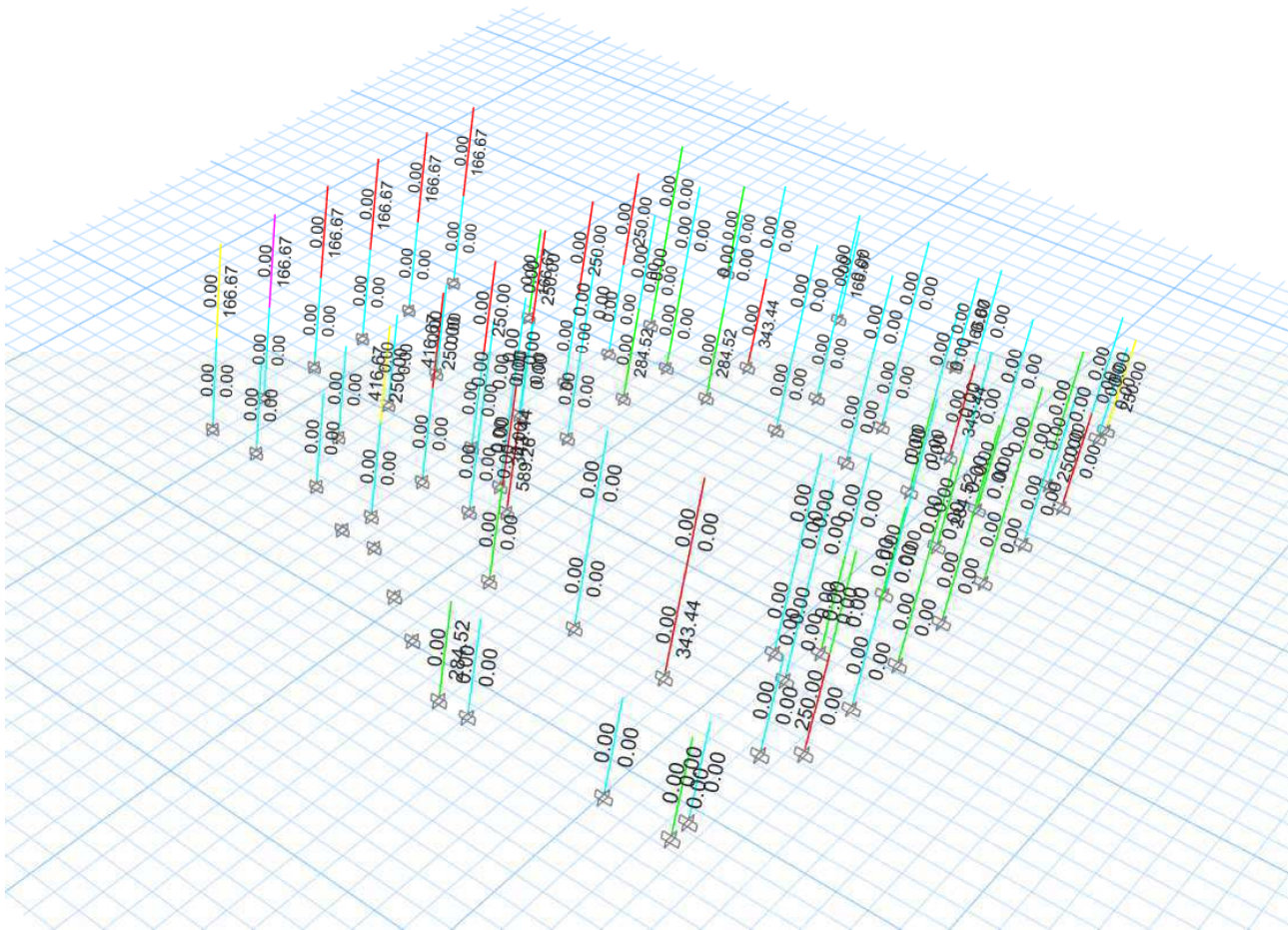


Figure 6-6 Shear Reinforcing

## II. Shear Wall Pier Design

Uniform Reinforcing pier section is specified for each wall pier to be checked, the program creates an interaction surface for that pier and uses that interaction surface to determine the critical flexural demand/capacity ratio for the pier. The wall pier demand/capacity ratio is a factor that gives an indication of the stress condition of the wall with respect to the capacity of the wall.

For pier shear reinforcing, the pier horizontal shear reinforcement requirements reported by the program are based purely on shear strength considerations. The minimum shear rebar requirements to satisfy spacing consideration is checked independently. The provided shear reinforcement shall satisfy the required shear reinforcement reported by the program.

Based on the results, the existing vertical reinforcement (T12-200) and horizontal reinforcement (T8-200) are sufficient. Therefore, wall pier design is adequate.

The Pier D/S ratio and pier shear reinforcing are shown from Figure 6-4



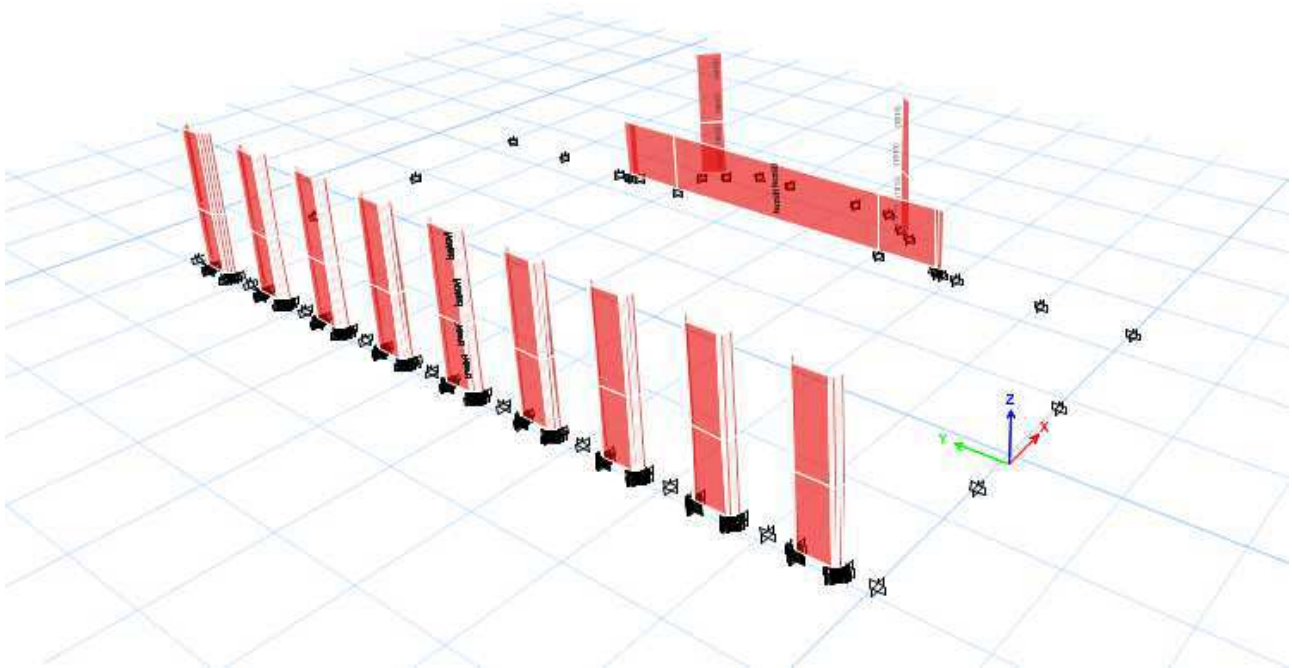


Figure 6-7 Pier D/C Ratio

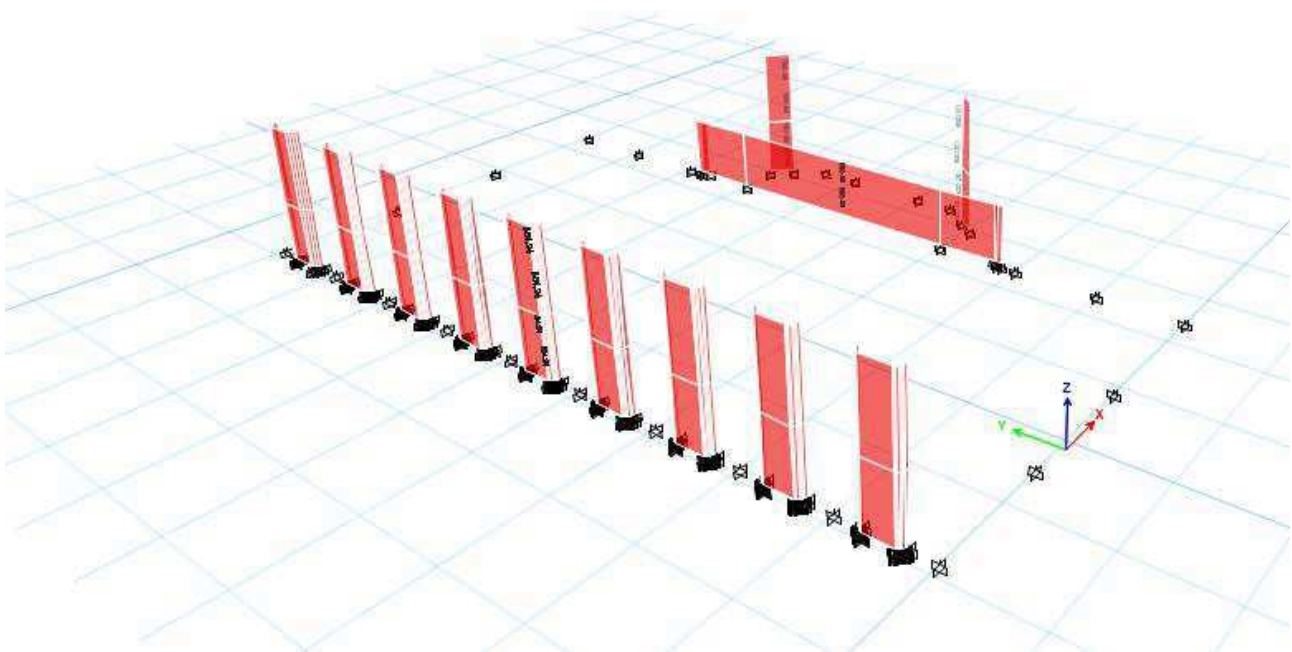


Figure 6-8 Pier Shear Reinforcing

### 6.1.3 SWIMMING PART



Figure 6-9 Column PMM Ratio at Ground Floor

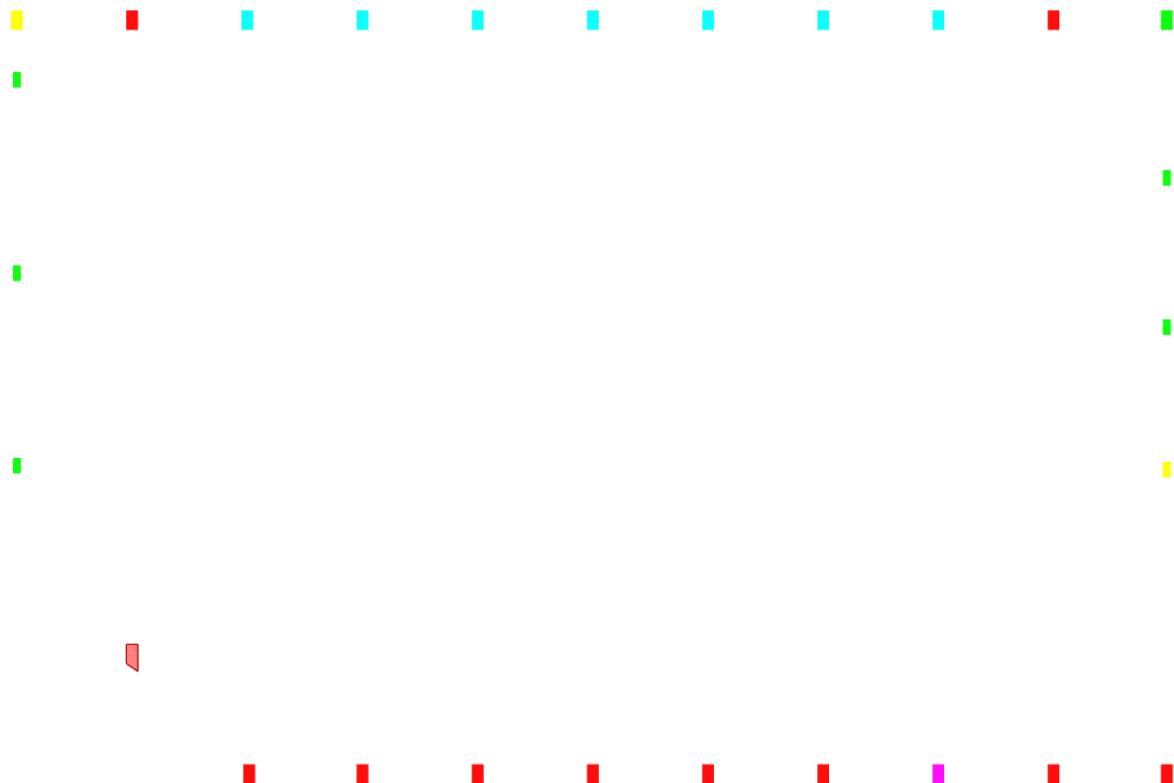


Figure 6-10 Column PMM Ratio at ROOF Floor

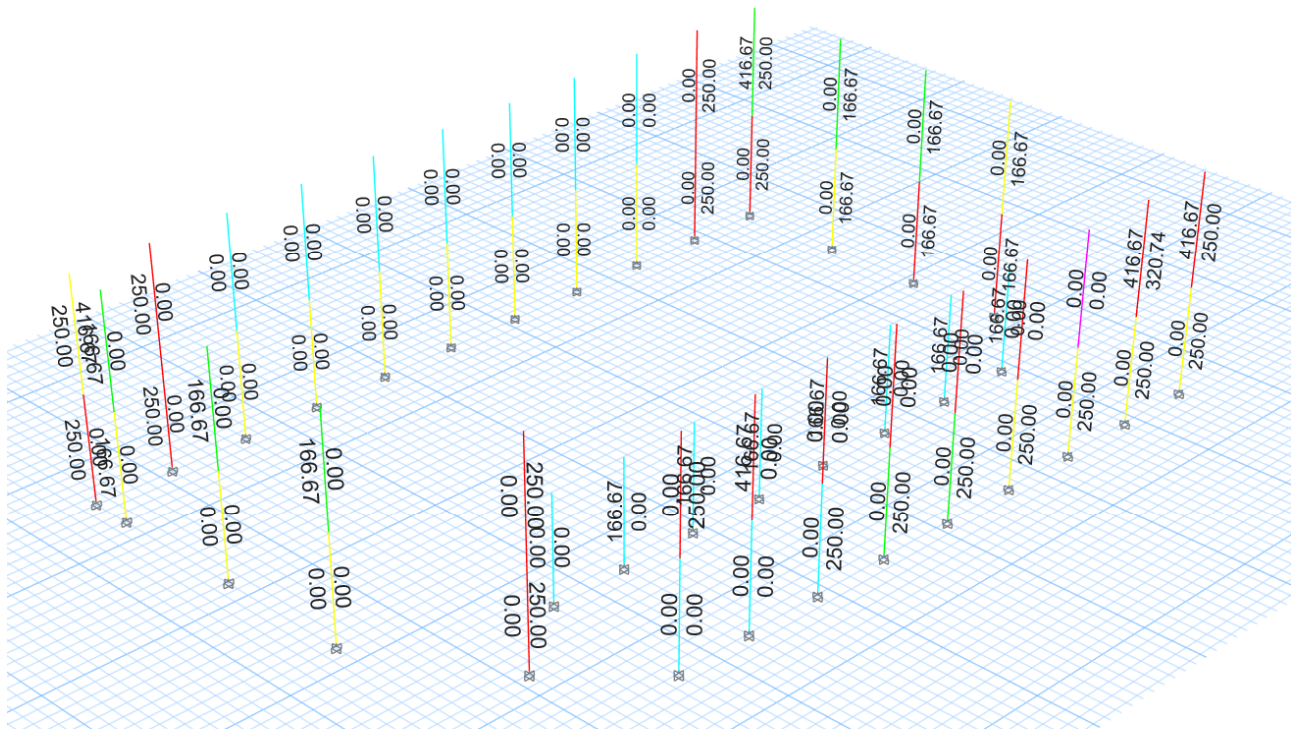


Figure 6-11 Shear Reinforcing

### III. Shear Wall Pier Design

Uniform Reinforcing pier section is specified for each wall pier to be checked, the program creates an interaction surface for that pier and uses that interaction surface to determine the critical flexural demand/capacity ratio for the pier. The wall pier demand/capacity ratio is a factor that gives an indication of the stress condition of the wall with respect to the capacity of the wall.

For pier shear reinforcing, the pier horizontal shear reinforcement requirements reported by the program are based purely on shear strength considerations. The minimum shear rebar requirements to satisfy spacing consideration is checked independently. The provided shear reinforcement shall satisfy the required shear reinforcement reported by the program.

Based on the results, the existing vertical reinforcement (T12-200) and horizontal reinforcement (T8-200) are sufficient. Therefore, wall pier design is adequate.

The Pier D/S ratio and pier shear reinforcing are shown from Figure 6-12

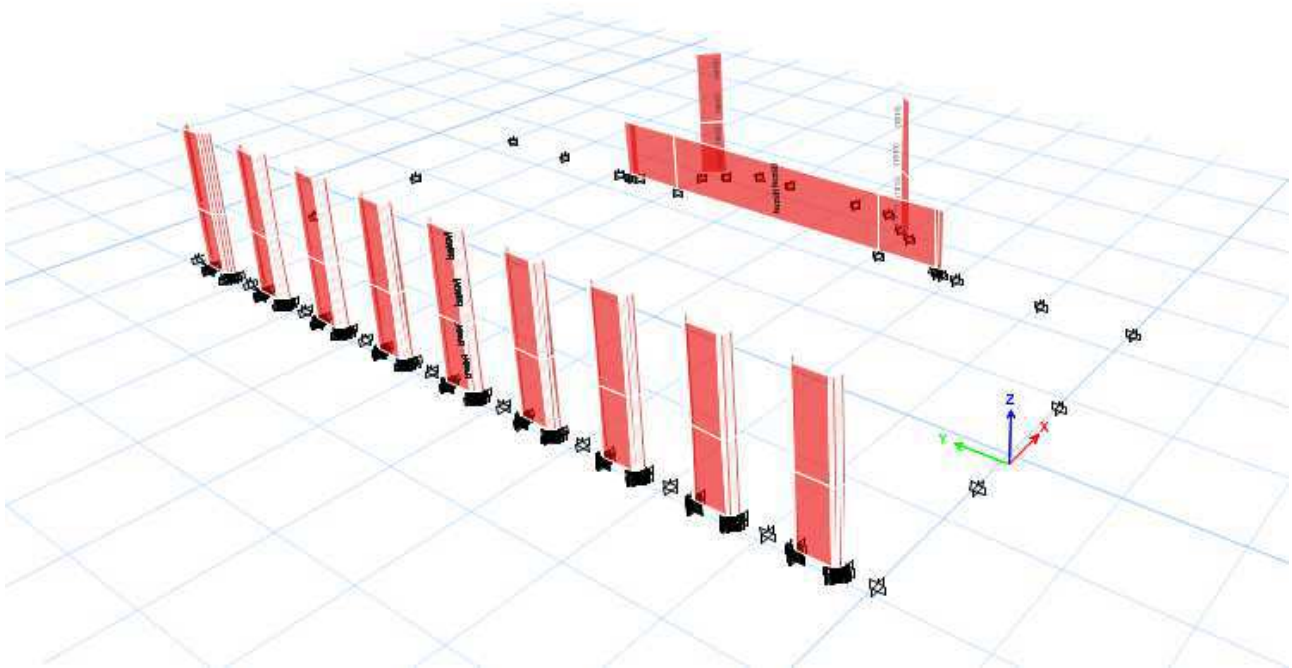


Figure 6-12 Pier D/C Ratio at gymnasium part

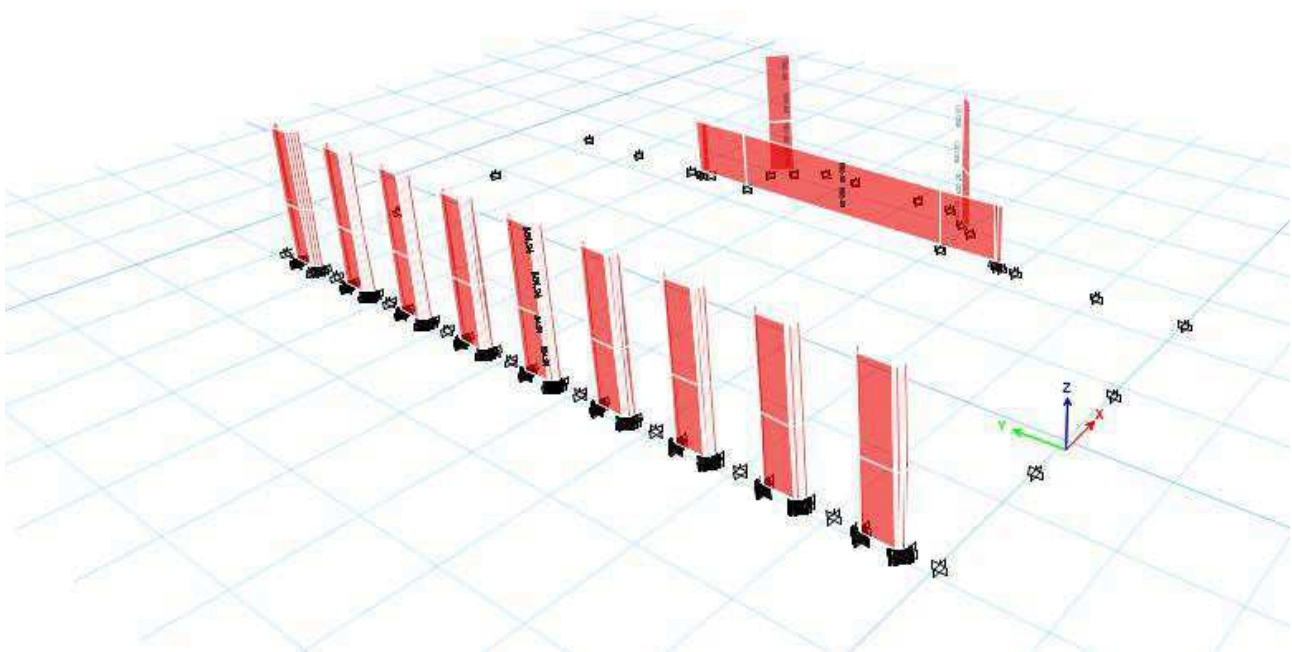


Figure 6-13 Pier Shear Reinforcing at gymnasium part



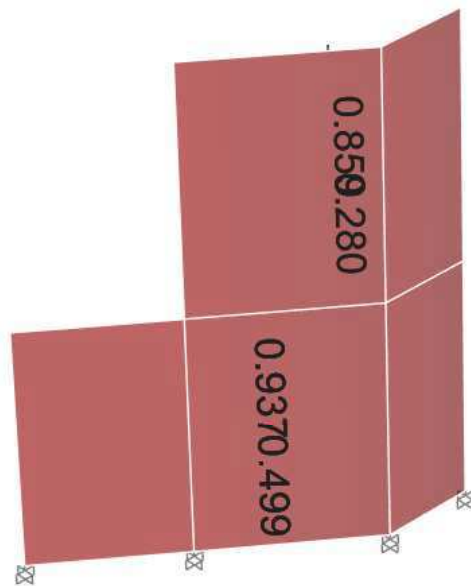


Figure 6-14 Pier D/C Ratio at Auditorium part

loads  
named Output Items  
named Plots

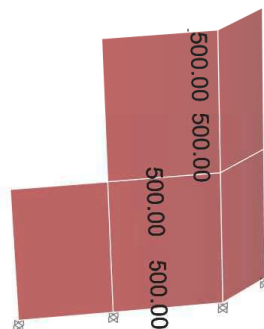
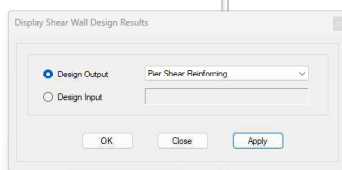


Figure 6-15 Pier Shear Reinforcing at Auditorium part

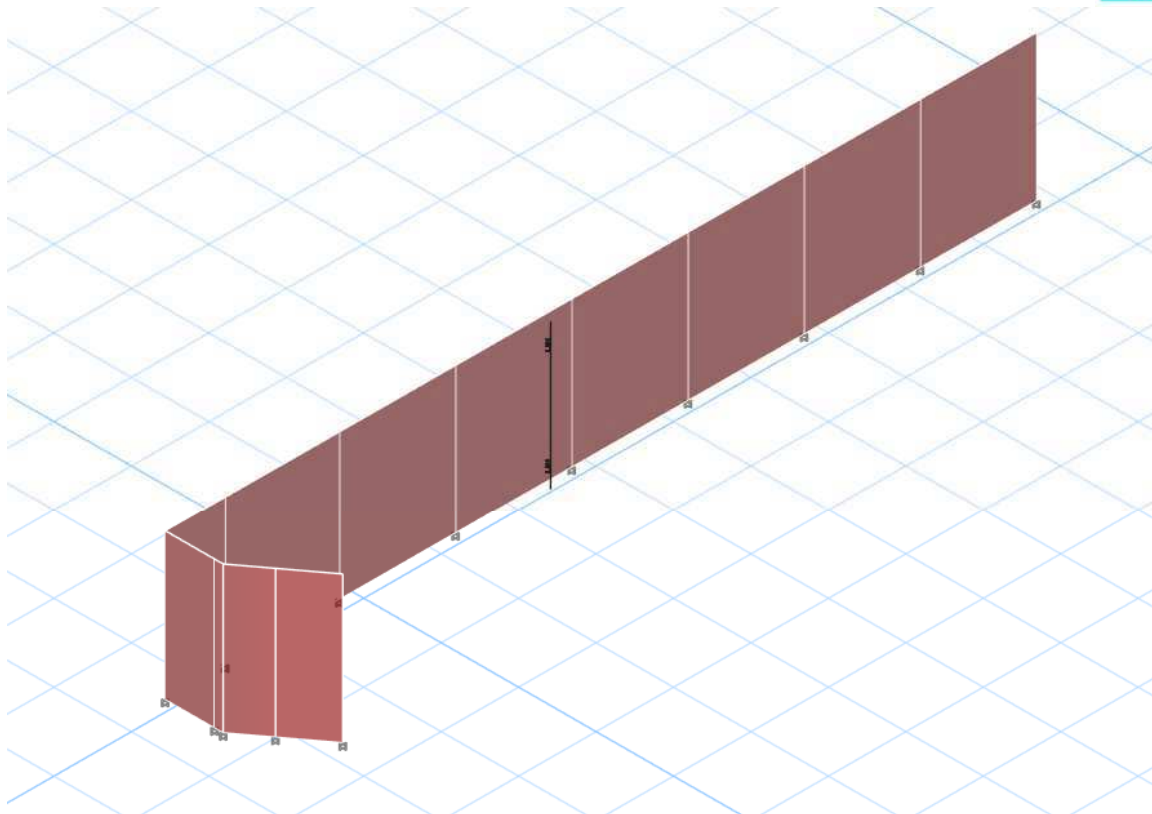


Figure 6-16 Pier D/C Ratio at Swimming Part

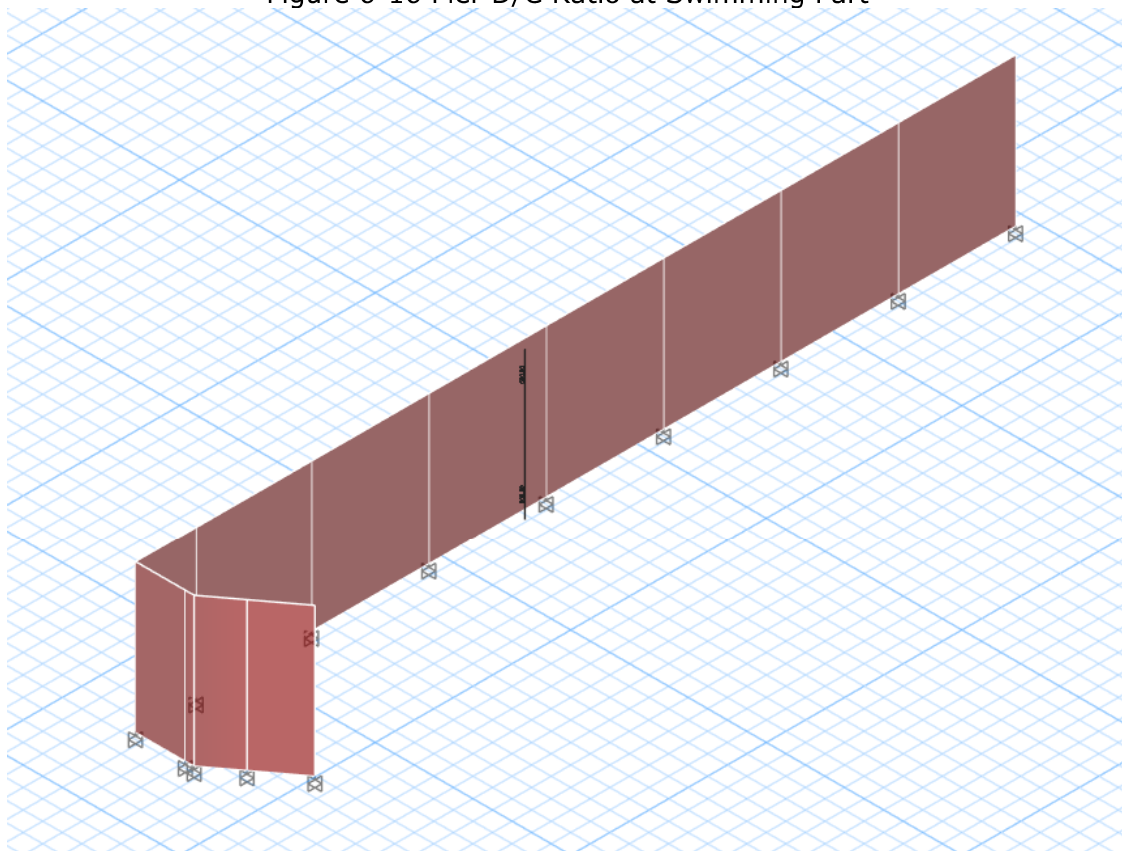


Figure 6-17 Pier Shear Reinforcing at Swimming Part

## 6.2 SAFE Design Result (Slabs and Beams)

### 6.2.1 Slab Design

The slab design has been carried out by checking the long-term deflection, Crack width and flexure. The Ground floor slab, Typical Floor slab and Roof floor slab are exported by ETABS and analyze using SAFE software.

Long term deflection is checked with the allowable deflection limit of  $L/240$ . The long-term deflection for each floor is found to be within the allowable limit.

For flexure, the slab design procedure involves defining sets of strips in two directions. The locations of the strips are based on the location of the rib slab. The moments for a particular strip are recovered from the analysis, and a flexural design is carried out based on the ultimate strength design method (ACI 318-14) for reinforced concrete as described in the following sections. The slab design moments were integrated across design strips and design the required reinforcement. The required flexural reinforcement of the existing slab is calculated and checked with the provided reinforcement (refer to Approved structural drawing nos. AD/23/76/S101 and AD/23/76/S09). The flexural reinforcement for each design strip is checked and found to be sufficient. Therefore, the slab design is adequate.

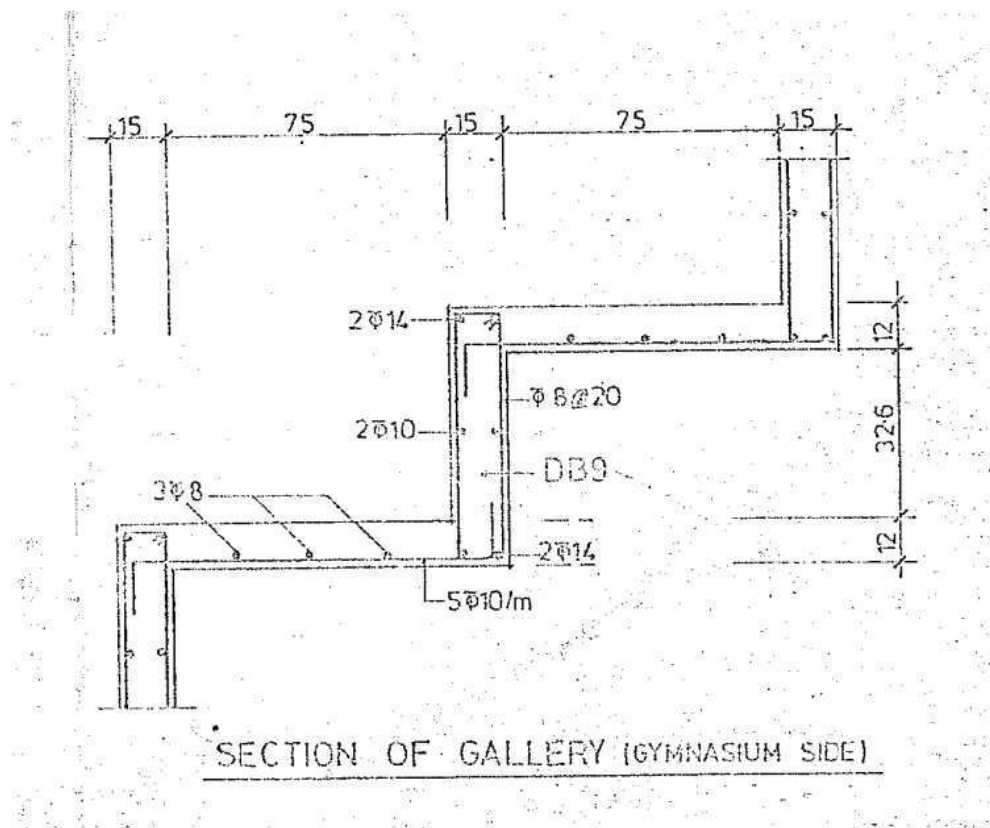


Figure 6-18 Reinforcement at Ground Floor Level as per as-built drawings refer to **APPENDIX B**



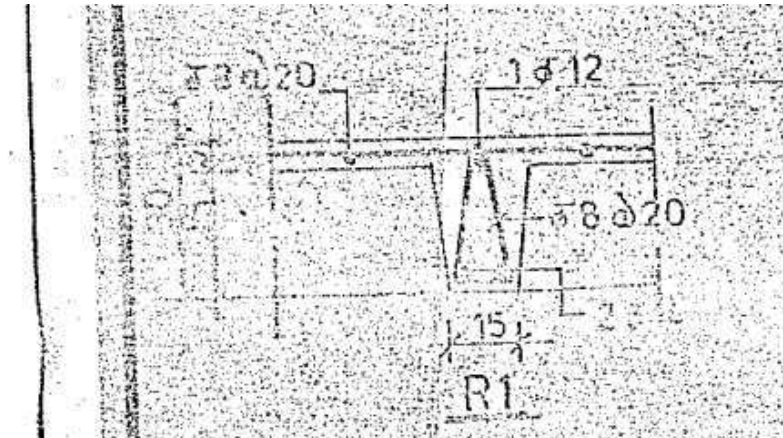


Figure 6-19 Reinforcement at Roof Floor Level for Ribbed slab-(R1) as per as-built drawings refer to **APPENDIX B**

### 6.2.1.1 Gymnasium part

#### A. Ground Floor

The computed maximum deflection at Ground Floor is summarized below.

Label	Length, m	Allowable deflection, mm	Computed deflection, mm	Remarks
1	5.0	20.83	11.5	Computed Deflection is within the allowable deflection

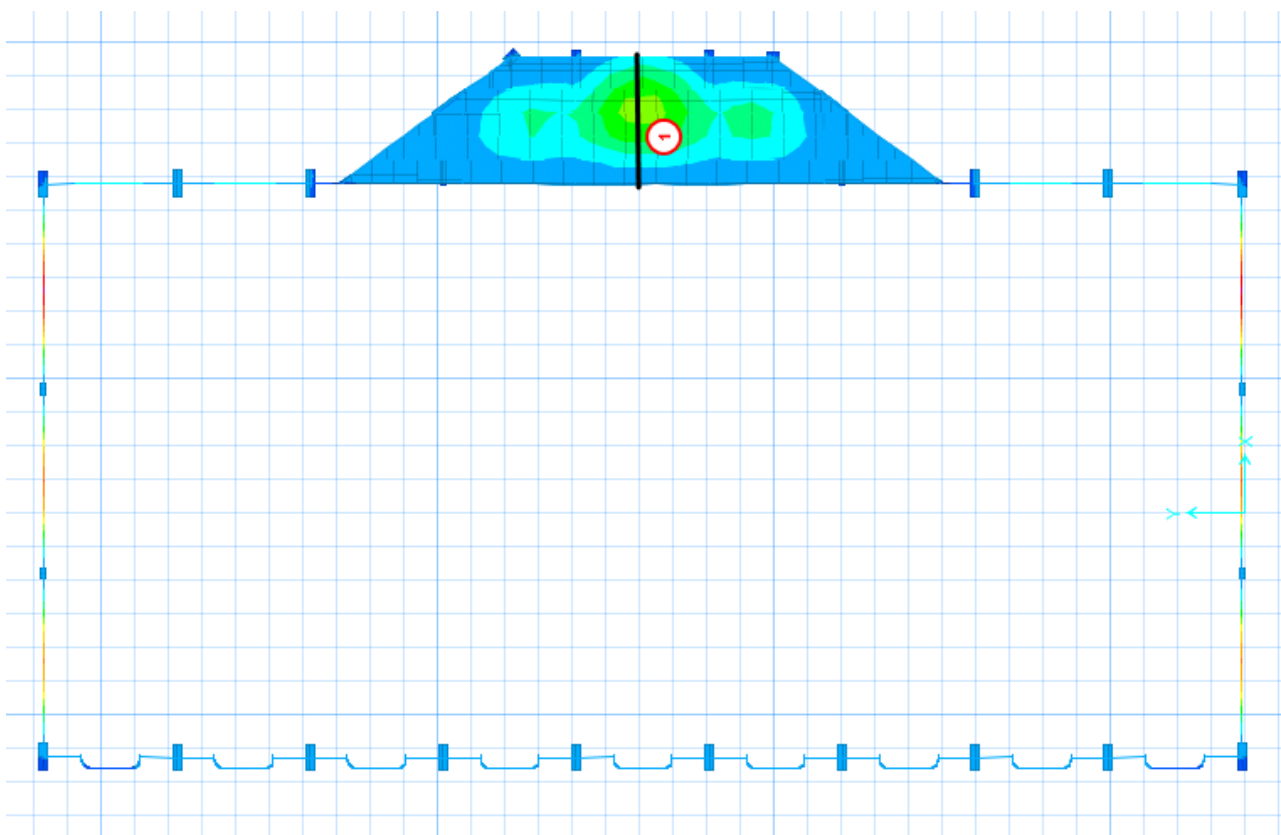


Figure 6-20 Long-Term Deflection at Ground Floor Level

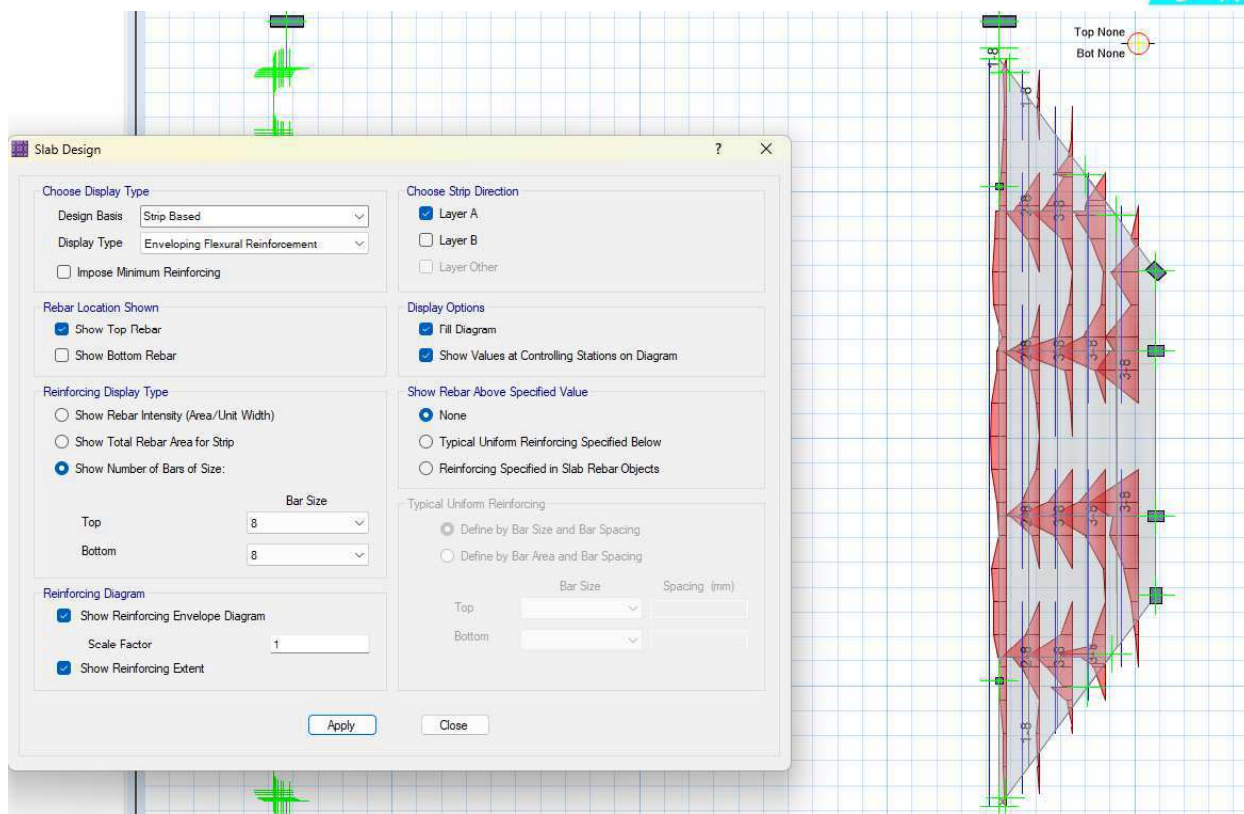


Figure 6-21 Slab Strip Design – Layer A – Top Reinforcing –3T8 mesh at Ground Floor Level as per as-built Drawings refer to **Appendix B**

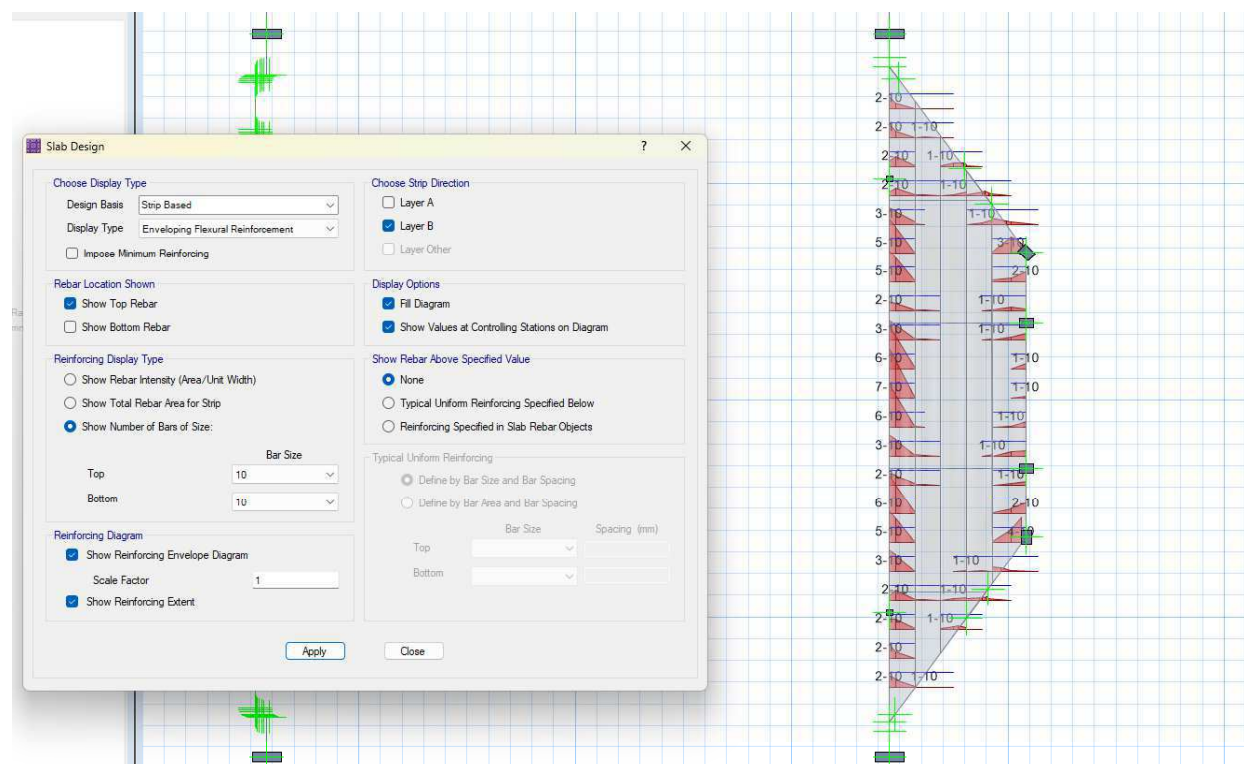


Figure 6-22 Slab Strip Design – Layer B – Top Reinforcing –5T10-mesh at Ground Floor Level as per as-built Drawings refer to **Appendix B**

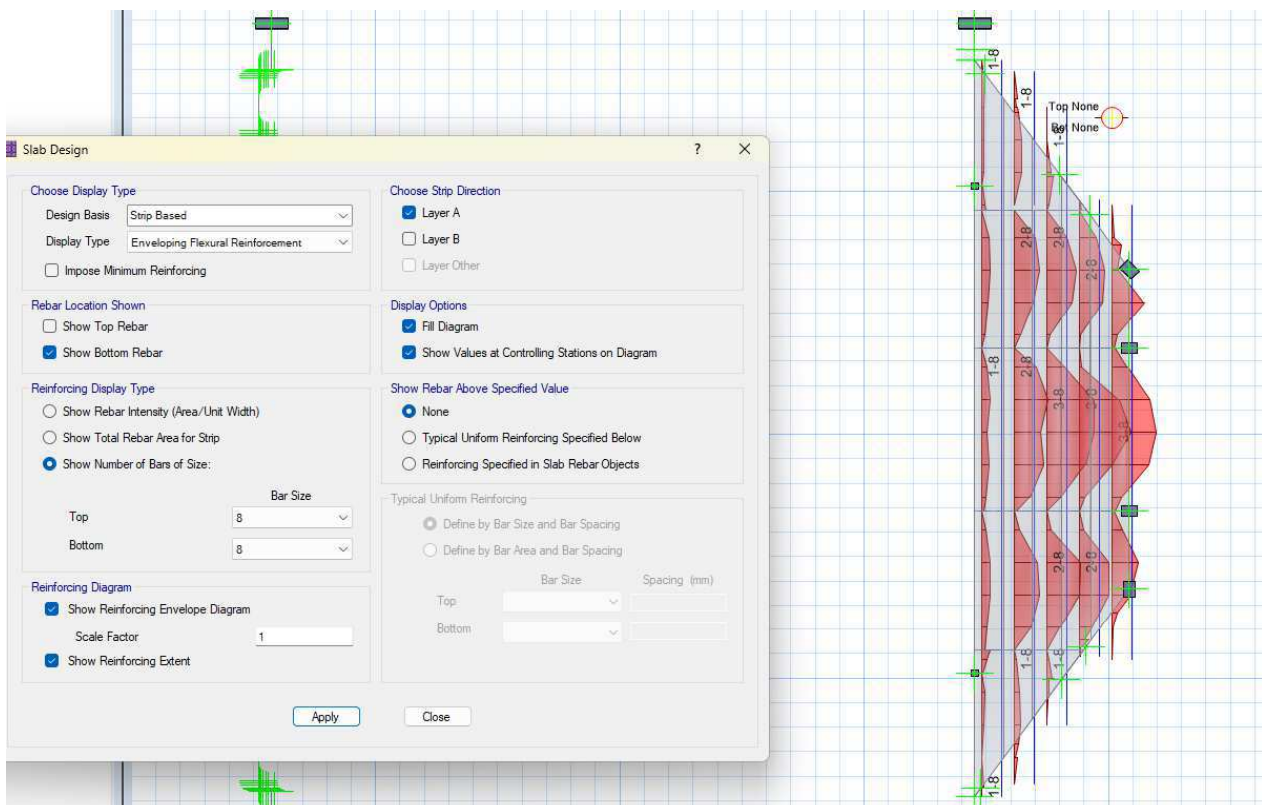


Figure 6-23 Slab Strip Design – Layer A – Top Reinforcing –3T8 mesh –Bottom Reinforcing at ground Floor Level as per as-built Drawings refer to **Appendix B**

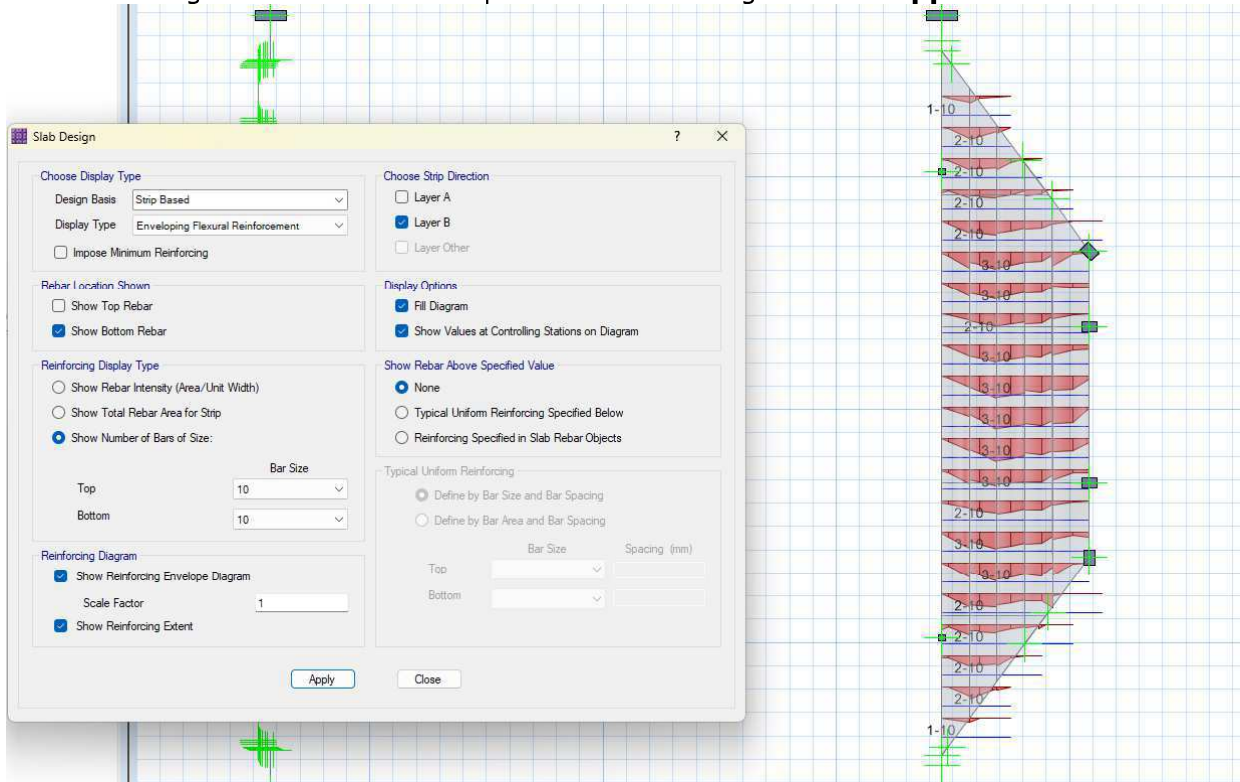


Figure 6-24 Slab Strip Design – Layer B –5T10 mesh –Bottom Reinforcing at ground Floor Level as per as-built Drawings refer to **Appendix B**

## B. Roof Floor

The computed maximum deflection at Roof Floor is summarized below.

Label	Length, m	Allowable deflection, mm	Computed deflection, mm	Remarks
1	5.00	20.1	7.8	Computed Deflection is within the allowable deflection

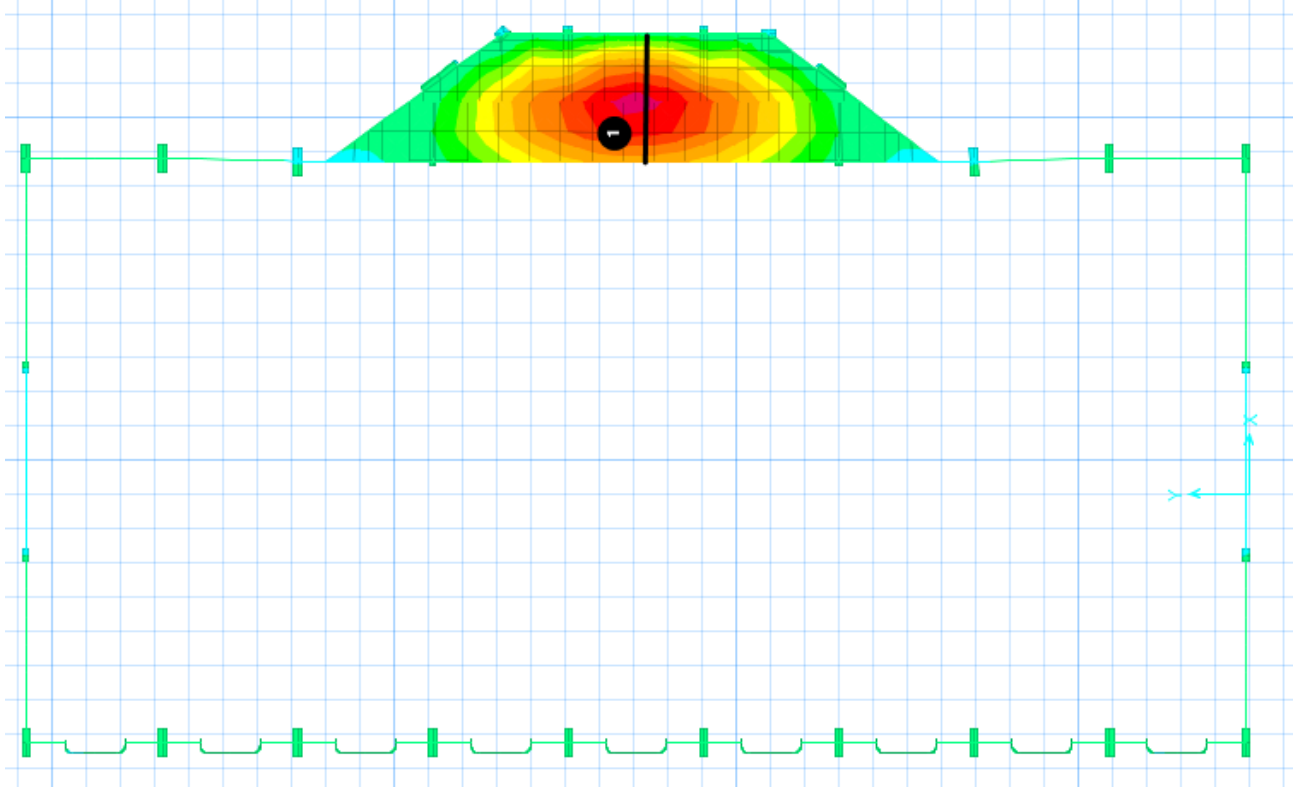


Figure 6-25 Long-Term Deflection at Roof Floor Level



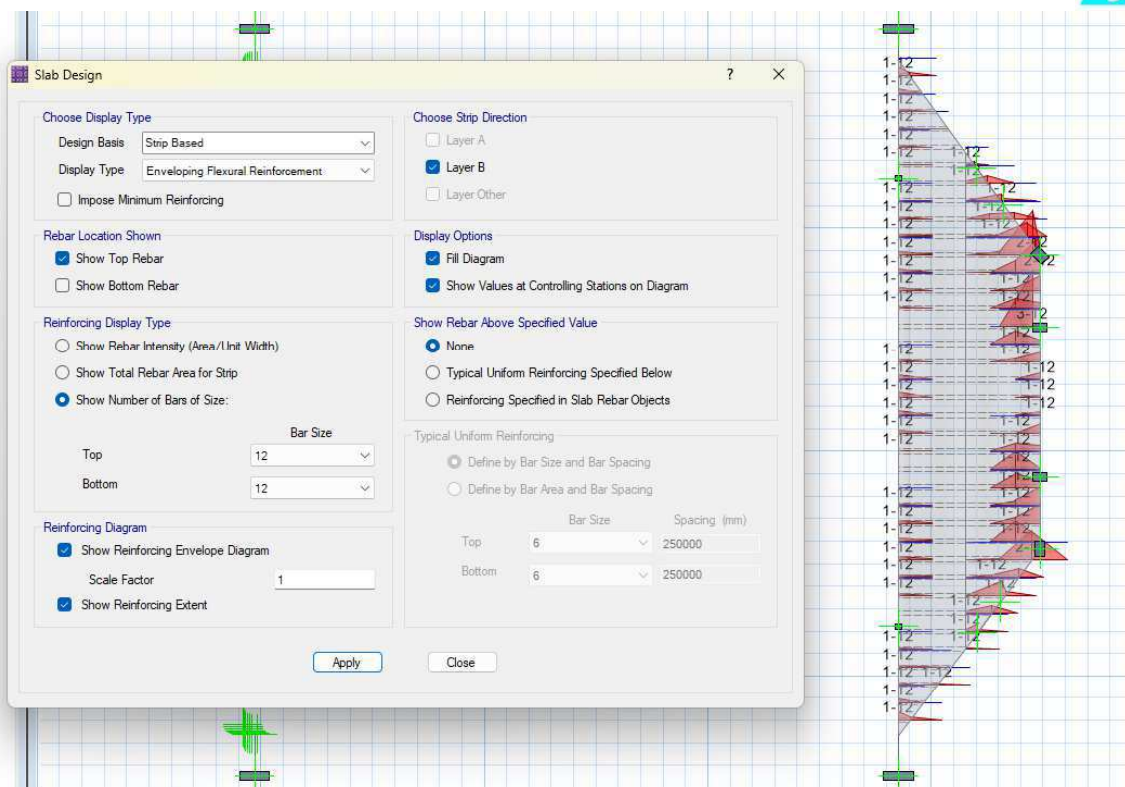


Figure 6-26 Slab Strip Design – Layer A – Top Reinforcing – Roof Floor Level

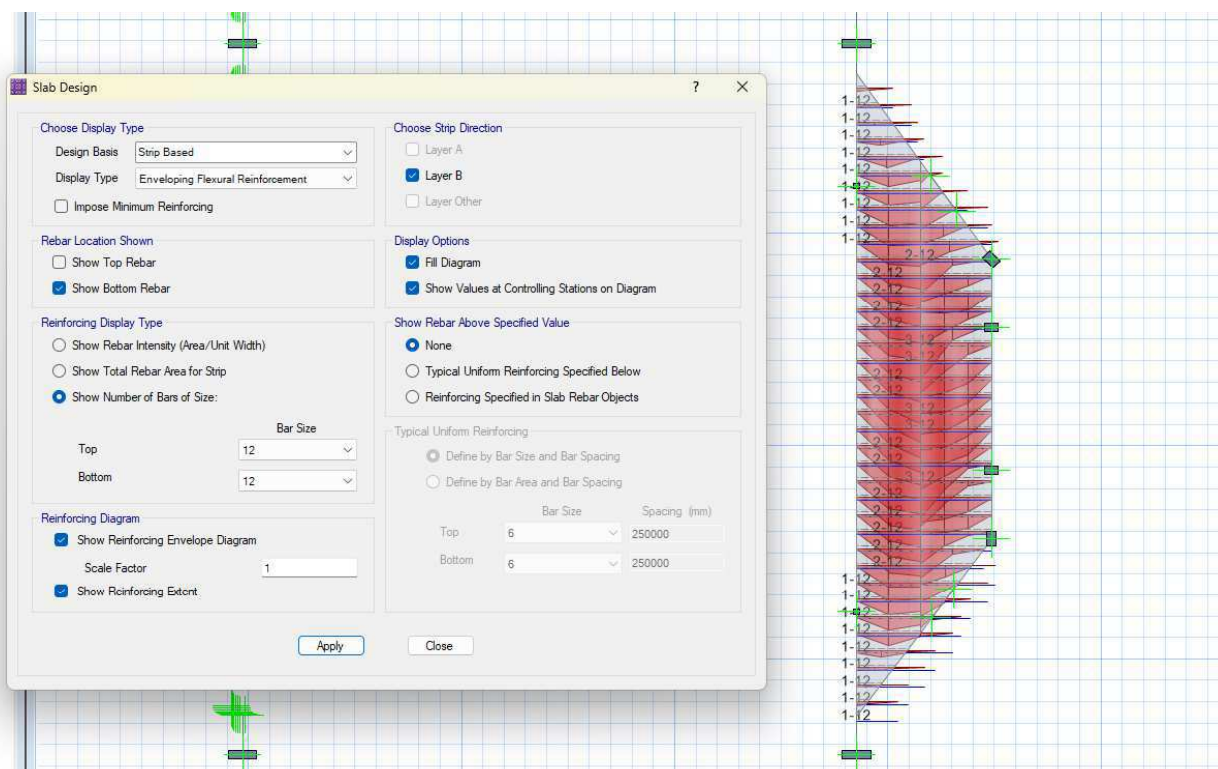


Figure 6-27 Slab Strip Design – Layer A –Bottom Reinforcing at Roof Floor Level

### 6.2.1.2 Auditorium part

#### A. Ground Floor

The computed maximum deflection at Ground Floor is summarized below.

Label	Length, m	Allowable deflection, mm	Computed deflection, mm	Remarks
1	6.0	25.00	19.51	Computed Deflection is within the allowable deflection

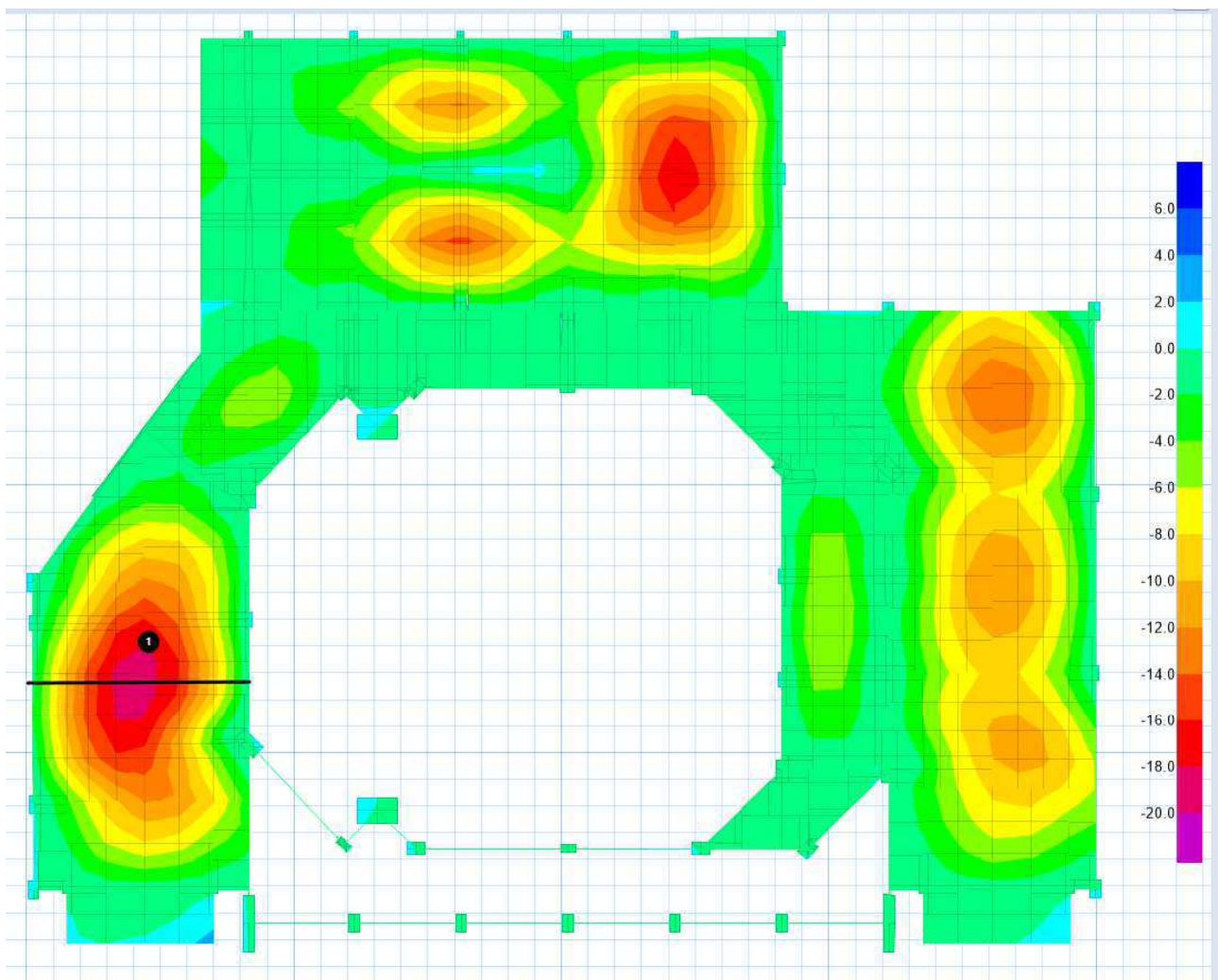


Figure 6-28 Long-Term Deflection at Ground Floor Level



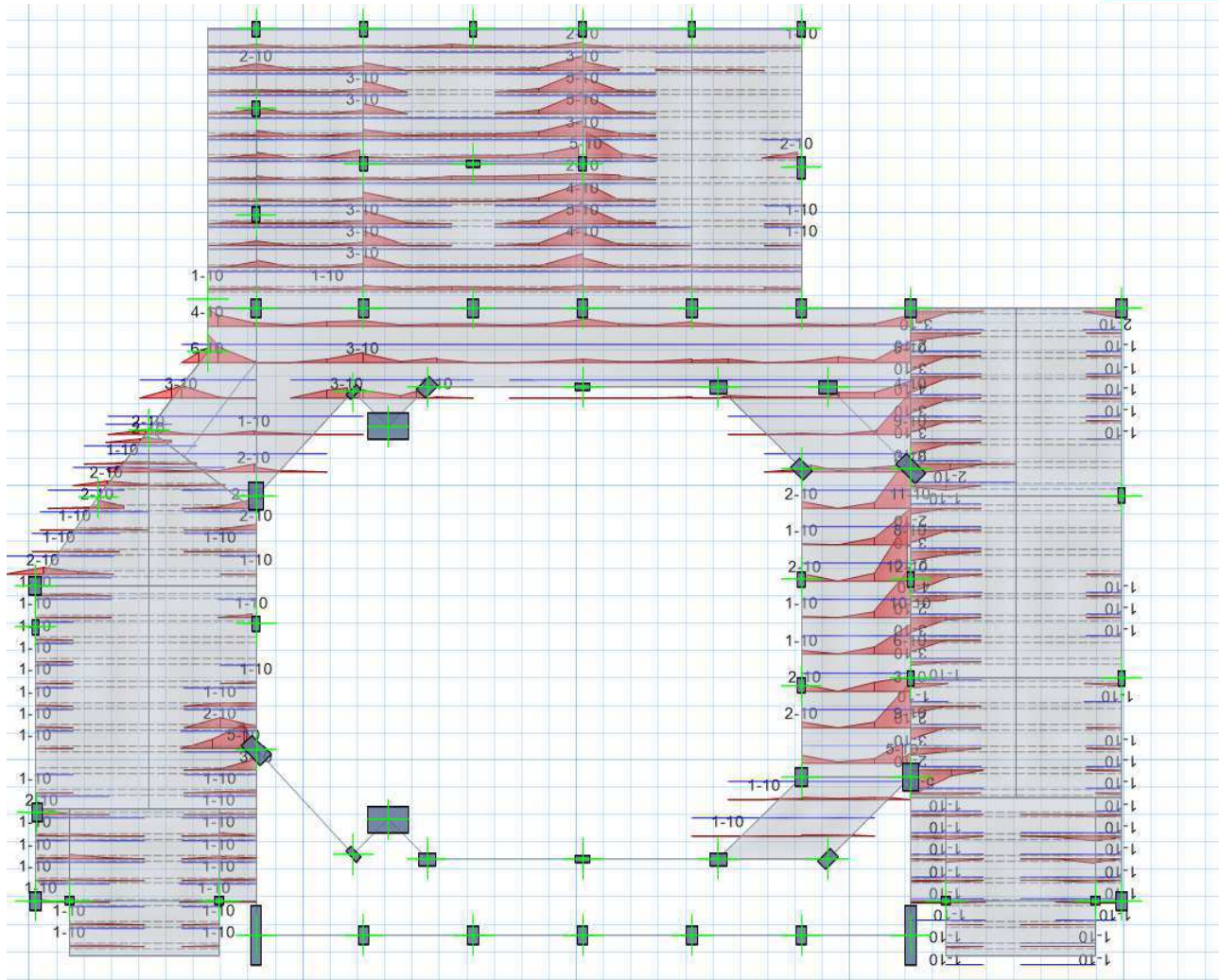


Figure 6-29 Slab Strip Design – Layer A – Top Reinforcing –5T8 for Ribs and 5T10 mesh for solid slab at GROUND Floor Level as per as-built Drawings refer to **Appendix B**

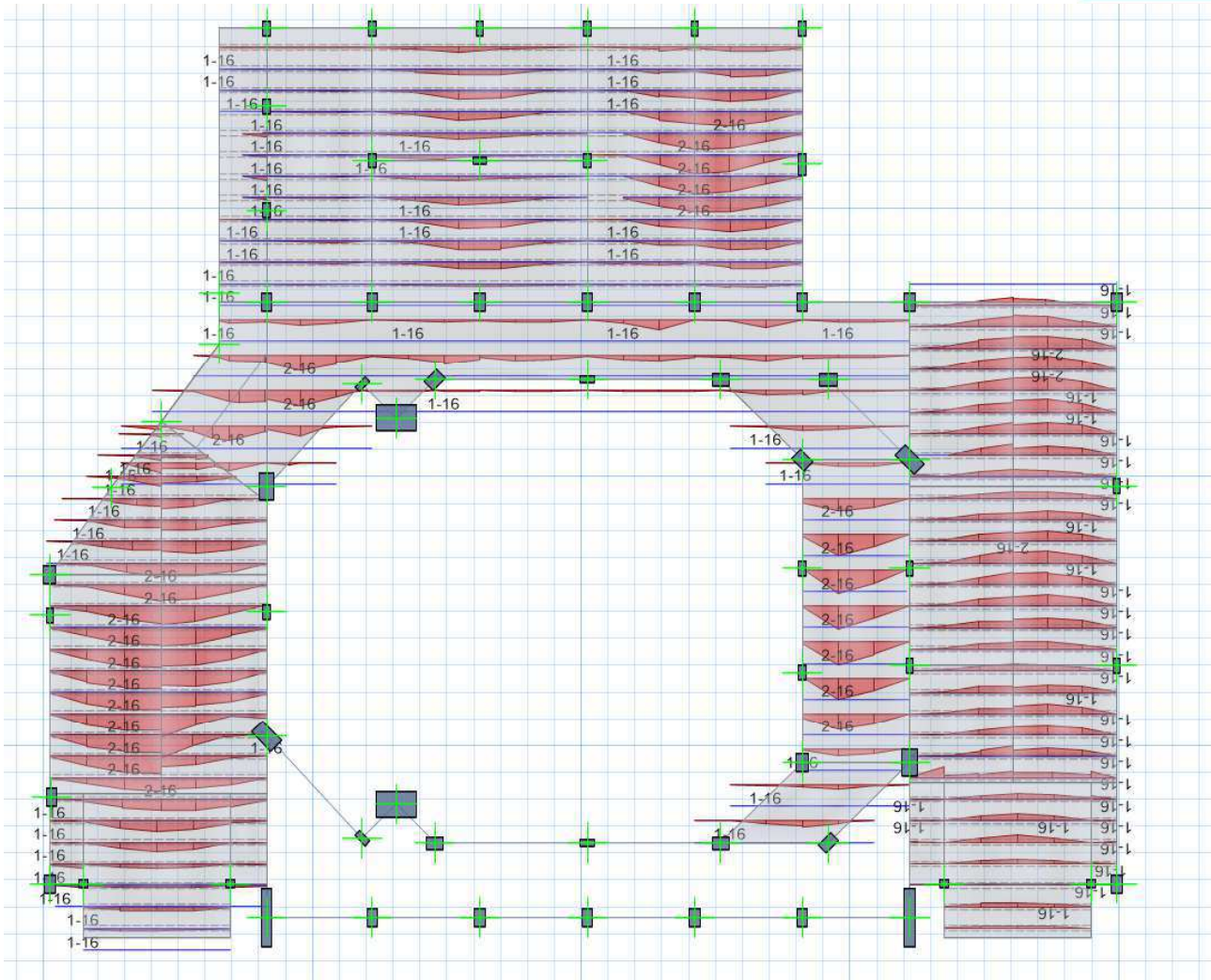


Figure 6-30 Slab Strip Design – Layer A – bottom Reinforcing –2T16-mesh at ground Floor Level as per as-built Drawings refer to **Appendix B**



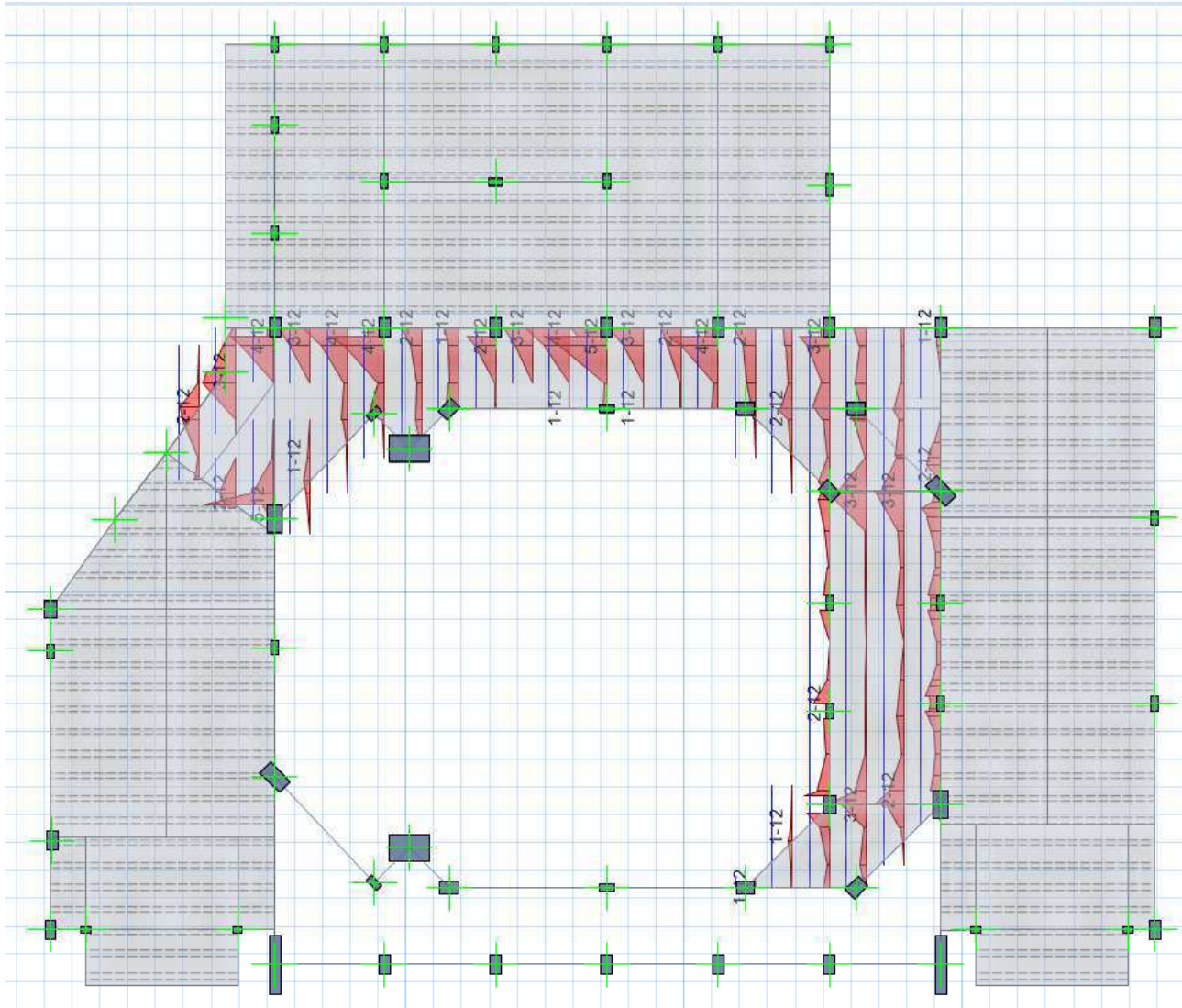


Figure 6-31 Slab Strip Design – Layer B – Top Reinforcing –6T12 mesh –Bottom Reinforcing at ground Floor Level as per as-built Drawings refer to **Appendix B**

Plotting Flexural)

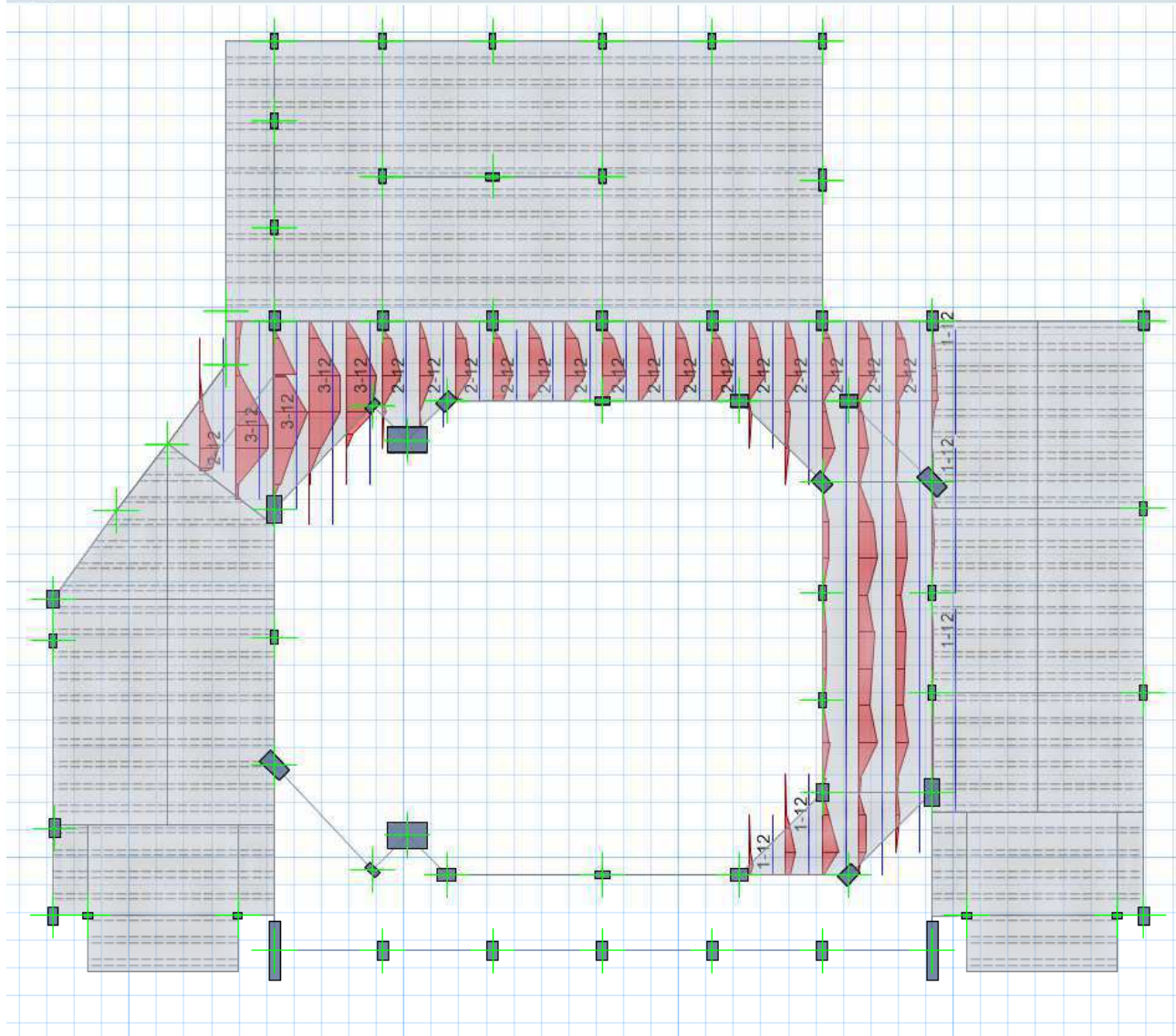


Figure 6-32 Slab Strip Design – Layer B –6T12 mesh –Bottom Reinforcing at Ground Floor Level as per as-built Drawings refer to **Appendix B** at Ground Floor Level

## B. Roof Floor

The computed maximum deflection at Roof Floor is summarized below.

Label	Length, m	Allowable deflection, mm	Computed deflection, mm	Remarks
1	7.85	32.7	18.5	Computed Deflection is within the allowable deflection

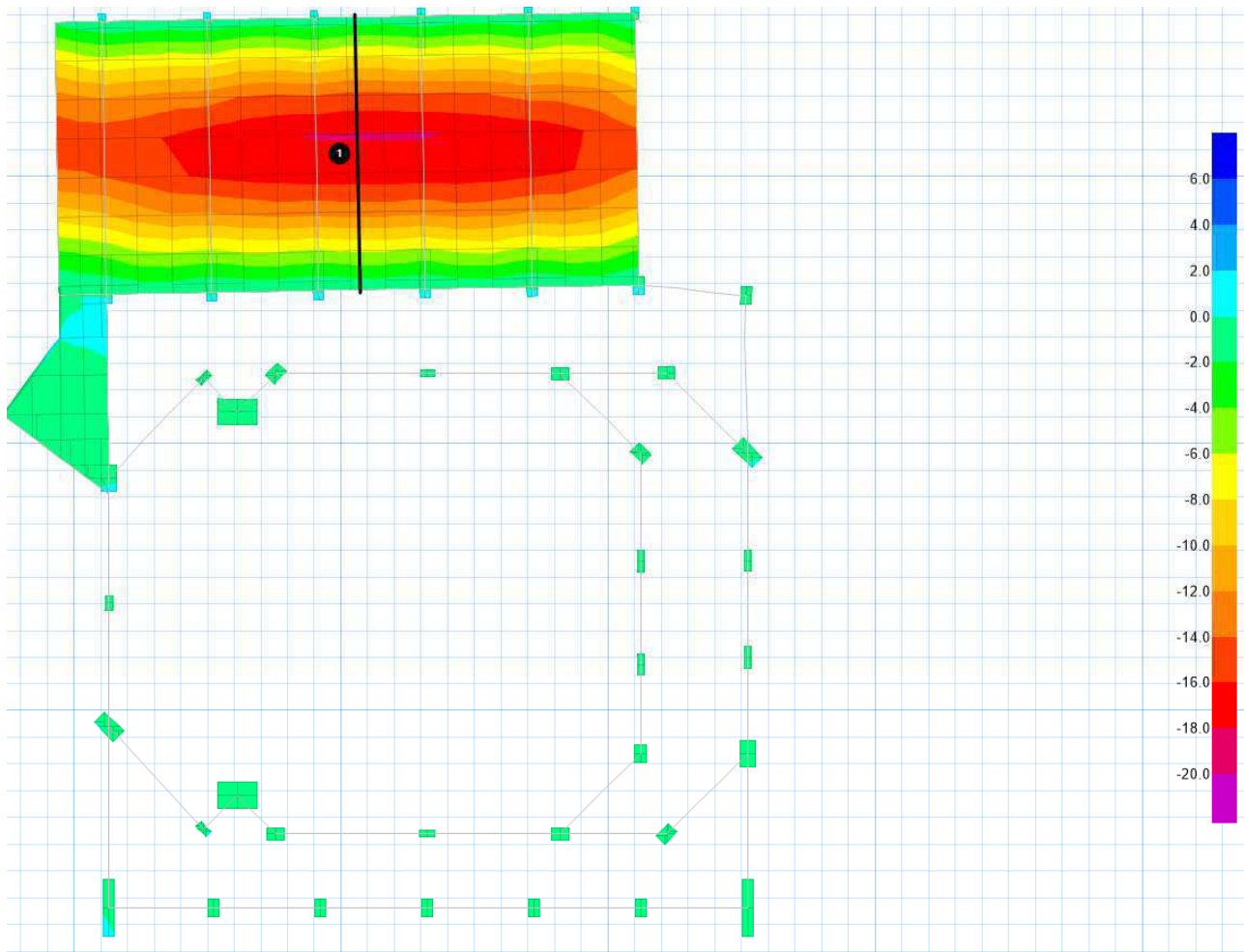


Figure 6-33 Long-Term Deflection at Roof Floor Level



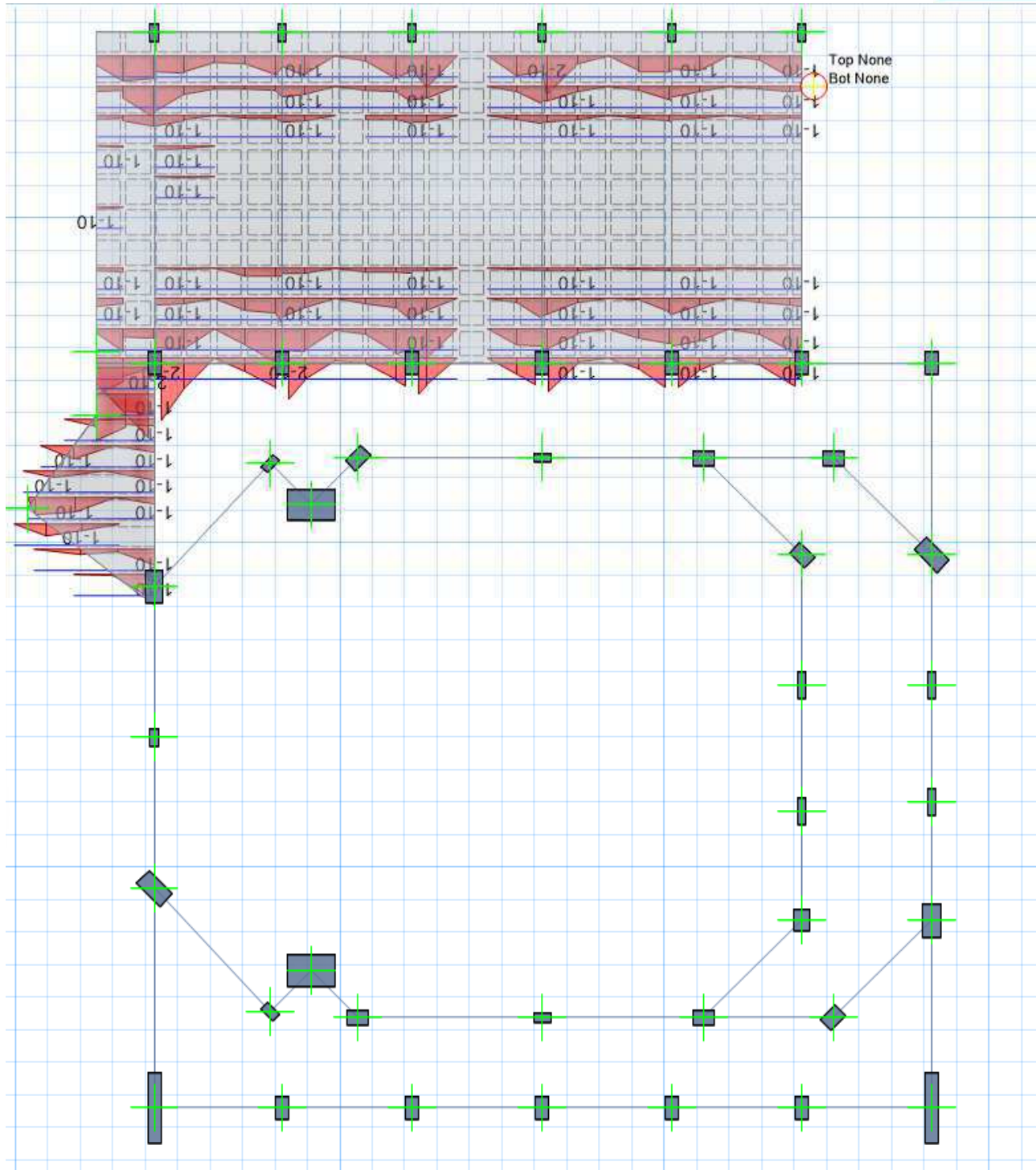


Figure 6-34 Slab Strip Design – Layer A – Top Reinforcing – 5T10 for Ribs at Roof Floor Level as per as-built Drawings refer to **Appendix B**



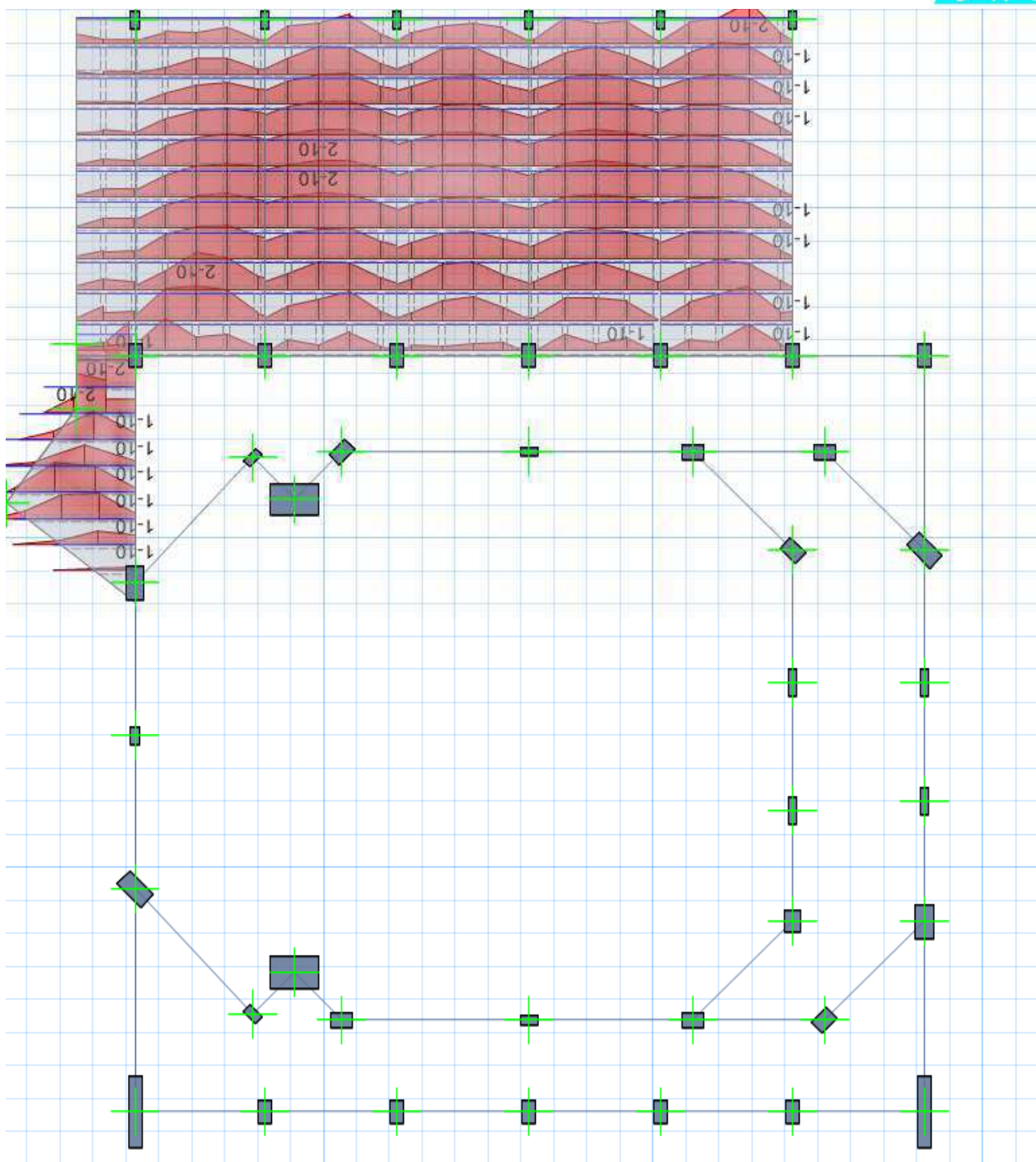


Figure 6-35 Slab Strip Design – Layer A –2T10 for Ribs at Bottom Reinforcing at Roof Floor Level as per as-built Drawings refer to **Appendix B**

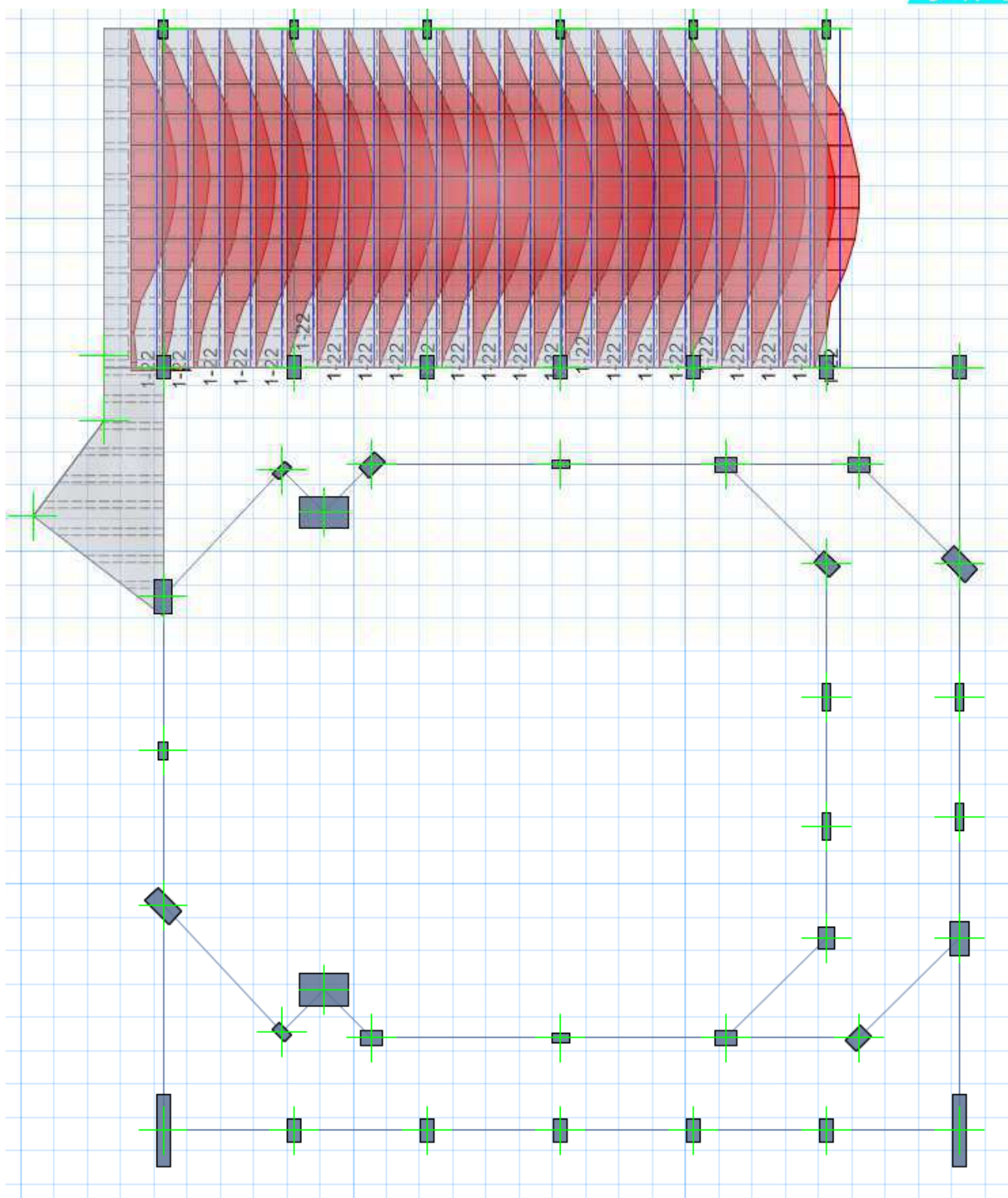


Figure 6-36 Slab Strip Design – Layer B –2T12 for Ribs at Bottom Reinforcing at Roof Floor Level as per as-built Drawings refer to **Appendix B**

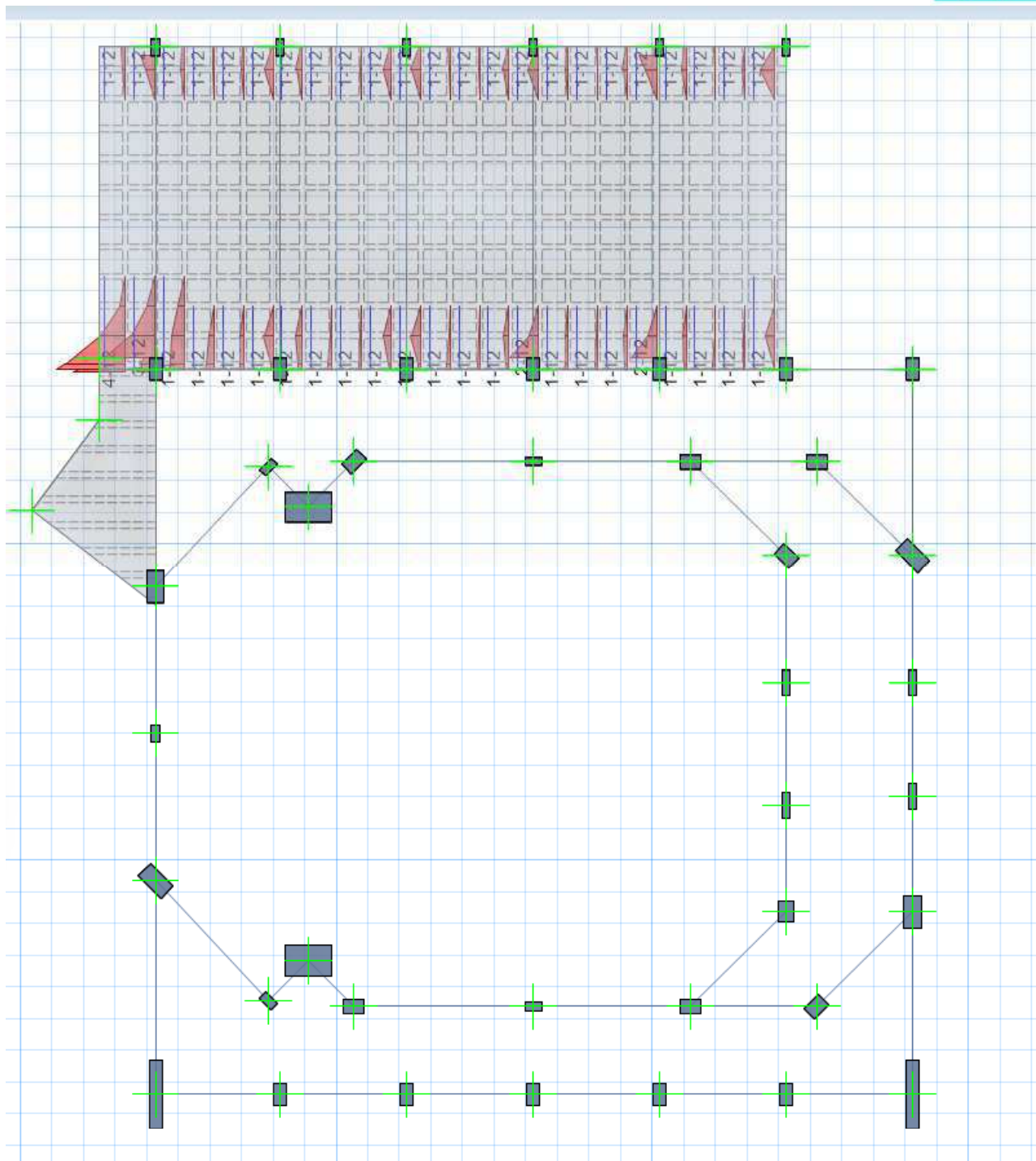


Figure 6-37 Slab Strip Design – Layer A –2T12 for Ribs at Top Reinforcing at Roof Floor Level as per as-built Drawings refer to **Appendix B**

### 6.2.1.3 Swimming Part

#### A. Ground Floor

The computed maximum deflection at Ground Floor is summarized below.

Label	Length, m	Allowable deflection, mm	Computed deflection, mm	Remarks
1	5.0	20.8	6.8	Computed Deflection is within the allowable deflection

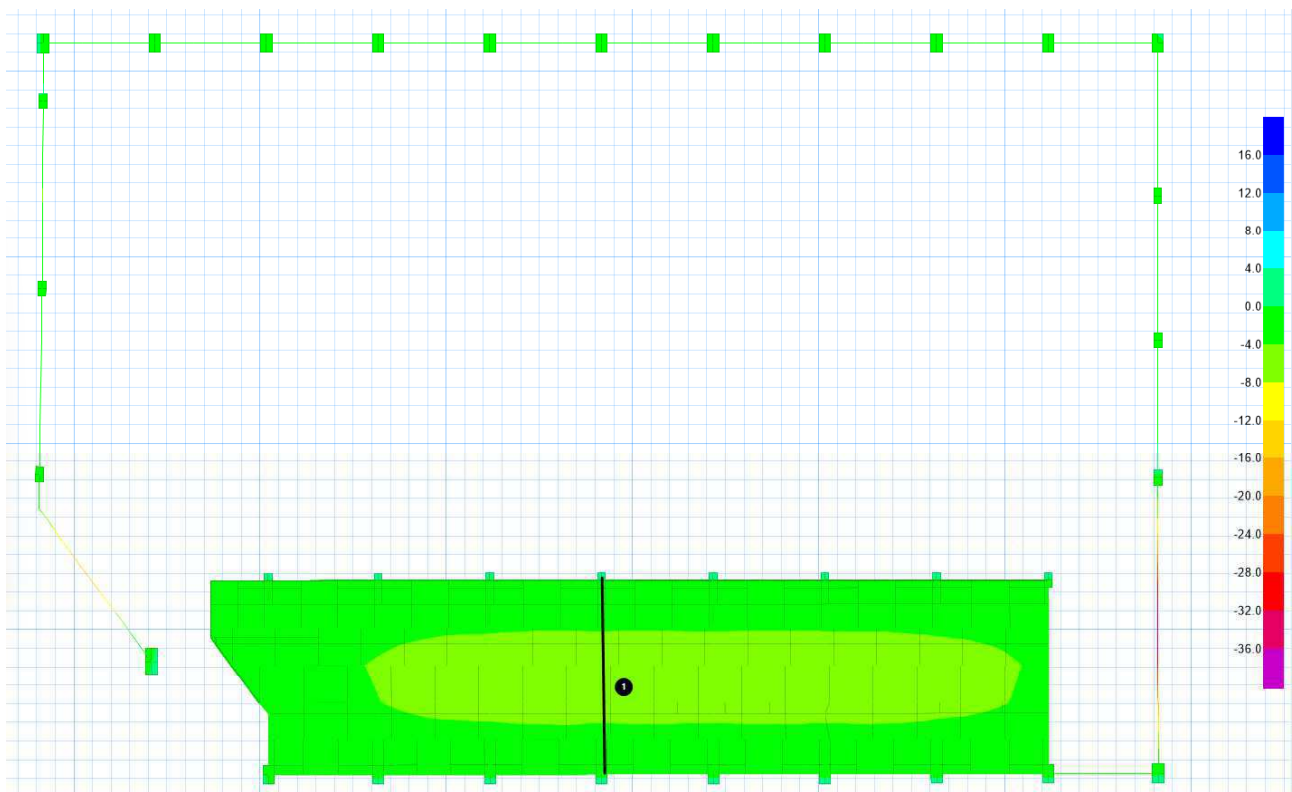


Figure 6-38 Long-Term Deflection at Ground Floor Level



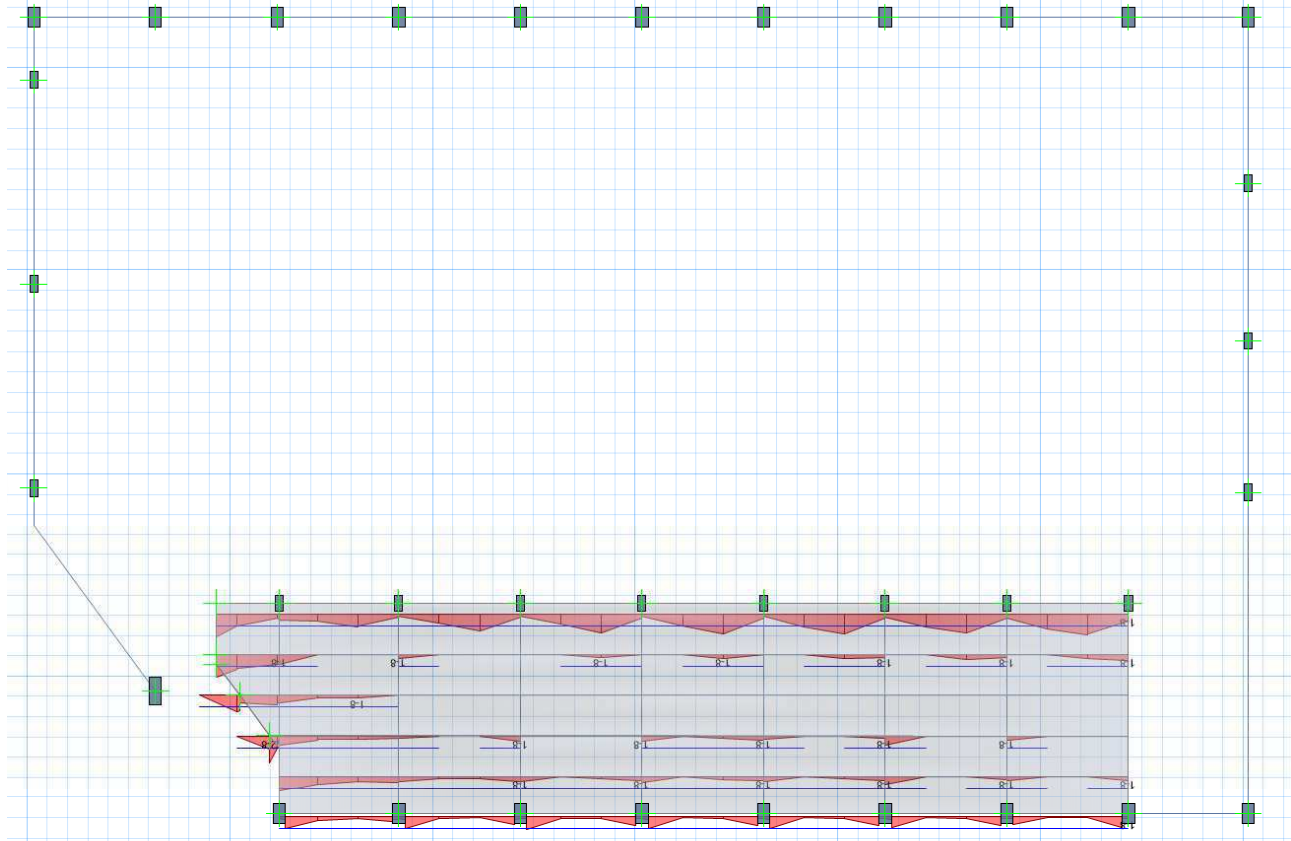


Figure 6-39 Slab Strip Design – Layer A – Top Reinforcing –4T8 for Ribs and 5T10 mesh for solid slab at GROUND Floor Level as per as-built Drawings refer to **Appendix B**

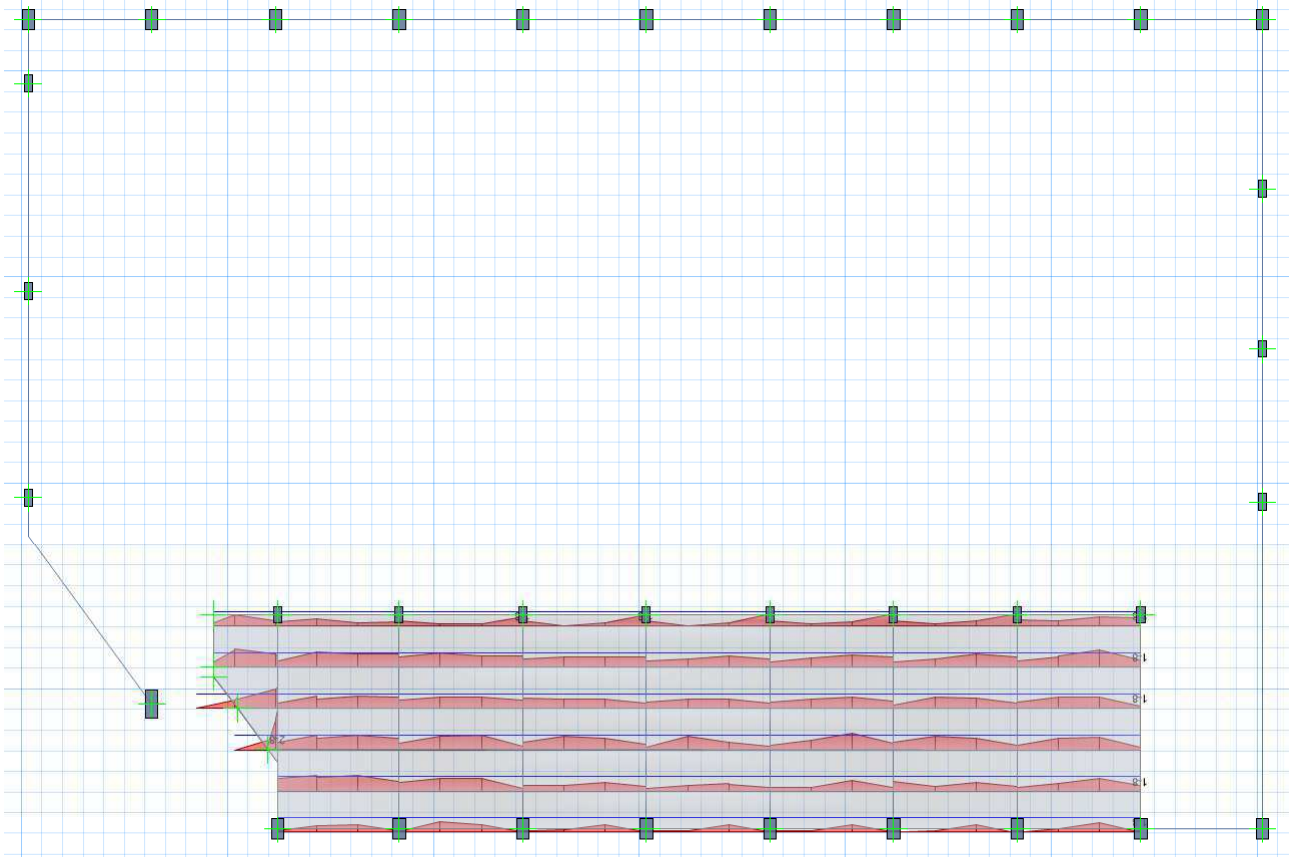


Figure 6-40 Slab Strip Design – Layer A – bottom Reinforcing –4T8-mesh at ground Floor Level as per as-built Drawings refer to **Appendix B**

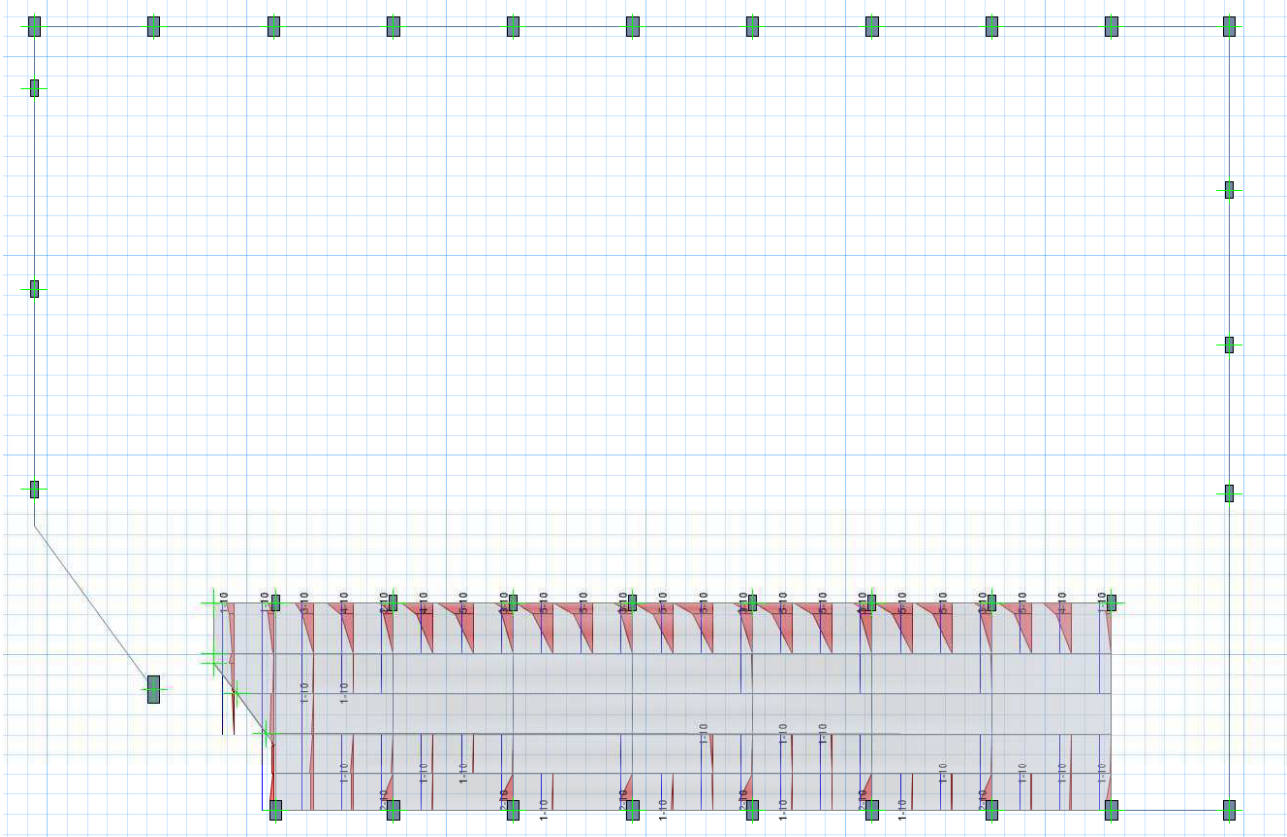


Figure 6-41 Slab Strip Design – Layer B – Top Reinforcing –5T10 mesh –Bottom Reinforcing at ground Floor Level as per as-built Drawings refer to **Appendix B**

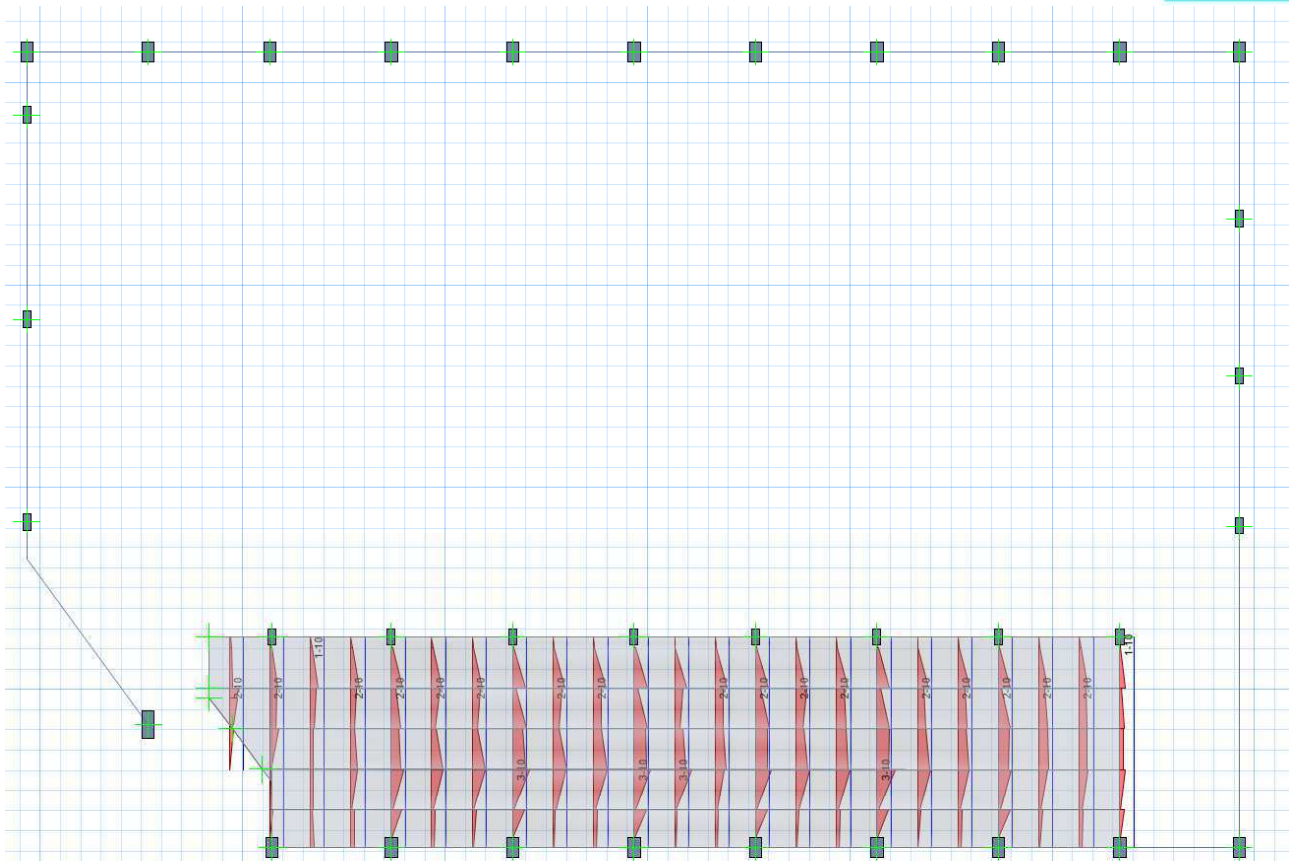


Figure 6-42 Slab Strip Design – Layer B –5T10 mesh –Bottom Reinforcing at Ground Floor Level as per as-built Drawings refer to **Appendix B** at Ground Floor Level



### A. Roof Floor

The computed maximum deflection at Roof Floor is summarized below.

Label	Length, m	Allowable deflection, mm	Computed deflection, mm	Remarks
1	19.2	80	31	Computed Deflection is within the allowable deflection

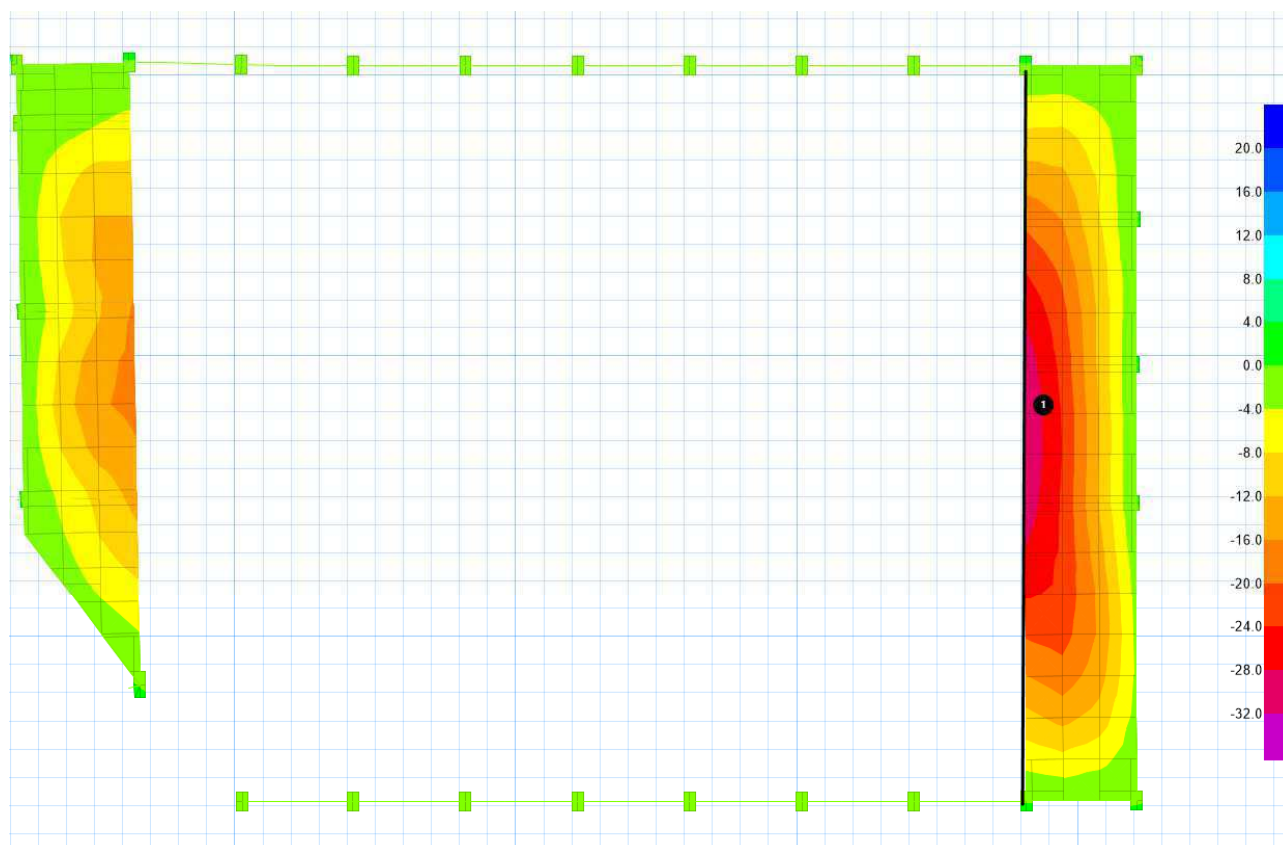


Figure 6-43 Long-Term Deflection at Roof Floor Level

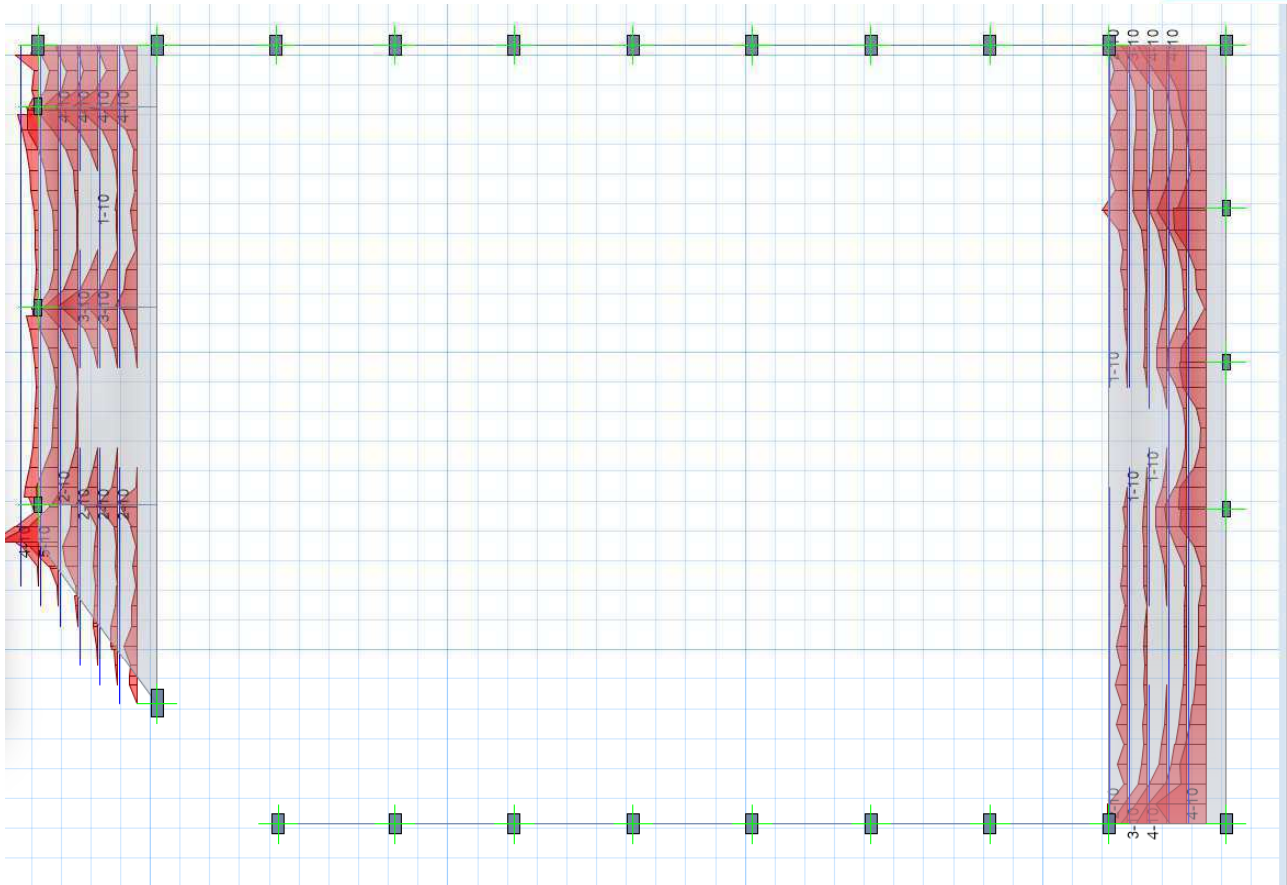


Figure 6-44 Slab Strip Design – Layer A – Top Reinforcing – 5T10 for Top Reinforcing at Roof Floor Level as per as-built Drawings refer to **Appendix B**

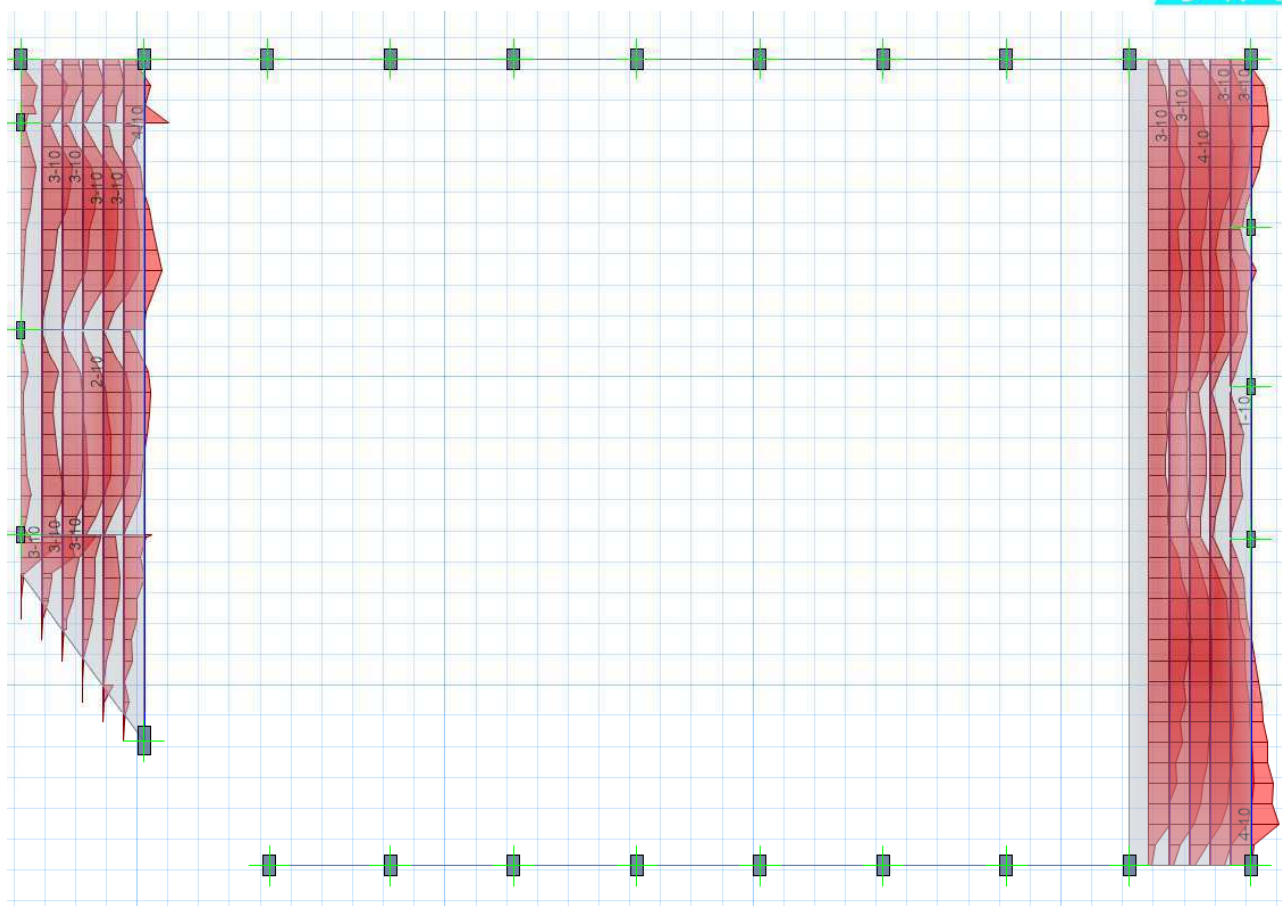


Figure 6-45 Slab Strip Design – Layer A –5T10 for at Bottom Reinforcing at Roof Floor Level as per as-built Drawings refer to **Appendix B**

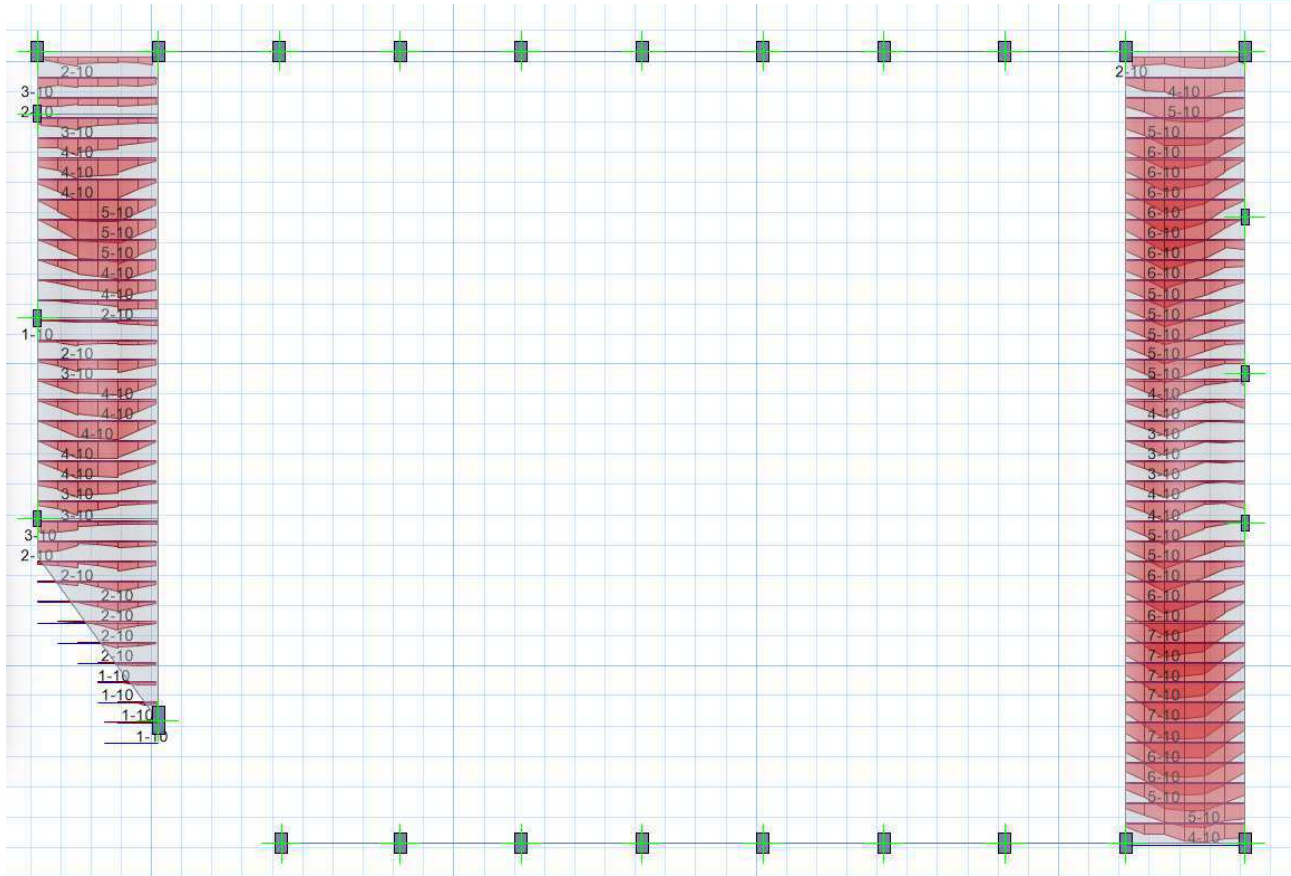


Figure 6-46 Slab Strip Design – Layer B –5T10 for Bottom Reinforcing at Roof Floor Level as per as-built Drawings refer to **Appendix B**



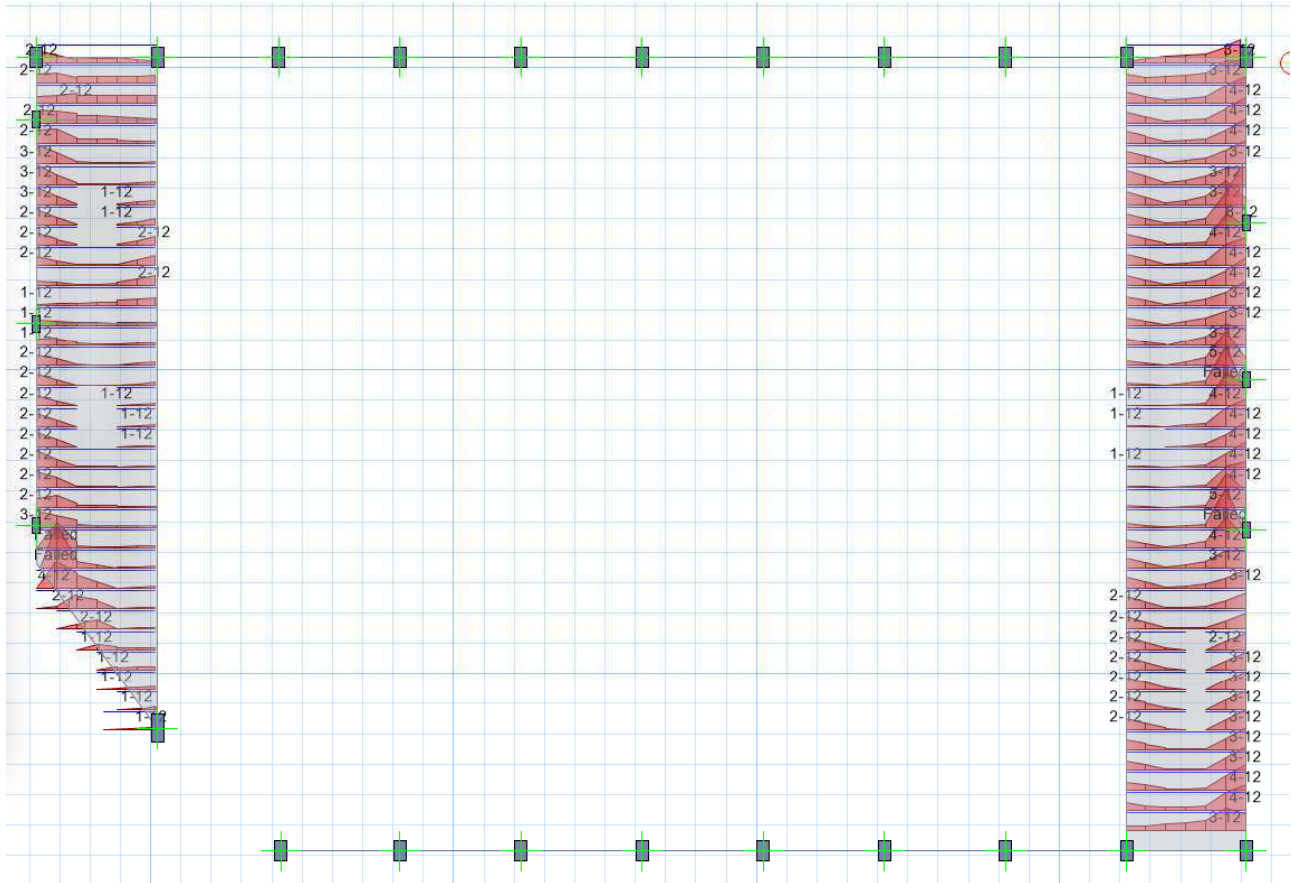


Figure 6-47 Slab Strip Design – Layer A –5T12 for Top Reinforcing at Roof Floor Level as per as-built Drawings refer to **Appendix B**

## 6.2.2 Beam Design

SAFE calculates and reports the required areas of reinforcement for flexure, shear, and torsion based on the moments, shear forces, torsion, load combination factors, and other criteria described in this section. The reinforcement requirements are calculated at each station along the length of the beam.

The beam top and bottom flexural reinforcement is designed at each station along the beam.

The shear reinforcement is designed for each load combination at each station along the length of the beam.

These reinforcements are checked with the provided shear reinforcement and found to be sufficient. Therefore, design of beams is adequate.

### 6.2.2.1 Gymnasium part

#### A. GROUND Floor

##### • Longitudinal Bar

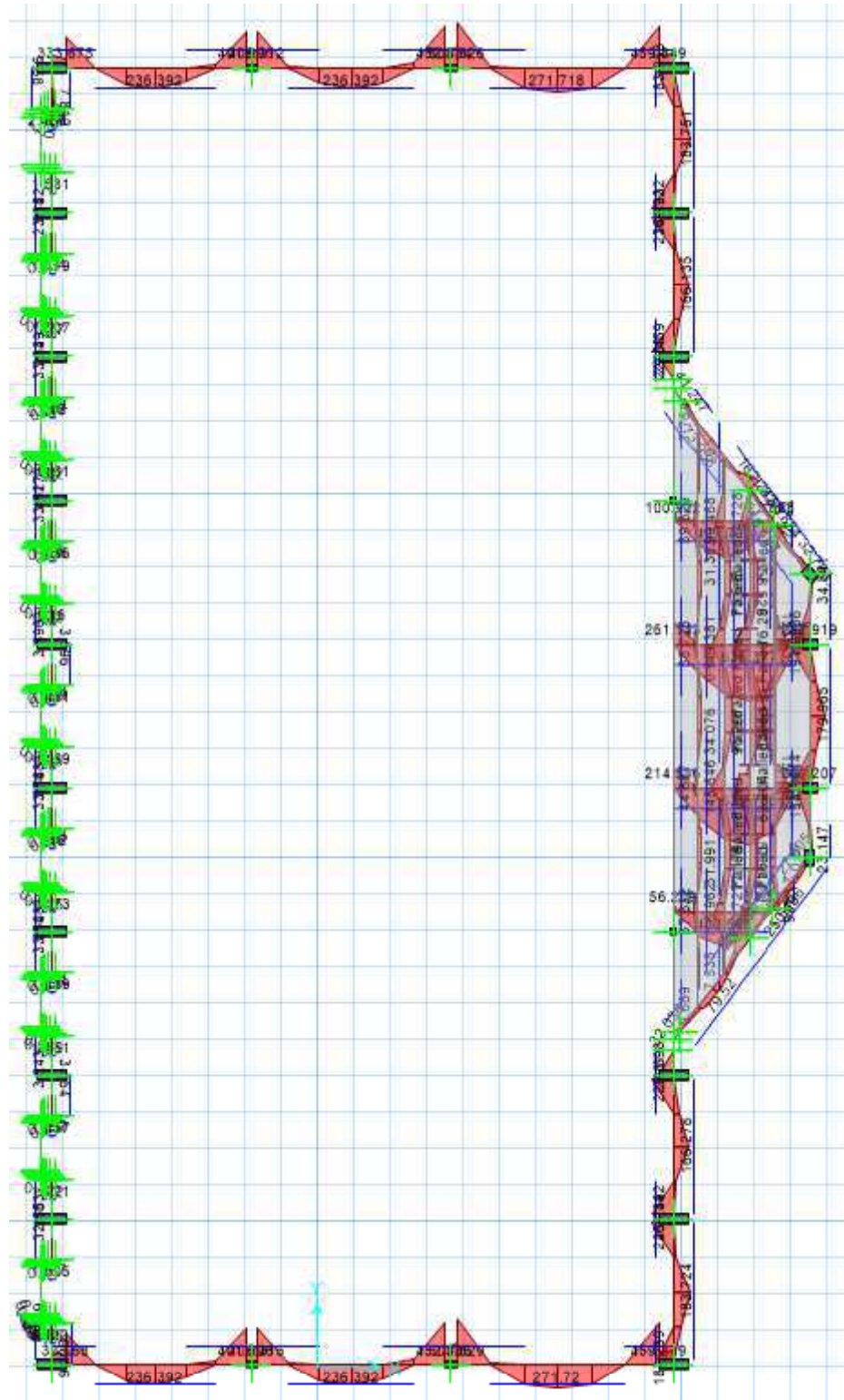


Figure 6-48 Required Longitudinal Reinforcement at GROUND Floor Level















## B. Roof Floor

- Longitudinal Bar

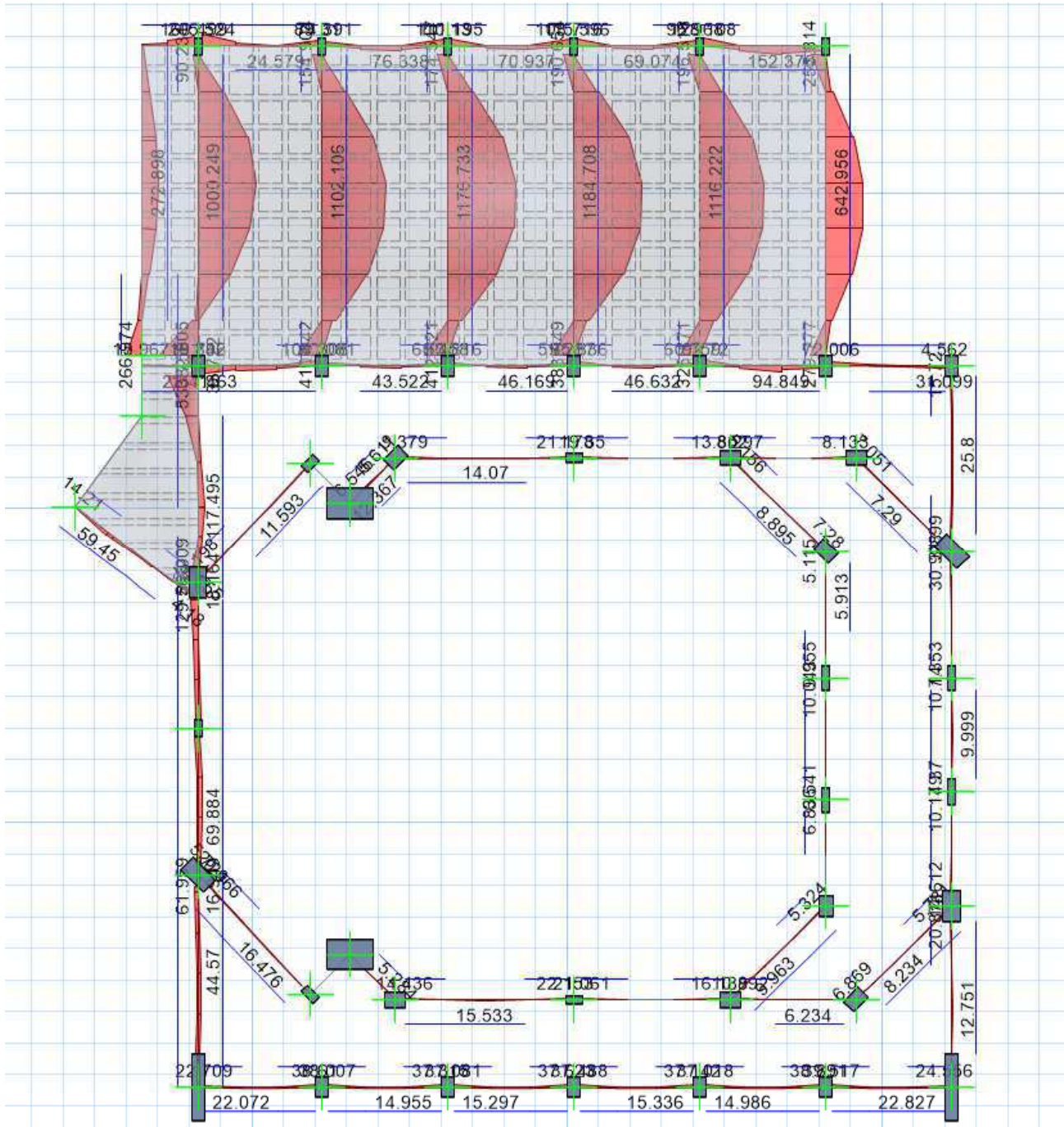


Figure 6-54 Required Longitudinal Reinforcement at Roof Floor Level



# • Transverse Reinforcement

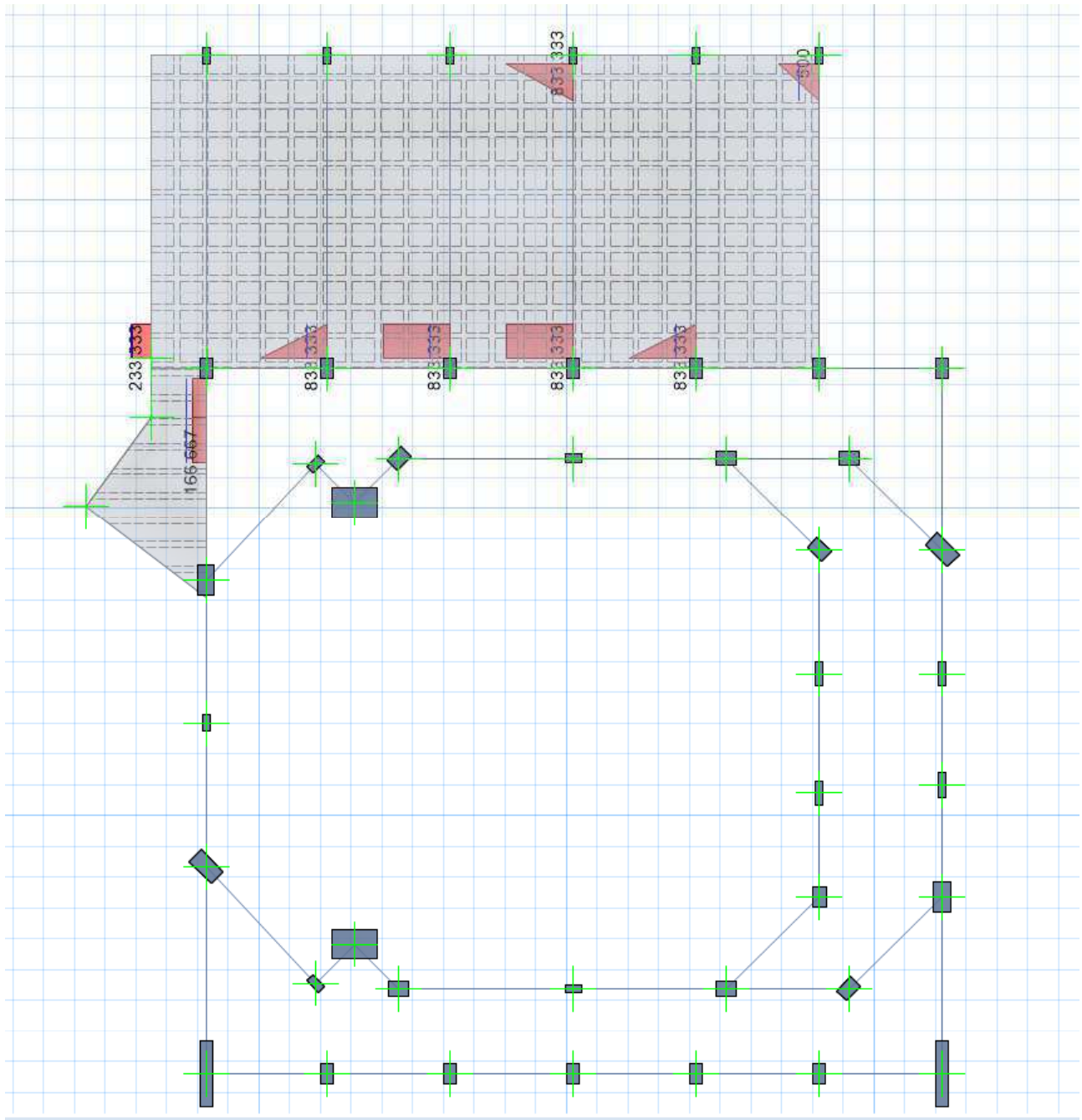


Figure 6-55 Required Transverse Reinforcement at Roof Floor Level

### 6.2.2.3 Swimming Part

#### A. GROUND Floor

##### • Longitudinal Bar

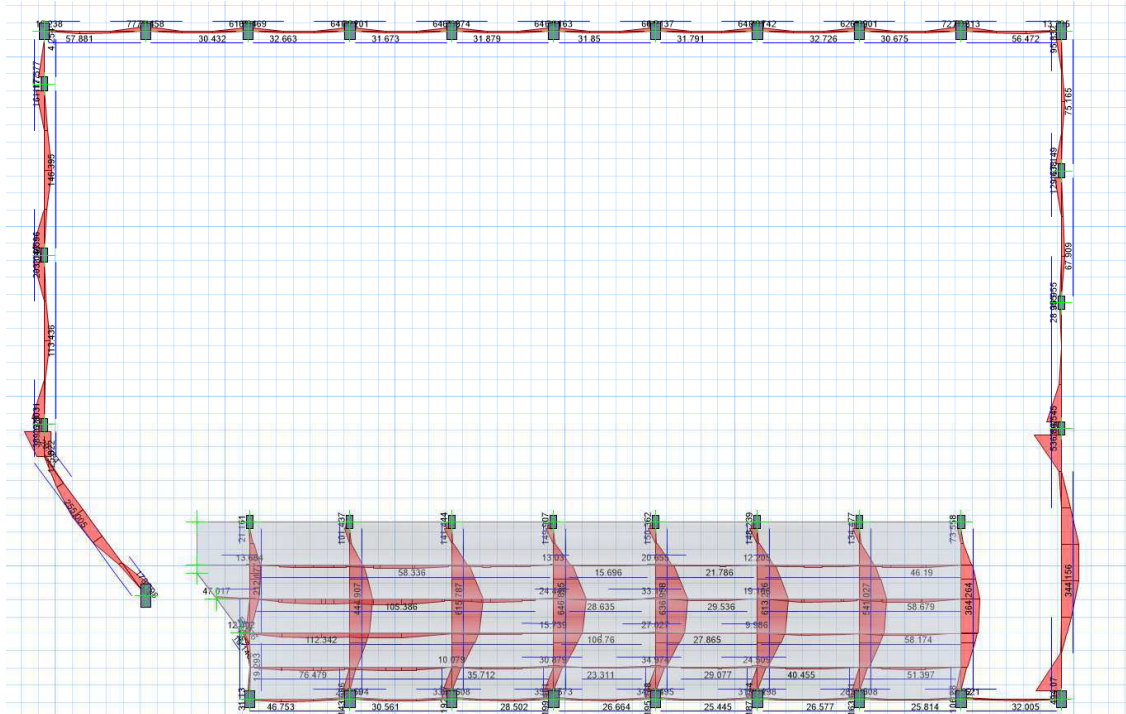


Figure 6-56 Required Longitudinal Reinforcement at GROUND Floor Level

##### • Transverse Reinforcement

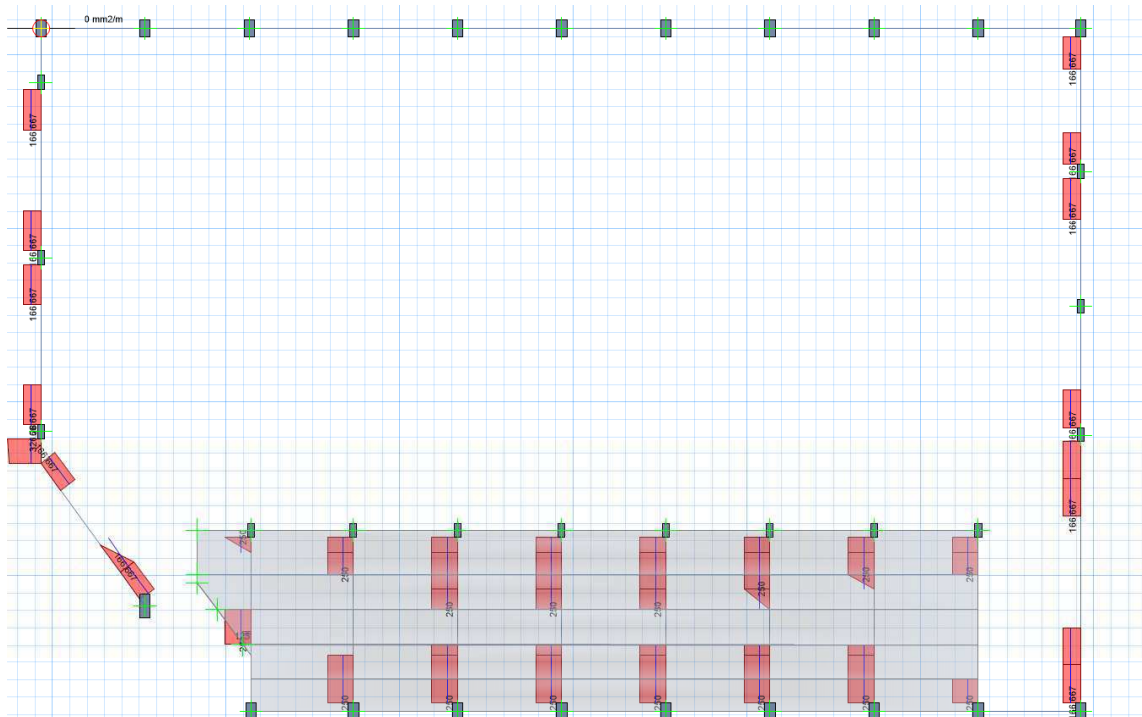


Figure 6-57 Required Transverse Reinforcement at GROUND Floor Level

## B. Roof Floor

- Longitudinal Bar

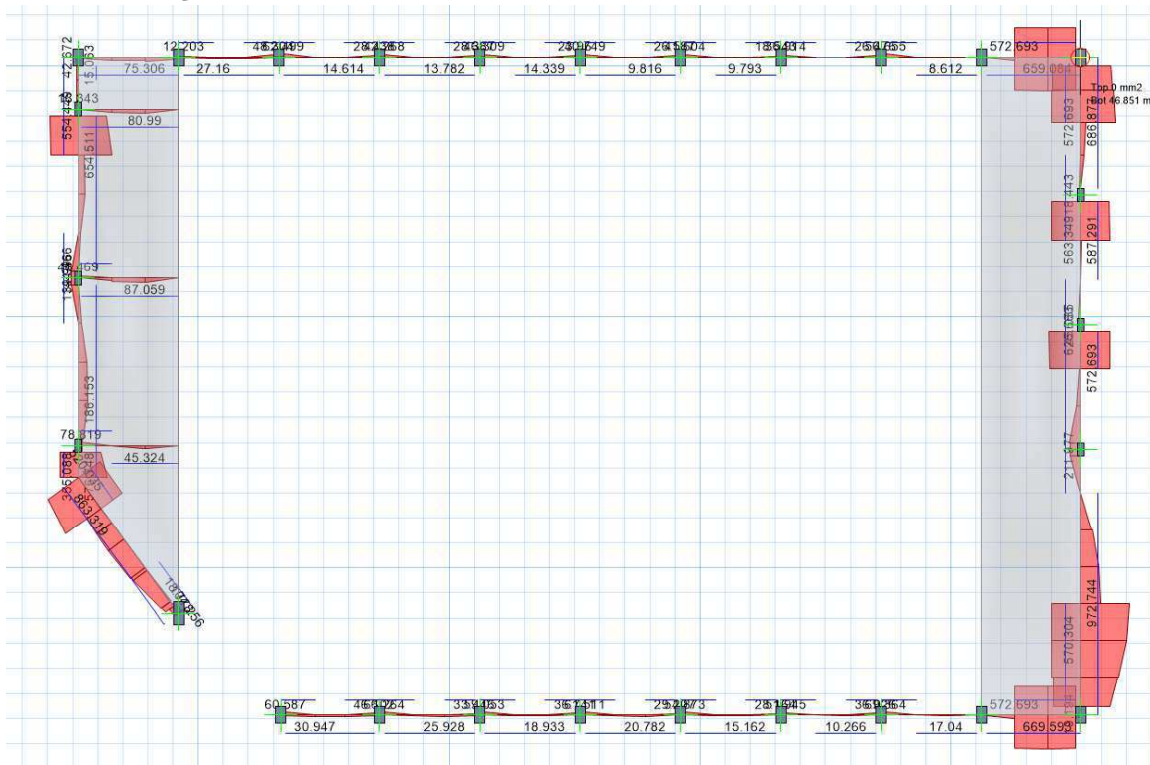


Figure 6-58 Required Longitudinal Reinforcement at Roof Floor Level

- Transverse Reinforcement

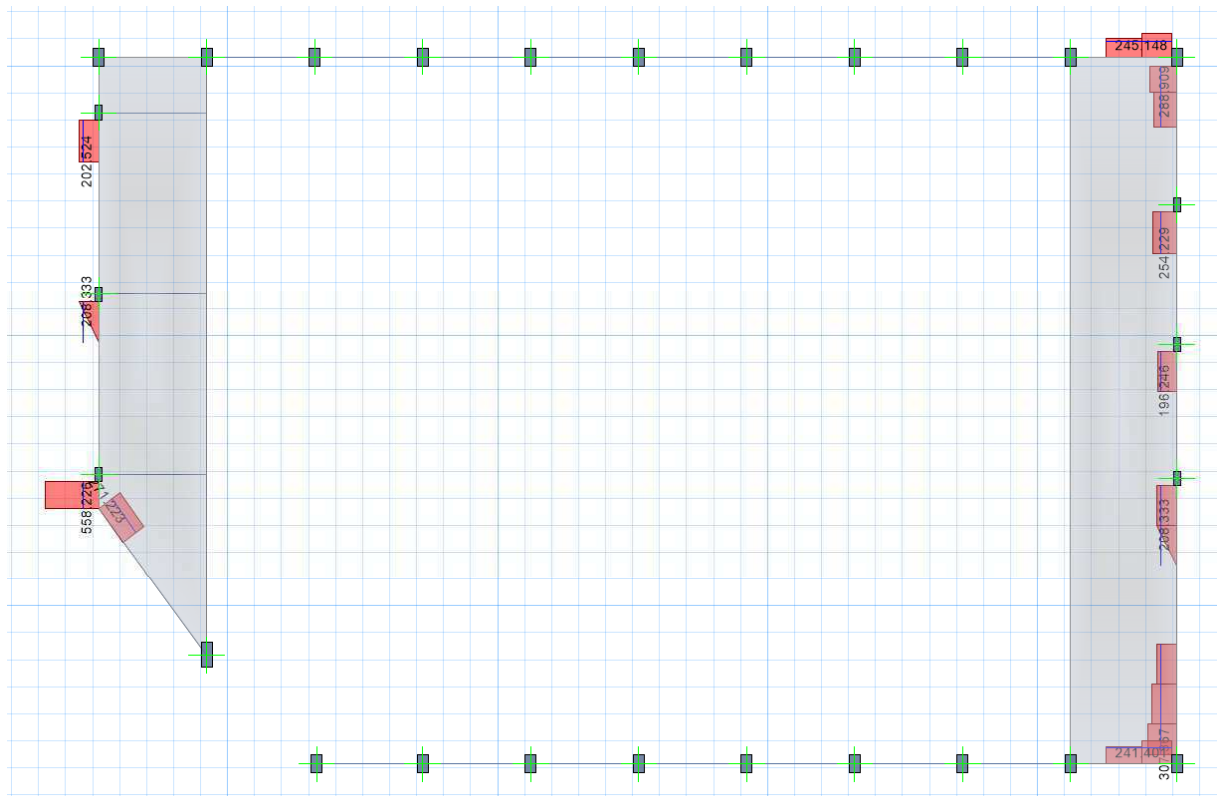


Figure 6-59 Required Transverse Reinforcement at Roof Floor Level



Figure 6-60 P-M RATIO STEEL BEAM at Roof Floor Level



## 7 DESIGN RESULT SUMMARY

### 7.1 Columns

The existing columns at ground floor and Roof floor are overstressed due to low concrete compressive strength. In addition, some of the columns at upper level are under design, these columns take greater lateral forces that causes larger column and beam bending moments.

Therefore, column strengthening is required by section enlargement technique to reach safe and sound structural elements.

### 7.2 Shear Walls

The existing shear wall is structurally adequate in flexural and shear design due to gravity and lateral loads.

### 7.3 Beam

The existing beam is structurally adequate in flexural and shear design due to gravity loads.

### 7.4 Slabs

The existing slab is structurally adequate in flexural and shear design due to gravity loads.

## 8 STRENGTHENING DESIGN

A second ETABS Model was created for purposed of strengthening design. In that model, the proposed strengthening is applied in the models to check the adequacy of the design, in addition to design requirements.

### 8.1 Column Strengthening Design

The under-design columns are strengthened by section enlargement to resist the under designed flexural strength; and CFRP wrap for confinement of partial column enlargement.

The proposed enlargement of the columns needs to be strengthened are created in section designer by providing the section that represents the proposed concrete enlargement. The new sections are provided with T16-150 Vertical reinforcement with 100mm/150mm thick C50 new micro-concrete as shown below.

Refer to **Appendix C** for strengthening drawings and related details.

## 8.1.1 GYMNASIUM COLUMNS

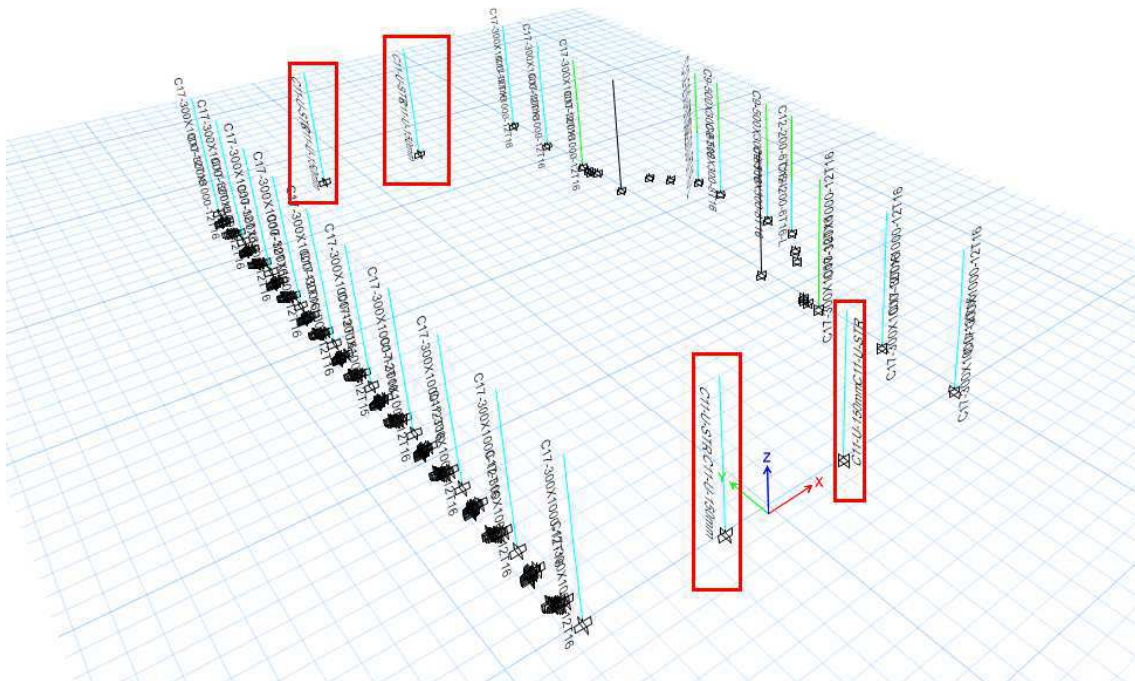


Figure 8-1 Column Strengthening Assignment at Ground to Roof Floor Level

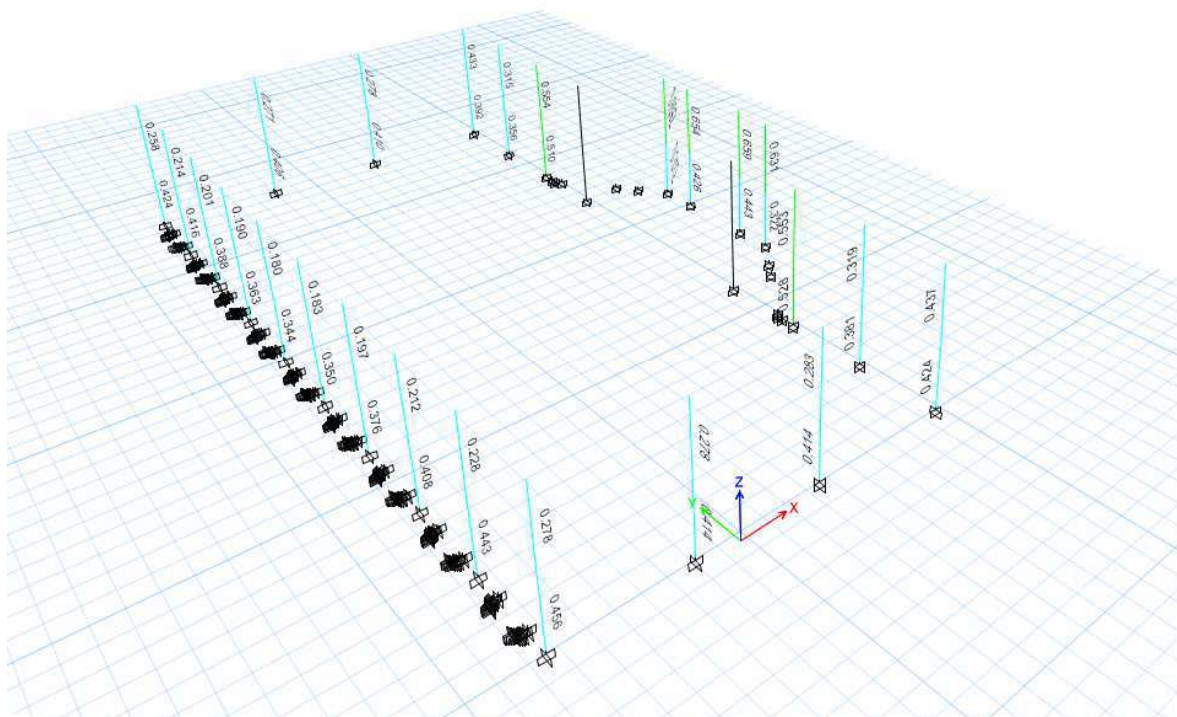


Figure 8-2 Column PMM Ratio after strengthening at Ground to Roof Floor Level



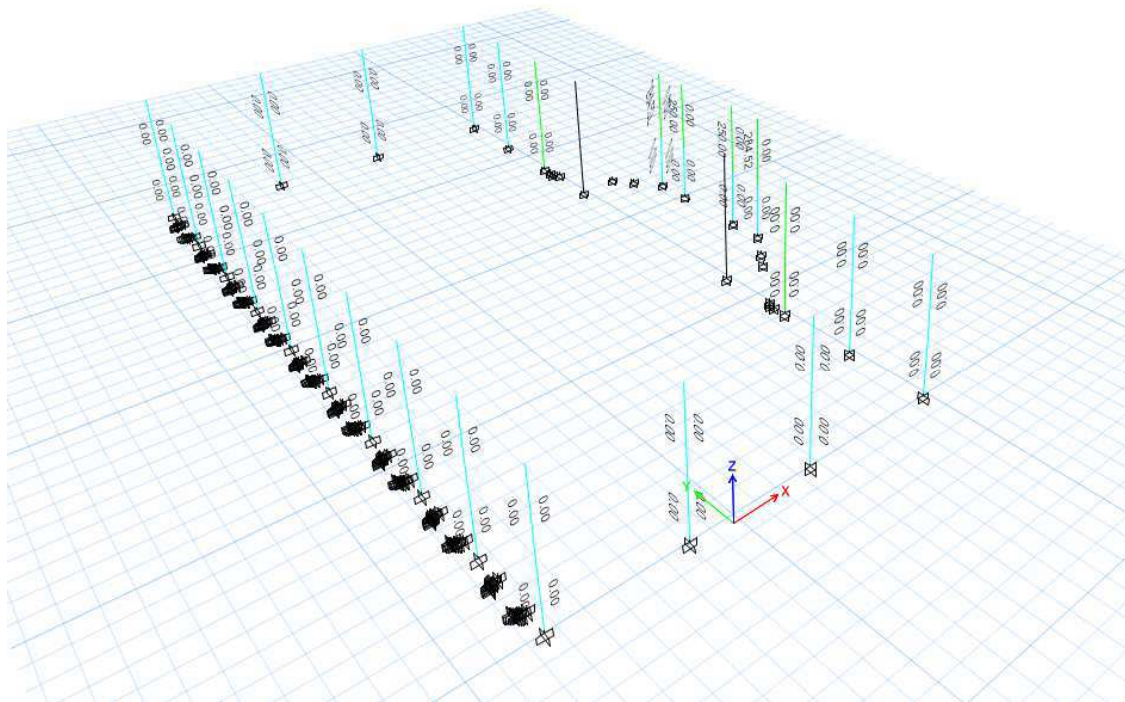


Figure 8-3 Column Shear Reinforcing after strengthening at Ground to Roof Floor Level

## 8.1.2 AUDITORIUM COLUMNS

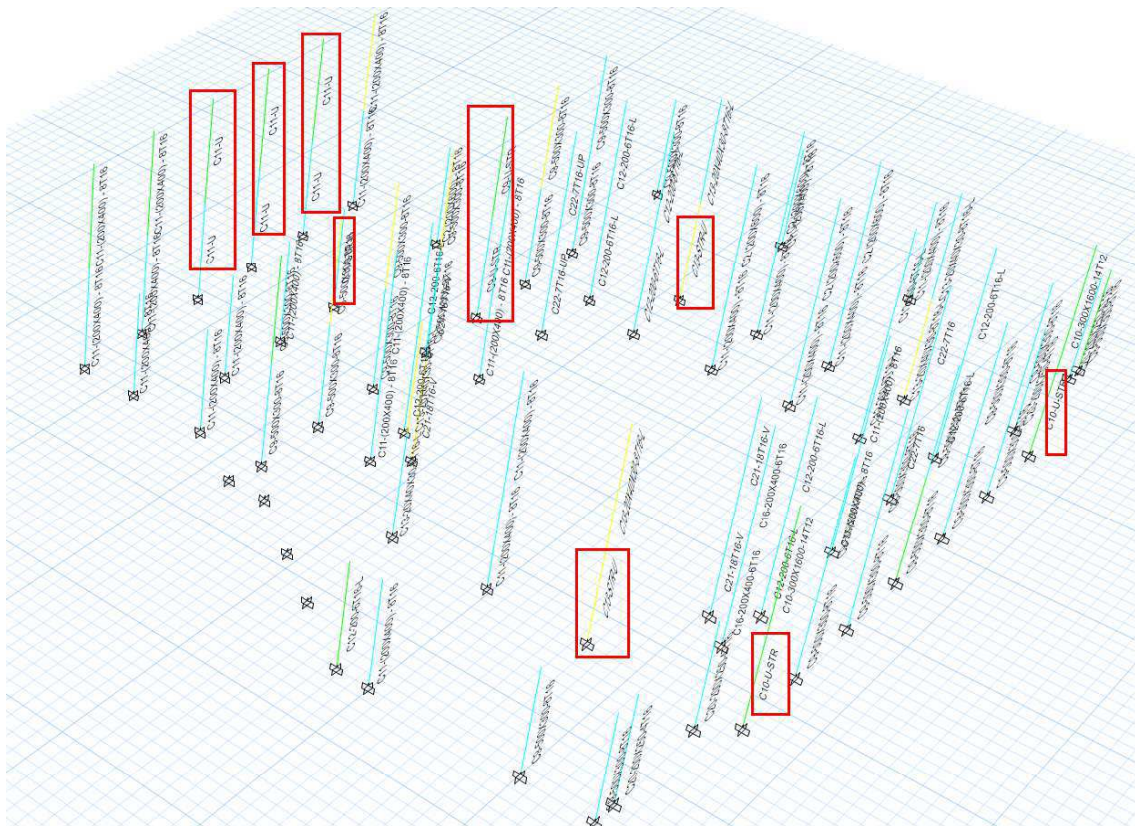


Figure 8-4 Column Strengthening Assignment at Ground to Roof Floor Level



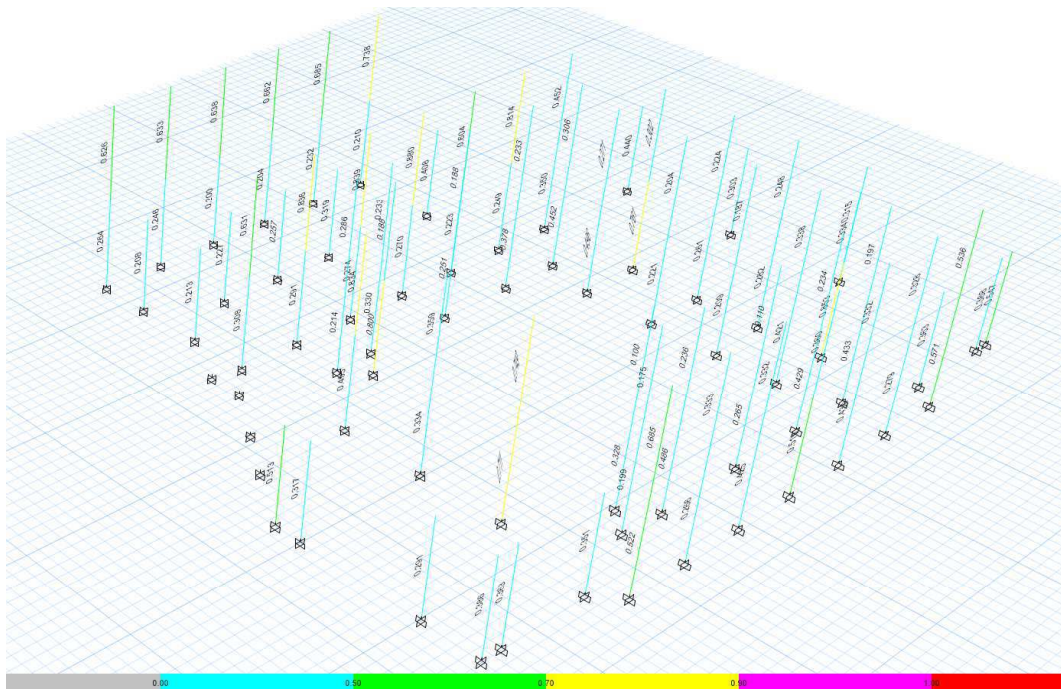


Figure 8-5 Column PMM Ratio after strengthening at Ground to Roof Floor Level

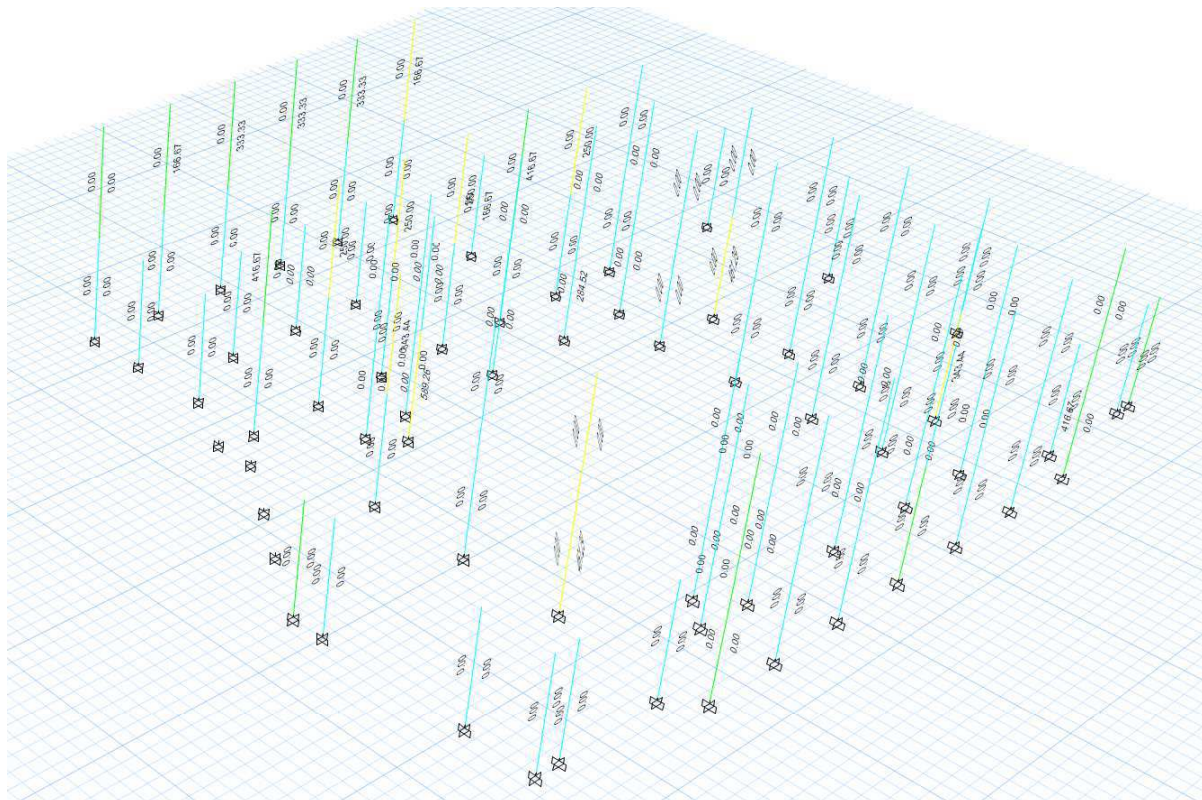


Figure 8-6 Column Shear Reinforcing after strengthening at Ground to Roof Floor Level



### 8.1.3 SWIMMING COLUMNS

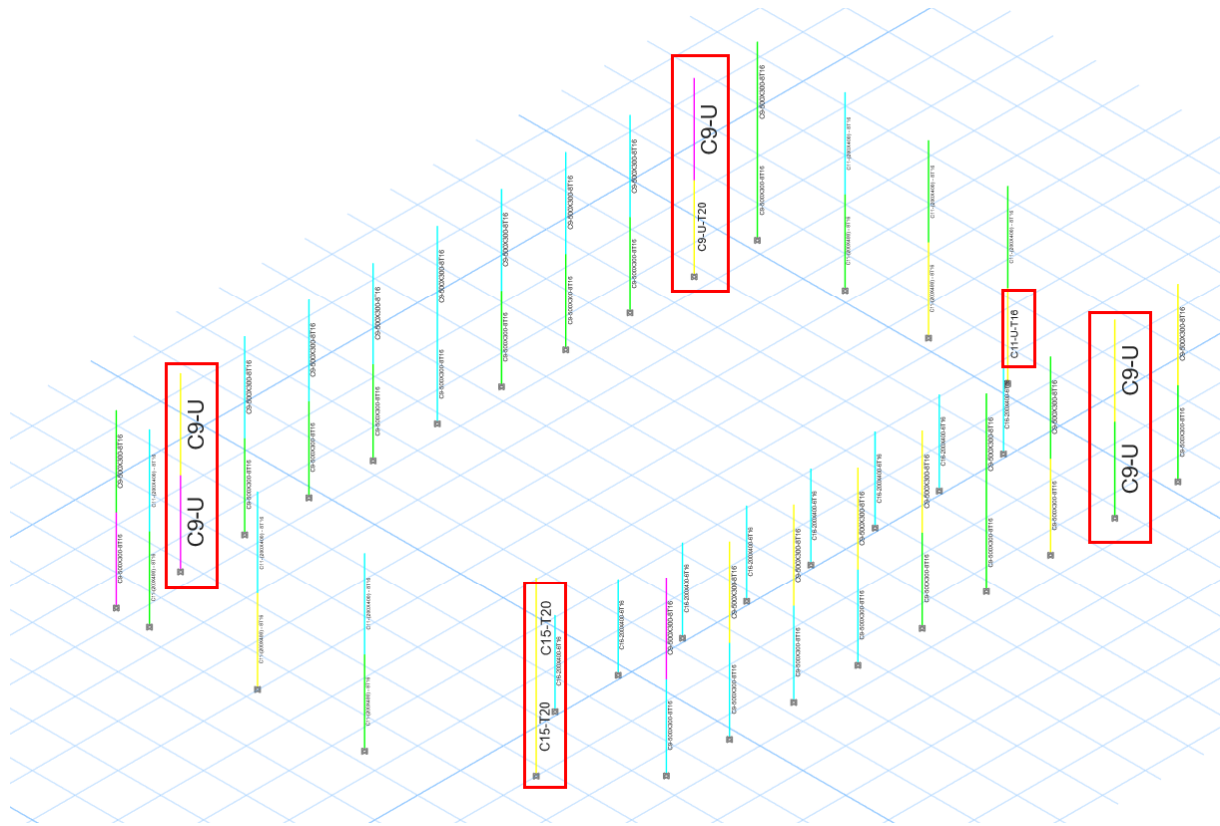


Figure 8-7 Column Strengthening Assignment at Ground to Roof Floor Level

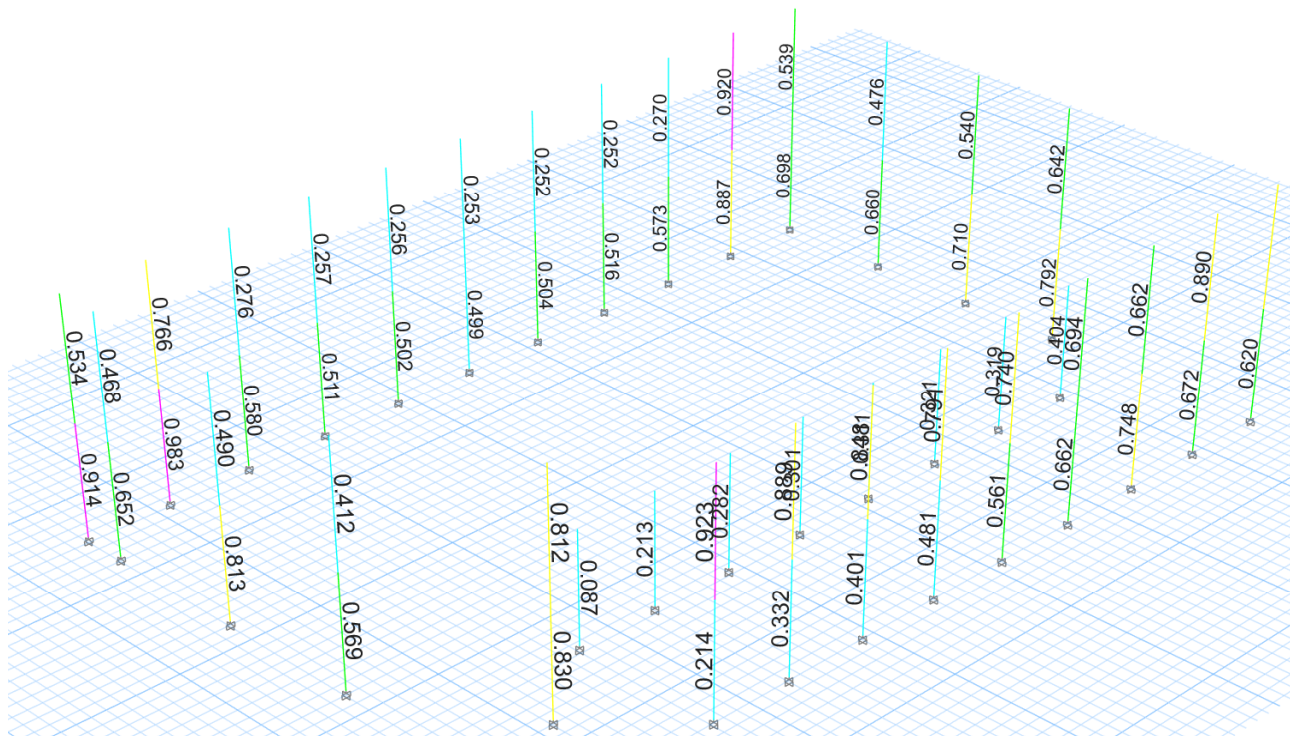


Figure 8-8 Column PMM Ratio after strengthening at Ground to Roof Floor Level

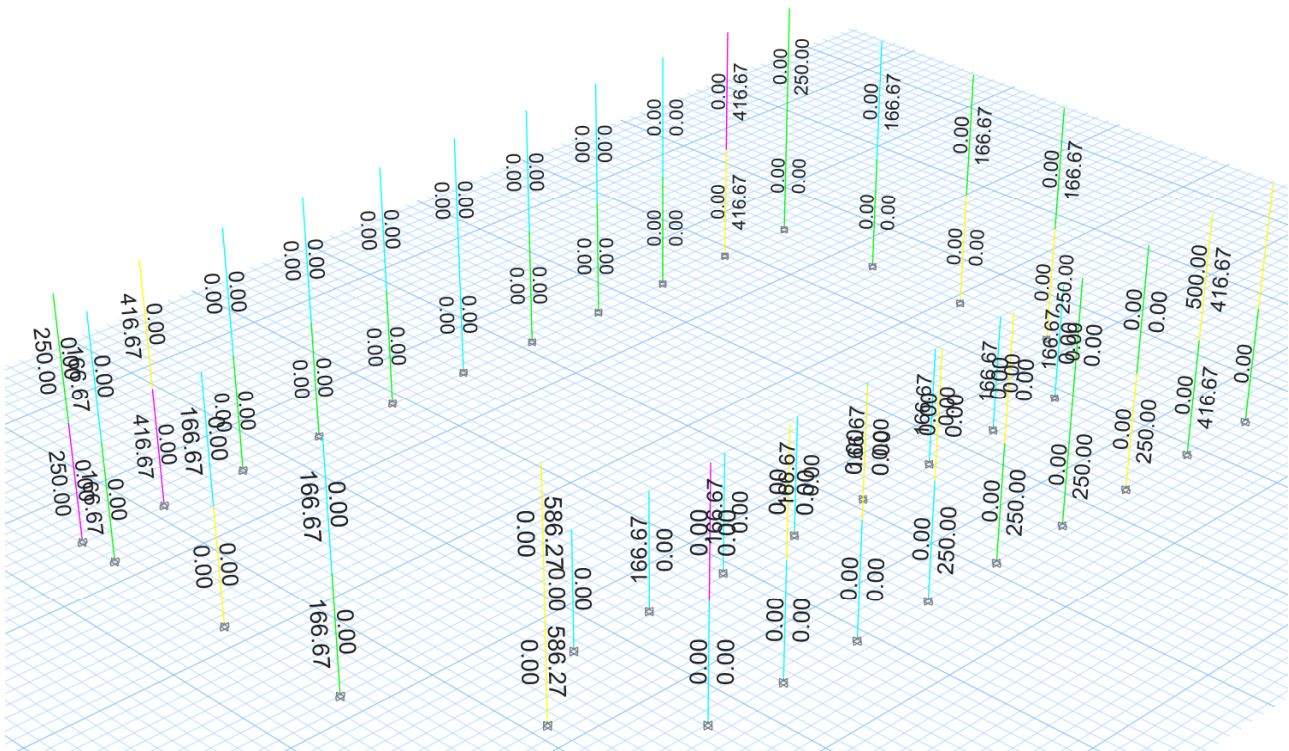


Figure 8-9 Column Shear Reinforcing after strengthening at Ground to Roof Floor Level

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American Concrete Institute, ACI 318M, 2011, Building Code and Commentary.

American Concrete Institute, ACI 437R, 1991, Strength evaluation of existing concrete buildings.

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CIRIA Report, 2002, Guide to the Construction of Reinforced Concrete in the Arabian Peninsula.

Frank Rendell, Raoul Jauberthie and Mike Grantham, 2002, Deteriorated Concrete, Inspection and Physicochemical Analysis, Thomas Trlford, 194 p



**Dr. Ashraf Biddah**

Reinforced concrete Professor  
Repair and Strengthening Expert



## **Appendix A**

# **STRENGTHENING DRAWINGS**



GENERAL NOTES

1. THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND CONDITIONS AT THE JOB SITE PRIOR TO BEGINNING WORK.
2. CONTRACTOR SHALL REPORT ANY DISCREPANCIES IN THE DRAWINGS TO THE DESIGN ENGINEER.
3. NOTES SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER AND RESOLVED BEFORE PROCEEDING TO WORK WHEN THERE IS AN OMISSIONS OR CONFLICT BETWEEN VARIOUS ELEMENTS OF THE DRAWINGS.
4. DRAWINGS MUST NOT BE SCALE. ALL DIMENSIONS ARE IN MILLIMETERS UNLESS INDICATED OTHERWISE.
5. THE REPAIR & STRENGTHENING WORK SHOULD BE EXECUTED ON SITE THROUGH APPROVED REPAIR AND STRENGTHENING SUB CONTRACTOR AND SUPERVISED BY SPECIALIZED REPAIR AND STRENGTHENING CONSULTANT.
6. THE CONTRACTOR IS REQUIRED TO PROVIDE A GENERAL METHOD STATEMENT DETAILING PHASING OF THE WORKS, PROGRAMMING, MATERIALS TO BE USED ETC.
7. THE CONTRACTOR SHOULD PAY ATTENTION TO THE SPECIFICATION REQUIREMENTS FOR APPROPRIATE SHORING, REPAIR PREPARATION, MATERIAL INSTALLATION AND SUBSEQUENT CURING.
8. DURING REPAIR WORKS, NEW LOADS SHALL BE AVOIDED ON THE AREAS WHERE STRENGTHENING WILL BE DONE. CONTRACTOR SHALL PROCEED WITH LIGHT FINISHING WORKS.
9. TEMPORARY WORKS SUCH AS PROPPING AND BRACING OF THE STRUCTURE SHALL BE INSTALLED AS PER THE RECOMMENDED PROCEDURE AND SPECIFICATION OF THE DESIGNER.
10. ALL REQUIRED TASKS INCLUDING CONCRETE REMOVAL AND SURFACE PREPARATION, NEED TO BE DONE IN ACCORDANCE WITH SPECIFICATIONS AND DRAWINGS AND TO GOOD INDUSTRY PRACTICE.
11. CONTRACTOR SHALL HAVE DAILY MONITORING OF TEMPERATURE, HUMIDITY, AND WEATHER CONDITIONS THAT MAY AFFECT THE REPAIR PROCEDURE.
12. SAFETY MEASURE SHALL BE IMPLEMENTED TO ENSURE THE SAFETY OF ALL PERSONNEL AND THE STRUCTURE BEING REPAIRED OR STRENGTHENED THROUGHOUT THE COURSE OF THE WORK.

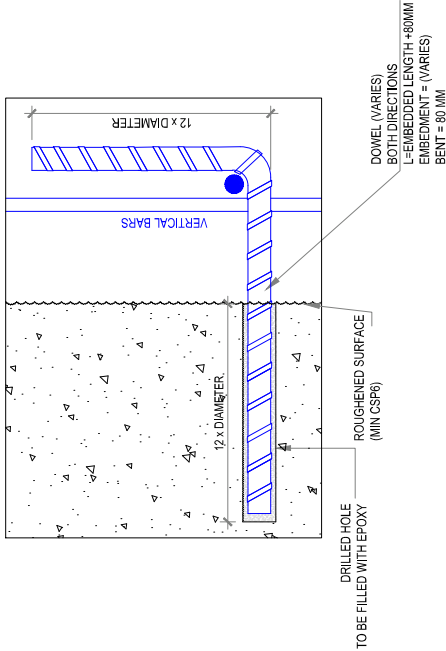
MINIMUM EMBEDMENT LENGTHS		
BAR DIAMETER (MM)	DRILLED HOLE DIAMETER (MM)	MINIMUM EMBEDMENT (MM)
10	14	120
12	16	150
16	20	200
20	25	250
25	32	300
32	40	350

USE PURE EPOXY BONDED ANCHORING SYSTEM SUCH AS:

- 1- HILTI RESIN 932 AN
  - 2- MASTER FLOX 932 AN
  - 3- Sika Anchor fix 3
- OR APPROVED EQUIVALENT

NOTES FOR CONCRETE ENLARGEMENT

1. EXISTING CONCRETE SURFACE SHOULD BE ROUGHEN TO 6MM AMPLITUDE INCLUDING SLAB SOFT, BEAM SIDES, WALLS AND COLUMN SIDES.
2. REINFORCEMENT STEEL SHALL BE GRADE 460MPA.
3. THE STARTER BARS SHOULD BE INSTALLED BY DRILLING HOLES IN THE SLAB TO PATH THROUGH THE BARS
4. STEEL DOWELS SHOULD BE ANCHORED THROUGH THE EXISTING COLUMNS OR WALLS USING HILTI RE 500 OR EQUIVALENT.
5. A MINIMUM CLEARANCE OF 25MM SHALL BE ALLOWED BETWEEN PROPOSED LONGITUDINAL BARS AND ROUGHENED SURFACE OF THE CONCRETE.
6. PROPOSED NORMAL CONCRETE C50 SHOULD BE USED WITH MINIMUM CUBE COMPRESSIVE STRENGTH 50NMM<sup>2</sup>.
7. CONCRETE POURING SHOULD BE FROM LETTERHEAD DOORS FROM THE SHUTTER AT MAXIMUM HEIGHT 2M.
8. THE LAST 300MM BELOW THE SLAB SHOULD BE CLOSED AND FILLED BY PUMPING CONCRETE GROUT SUCH AS MASTEREMACO S466 OR EQUIVALENT.
9. FULL PUMPING TECHNIQUE IS ALSO ACCEPTED.
10. PROPER CURING IS REQUIRED FOR MIN 15 DAYS
11. ALL PROPOSED STEEL BARS SHOULD ACHIEVE ITS DEVELOPED LENGTH WITH THE EXISTING STARTER BARS, IF NOT, SHOULD BE ANCHORED TO THE EXISTING WALL BASED ON THE TABLE AT GENERAL NOTES.
12. ALL EXPOSED STEEL BARS SHALL BE COATED WITH ZINC-RICH PAINT
13. MINIMUM CONCRETE CLEAR COVER TO BE AS FOLLOWS:
  - COLUMNS 40MM
  - WALLS 25MM
  - SLABS 25MM
  - BEAMS 40MM



DOWEL DETAIL  
— N.T.S

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5. ANY DISCREPANCIES TO BE BROUGHT TO THE ATTENTION OF eFORCE INSPECTION CONSULTANCIES BEFORE COMMENCEMENT OF WORK.

REV	FOR SUBMITTAL	DESCRIPTION	DATE	DESIGN	CHECK
01					

CLIENT

M/S AL HILAL ENGINEERING CONSULTANTS

REPAIR AND STRENGTHENING CONSULTANT  
eFORCE Inspection Consultancies  
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Riad Street, Al-Sayada Street,  
Abu Dhabi, UAE  
Tel: 02-4777788 | Fax: 02-4777788 | Email: info@eforce.ae  
P.O. Box 123456 | Abu Dhabi, UAE





PROJECT:

LYCEE LOUIS MASSIGNON  
SCHOOL, AL SAYADA STREET,  
ABU DHABI, UAE

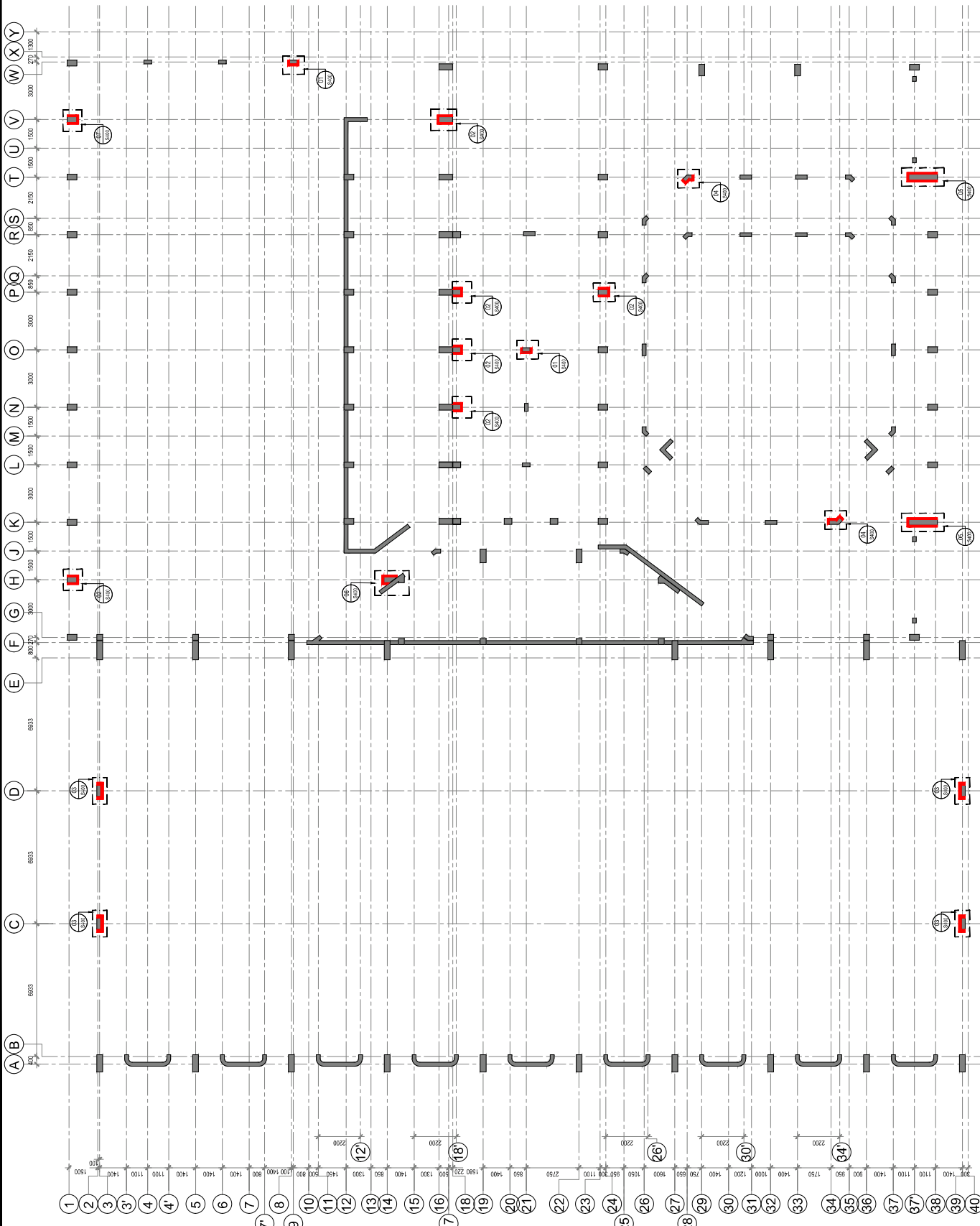
DRAWING TITLE:  
GENERAL NOTES  
AND DETAILS

SHEET SIZE	SCALE	PROJECT NO.
A1	N.T.S	P204-24
DRAWN BY	CHECKED BY	APPROVED BY
A.S	K.M	A.B
DRAWING NO.	REVISION NO.	
eFORCE-P204-24-S000	00	

AUTHORITIES APPROVAL

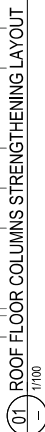
	EXISTING COLUMN OR WALL BELOW
	EXISTING COLUMN OR WALL ABOVE
	COLUMN OR WALL STRENGTHENING BELOW
	COLUMN OR WALL STRENGTHENING ABOVE

DRAWING TITLE	GROUND FLOOR COLUMNS STRENGTHENING LAYOUT				
SHEET SIZE	SCALE	PROJECT NO.			
A1	AS SHOWN	P204-24			
DRAWN BY	CHECKED BY	APPROVED BY			
A.S	A.J	A.B			
DRAWING NO.	REVISION NO.				
e/FORCE-P204-24-S100	01				
AUTHORITIES APPROVAL					



01 GROUND FLOOR COLUMNS STRENGTHENING LAYOUT

**AUTHORITIES APPROVAL**







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3. THIS DRAWING NOT TO BE SCALED. THE STATED DIMENSIONS ONLY ARE TO BE CONSIDERED.

4. DRAWINGS AS WELL AS AS-BUILT CONDITIONS.

5. ANY DISCREPANCIES TO BE BROUGHT TO THE ATTENTION OF eFORCE INSPECTION CONSULTANTS IMMEDIATELY UPON DETECTION.

**NOTE**

1. REFER TO DRAWING FOR GENERAL NOTES AND DETAILS.

2. REFER TO DRAWING FOR GENERAL NOTES AND DETAILS.

3. CONTRACTOR IS RESPONSIBLE TO OBTAIN NECESSARY APPROVALS IN ACCORDANCE WITH APPROVED DRAWING. ANY DISCREPANCY TO BE BROUGHT TO eFORCE INSPECTION CONSULTANTS IMMEDIATELY UPON DETECTION.

REV	FOR SUBMISSION	DESCRIPTION	DATE	DRAWN	CHECK
01	AS	AL			

CLIENT

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PROJECT

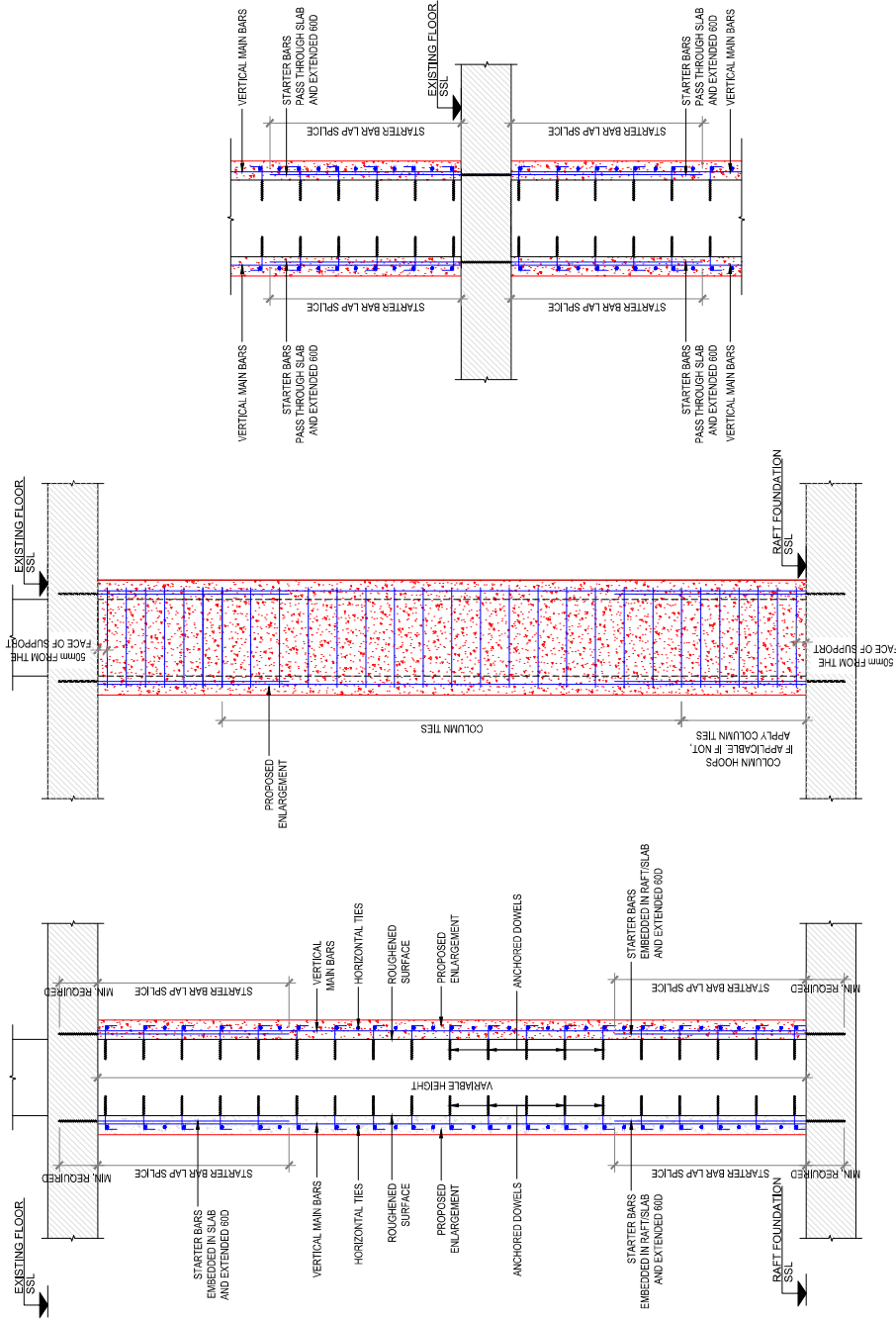
LYCEE LOUIS MASSIGNON  
SCHOOL, AL SA'ADA STREET,  
ABU DHABI, UAE

DRAWING TITLE

TYPICAL COLUMNS  
STRENGTHENING DETAILS

SHEET NO.	SCALE	PROJECT NO.
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DRAWN BY	CHECKED BY	APPROVED BY
AS	AL	AB
DRAWING NO.	REVISION NO.	
eFORCE-PD04-24-S401	00	

AUTHORITIES APPROVAL



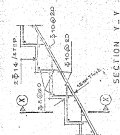
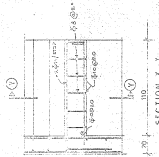
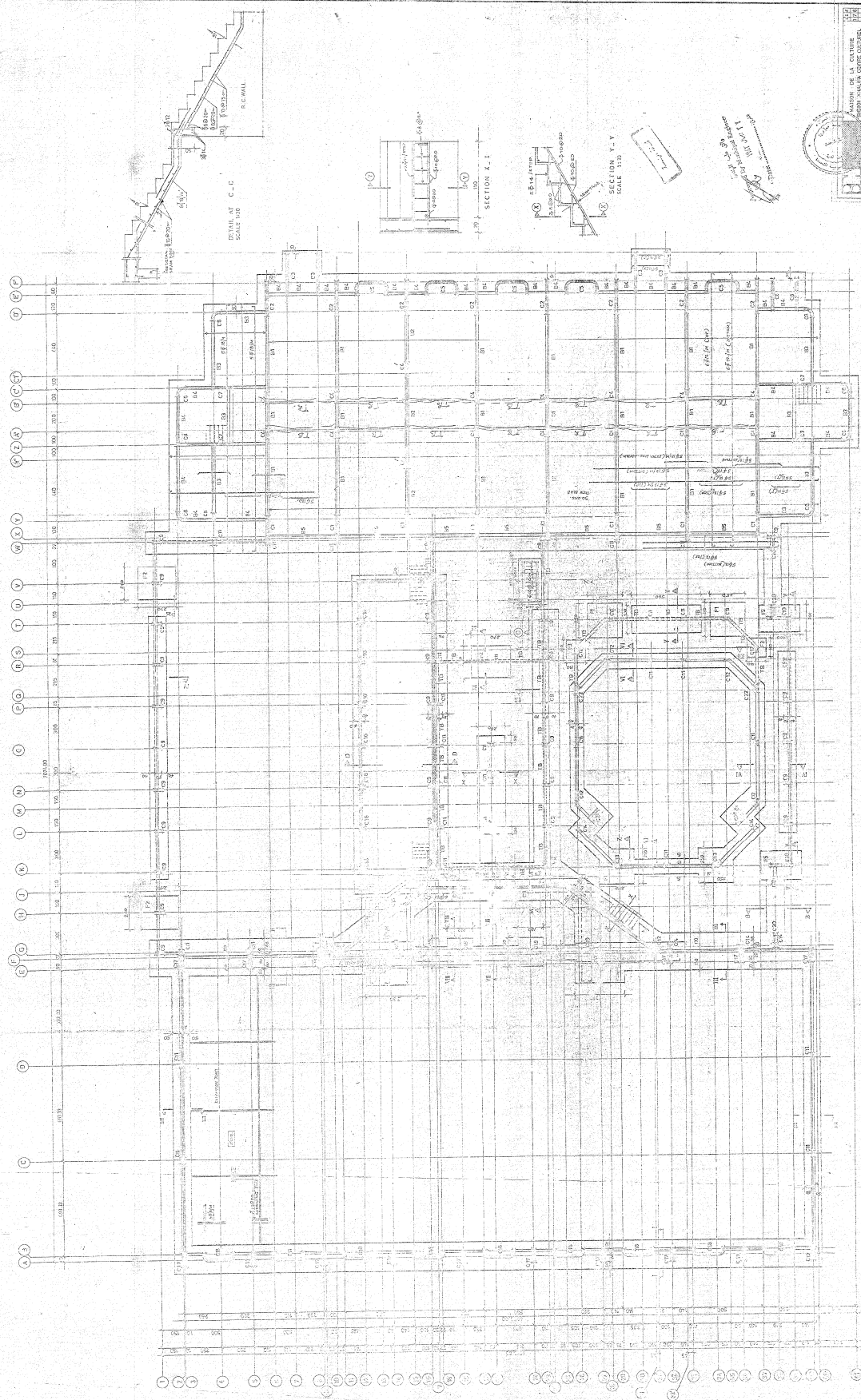
1. COLUMN ENLARGEMENT TYPICAL ELEVATION  
N.T.S

2. COLUMN ENLARGEMENT TYPICAL ELEVATION  
SHOWING TIES AND HOOPS SPACING DETAIL  
N.T.S

3. STARTER BAR PASS THROUGH  
SLAB OR BEAM TYPICAL DETAIL  
N.T.S

## **Appendix B**

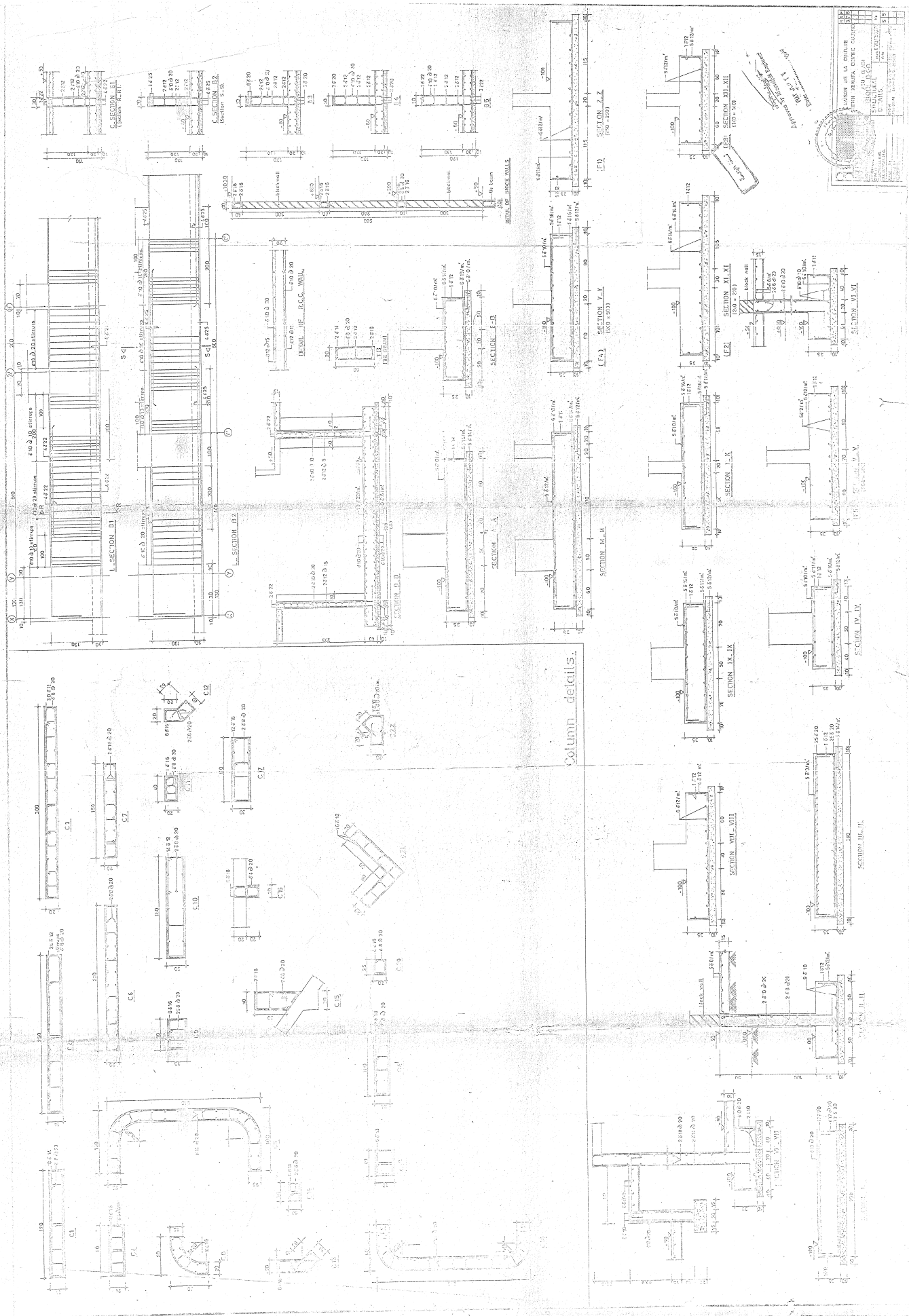
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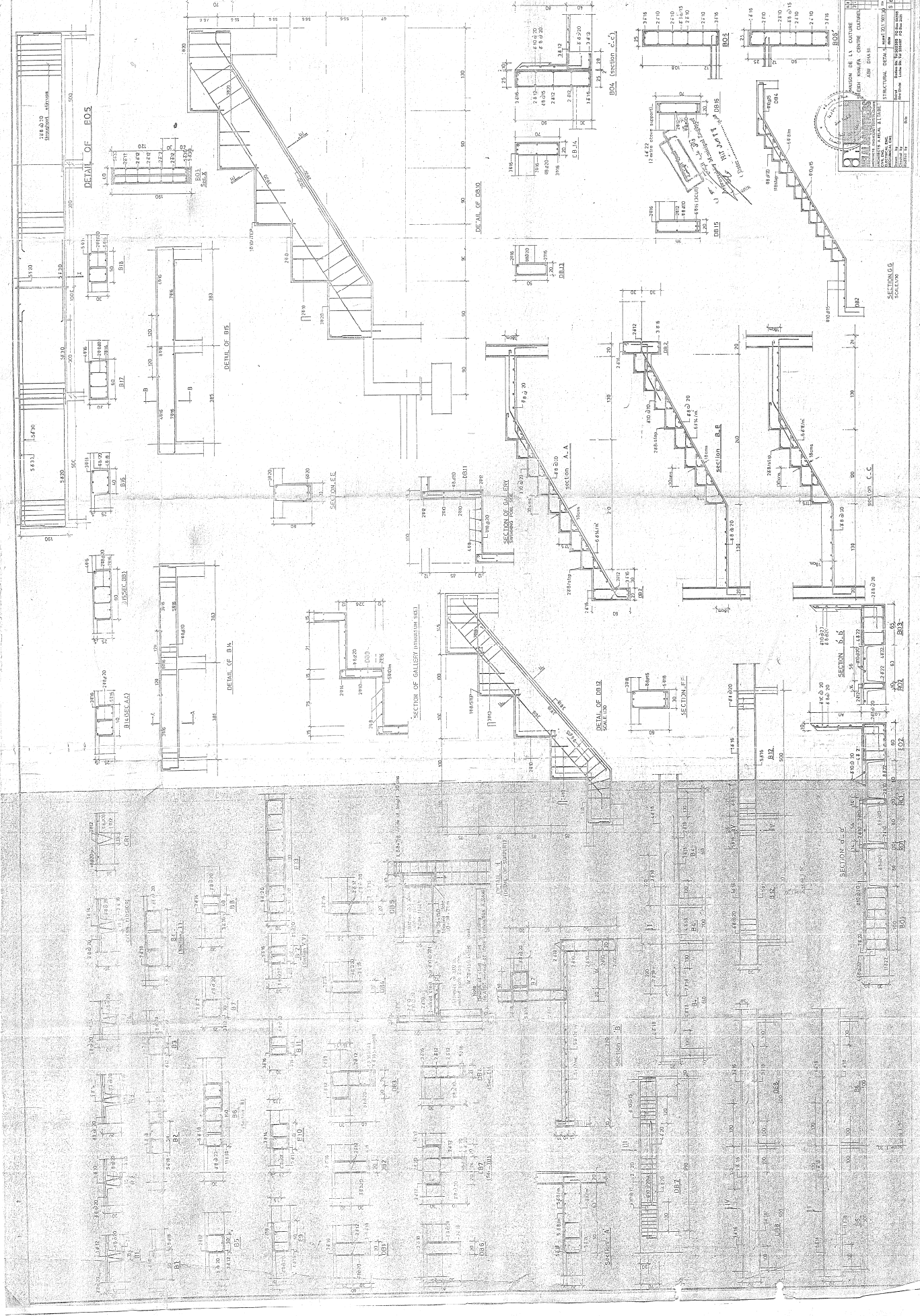












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SECTION G-G

SECTION C-C

SECTION B-B

SECTION A-A

SECTION F-F

SECTION E-E

SECTION D-D

SECTION H-H

SECTION I-I

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SECTION N-N

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SECTION P-P

SECTION Q-Q

SECTION R-R

SECTION S-S

SECTION T-T

SECTION U-U

SECTION V-V

SECTION W-W

SECTION X-X

SECTION Y-Y

SECTION Z-Z

## **Appendix C**

# **SHEAR WALL ANALYSIS FOR GYMNASIUM**



TABLE: Shear Wall Pier Design Summary-ACI 318-14

TABLE: Shear Wall Detail Design Summary - ACI 318-14																											
Story	Pier Label	Station	Design Type	Edge Rebar	End Rebar	Rebar Spacing	D/C Ratio	Pier Leg	Leg X1	Leg Y1	Leg X2	Leg Y2	Length	Thickness	Shear Rebar	Compressive Stress Left	Compressive Stress Right	Compressive Stress Limit Left	Compressive Stress Limit Right	C Depth Left	C Limit Left	C Depth Right	C Limit Right	Boundary Zone Left	Boundary Zone Right	Warnings	Errors
MID	P4	Top	Uniform	12	12	150	0.434 Top Leg 1	-12389.9	11050	12389.9	34590	23300	200	500	0.21	3.88	3.88	3.88	3.88	27.8	67	10			No Message	No Message	
	P4	Bottom	Uniform	12	12	150	0.434 Top Leg 2	12389.9	34590	12699.6	39614.7	501.5	200	500	1.83	3.88	3.88	3.88	3.88	28.5	67		13.9	No Message	No Message		
	MID	P4	Bottom	Uniform	12	12	150	0.465 Bottom Leg 1	-12389.9	11050	12389.9	34590	23300	200	500	0.4	3.88	3.88	3.88	3.88	26.5	67		26.5	No Message	No Message	
	MID	P4	Bottom	Uniform	12	12	150	0.465 Bottom Leg 3	12389.9	34016.8	12699.6	39614.7	501.5	200	500	3.44	3.88	3.88	3.88	3.88	11.2	67		11.2	No Message	No Message	
ROOF	P6	Top	Uniform	12	12	150	0.508 Top Leg 1	-15050.8	14941.7	15019.1	15099.1	1292.9	200	500	4.07	3.88	3.88	3.88	3.88	248.6	206.1	256.6	206.1	124.8	No Message	No Message	
	P6	Bottom	Uniform	12	12	150	0.587 Bottom Leg 1	15050.8	14941.7	15019.1	15099.1	1292.9	200	500	6.35	3.88	3.88	3.88	3.88	258.6	206.1	256.6	206.1	130.3	No Message	No Message	
	MID	P6	Bottom	Uniform	12	12	150	0.46 Top Leg 1	-15050.8	14941.7	15019.1	15099.1	1292.9	200	500	4.38	3.88	3.88	3.88	3.88	283.4	285.1	283.4	285.1	155.1	No Message	No Message
	MID	P6	Bottom	Uniform	12	12	150	0.71 Bottom Leg 1	15050.8	14941.7	15019.1	15099.1	1292.9	200	500	7.21	3.88	3.88	3.88	3.88	293.6	285.1	290.6	285.1	162.3	No Message	No Message
ROOF	P7	Top	Uniform	12	12	150	0.656 Bottom Leg 1	-15010.1	30514.7	15930	29280.5	1539.3	200	500	3.5	3.88	3.88	3.88	3.88	281.3	283.3	283.3	342.1	141.7	No Message	No Message	
	P7	Bottom	Uniform	12	12	150	0.656 Bottom Leg 1	15011.2	30515.6	15929.3	29280	1539.3	200	500	4.18	3.88	3.88	3.88	3.88	294.1	284.1	294.1	342.1	147.1	No Message	No Message	
	MID	P7	Bottom	Uniform	12	12	150	0.889 Bottom Leg 1	-13010.1	30514.7	15930	29280.5	1539.3	200	500	4.96	3.88	3.88	3.88	3.88	319.7	342.1	319.7	342.1	165.7	No Message	No Message
	MID	P7	Bottom	Uniform	12	12	150	0.889 Bottom Leg 1	13010.1	30514.7	15930	29280.5	1539.3	200	500	7.73	3.88	3.88	3.88	3.88	327.9	342.1	327.9	342.1	174	No Message	No Message
ROOF	P8	Top	Uniform	16	16	150	0.019 Top Leg 1	-9591.9	43715.5	-9597.4	43746.3	138.7	200	500	0.000000	3.88	3.88	3.88	3.88	0.000000	3.88	3.88	3.88	1.7	No Message	No Message	
	P8	Bottom	Uniform	16	16	150	0.019 Top Leg 3	-9593.3	43804.2	-9441.9	43715.5	168.1	200	500	0.92	3.88	3.88	3.88	3.88	0.92	3.88	3.88	3.88	14.4	No Message	No Message	
	ROOF	P8	Top	Uniform	16	16	150	0.019 Top Leg 4	-9599.1	43463.3	-9593.3	43804.2	146.7	200	500	0.15	3.88	3.88	3.88	3.88	0.15	3.88	3.88	3.88	36.2	No Message	No Message
	ROOF	P8	Bottom	Uniform	16	16	150	0.019 Top Leg 5	-9600	41850	-9599.1	43463.3	161.33	200	500	0.02	3.88	3.88	3.88	3.88	0.02	3.88	3.88	3.88	1.1	No Message	No Message
ROOF	P8	Top	Uniform	16	16	150	0.019 Top Leg 6	-9600	41850	-9607.8	41712.6	141.1	200	500	0.84	3.88	3.88	3.88	3.88	0.86	3.88	3.88	3.88	0.03	No Message	No Message	
	P8	Bottom	Uniform	16	16	150	0.019 Top Leg 7	-9657.8	41712.6	-9495	41592.7	183.9	200	500	0.55	3.88	3.88	3.88	3.88	0.65	3.88	3.88	3.88	0.03	No Message	No Message	
	ROOF	P8	Top	Uniform	16	16	150	0.019 Top Leg 8	-9495	41592.7	-9300	41590	161.8	200	500	2.13	3.88	3.88	3.88	3.88	2.26	3.88	3.88	3.88	0.85	No Message	No Message
	ROOF	P8	Bottom	Uniform	16	16	150	0.019 Top Leg 9	-9300	41590	-9100	41580	200	200	500	1.18	3.88	3.88	3.88	3.88	2.3	3.88	3.88	3.88	1.18	No Message	No Message
ROOF	P8	Top	Uniform	16	16	150	0.019 Top Leg 10	-9377.1	36593.1	-9100	36590	277.1	200.3	500	0.82	3.88	3.88	3.88	3.88	1.37	3.88	3.88	3.88	1.37	No Message	No Message	
	P8	Bottom	Uniform	16	16	150	0.019 Top Leg 11	-9456	36592.7	-9377.1	36590.1	85.7	200	500	1.59	3.88	3.88	3.88	3.88	1.69	3.88	3.88	3.88	1.69	No Message	No Message	
	ROOF	P8	Top	Uniform	16	16	150	0.019 Top Leg 12	-9514.3	36638.7	-9456	36592.7	74.2	200	500	0.74	3.88	3.88	3.88	3.88	0.82	3.88	3.88	3.88	0.74	No Message	No Message
	ROOF	P8	Bottom	Uniform	16	16	150	0.019 Top Leg 13	-9567.8	36712.6	-9514.3	36638.7	91.2	200	500	0.49	3.88	3.88	3.88	3.88	0.54	3.88	3.88	3.88	0.54	No Message	No Message
ROOF	P8	Top	Uniform	16	16	150	0.019 Top Leg 14	-9592.5	36780.4	-9592.5	36780.4	72.2	200	500	1.41	3.88	3.88	3.88	3.88	1.43	3.88	3.88	3.88	1.43	No Message	No Message	
	P8	Bottom	Uniform	16	16	150	0.019 Top Leg 15	-9599.1	34463.3	-9592.5	36780.4	1674	201.1	502.76	0.004096	3.88	3.88	3.88	3.88	0.01	3.88	3.88	3.88	0.01	No Message	No Message	
	ROOF	P8	Top	Uniform	16	16	150	0.019 Top Leg 16	-9599.1	34463.3	-9589	33832.3	69.7	200	500	1.35	3.88	3.88	3.88	3.88	1.35	3.88	3.88	3.88	1.35	No Message	No Message
	ROOF	P8	Bottom	Uniform	16	16	150	0.019 Top Leg 17	-9589	33832.3	-9583	33804.2	78.2	200	500	1.14	3.88	3.88	3.88	3.88	1.16	3.88	3.88	3.88	1.16	No Message	No Message
ROOF	P8	Top	Uniform	16	16	150	0.019 Top Leg 18	-9583	33804.2	-9511.9	33863.7	75.5	200	500	1.28	3.88	3.88	3.88	3.88	1.28	3.88	3.88	3.88	1.28	No Message	No Message	
	P8	Bottom	Uniform	16	16	150	0.019 Top Leg 19	-9411.9	33863.7	-9441.9	33715.5	87.1	200	500	1.08	3.88	3.88	3.88	3.88	1.08	3.88	3.88	3.88	1.08	No Message	No Message	
	ROOF	P8	Top	Uniform	16	16	150	0.019 Top Leg 20	-9441.9	33715.5	-9399.5	33741.8	77	200	500	0.91	3.88	3.88	3.88	3.88	1.02	3.88	3.88	3.88	1.02	No Message	No Message
	ROOF	P8	Bottom	Uniform	16	16	150	0.019 Top Leg 21	-9399.5	33741.8	-9100	33790	268.6	201	502.56	1.91	3.88	3.88	3.88	3.88	1.91	3.88	3.88	3.88	1.91	No Message	No Message
ROOF	P8	Top	Uniform	16	16	150	0.019 Top Leg 22	-9377.1	31593.1	-9100	31590	277.1	200.4	500	0.78	3.88	3.88	3.88	3.88	1.32	3.88	3.88	3.88	1.32	No Message	No Message	
	P8	Bottom	Uniform	16	16	150	0.019 Top Leg 23	-9456	31592.7	-9377.1	31590.1	85.7	200	500	1.62	3.88	3.88	3.88	3.88	1.62	3.88	3.88	3.88	1.7	No Message	No Message	
	ROOF	P8	Top	Uniform	16	16	150	0.019 Top Leg 24	-9517.7	31636.6	-9496	31592.7	70.9	200	500	0.78	3.88	3.88	3.88	3.88	0.87	3.88	3.88	3.88	0.87	No Message	No Message
	ROOF	P8	Bottom	Uniform	16	16	150	0.019 Top Leg 25	-9567.8	31712.6	-9511.7	31636.6	94.5	200	500	0.51	3.88	3.88	3.88	3.88	0.57	3.88	3.88	3.88	0.57	No Message	No Message
ROOF	P8	Top	Uniform	16	16	150	0.019 Top Leg 26	-9592.5	31780.4	-9567.8	31712.6	72.2	200	500	0.54	3.88	3.88	3.88	3.88	0.56	3.88	3.88	3.88	0.56	No Message	No Message	
	P8	Bottom	Uniform	16	16	150	0.019 Top Leg 27	-9599.1	33463.3	-9592.5	31780.4	1674	201.1	502.76	0.004157	3.88	3.88	3.88	3.88	0.01	3.88	3.88	3.88	0.01	No Message	No Message	
	ROOF	P8	Top	Uniform	16	16	150	0.019 Top Leg 28	-9599.1	33463.3	-9590.3	33929.3	66.6	200	500	0.67	3.88	3.88	3.88	3.88	0.68	3.88	3.88	3.88	0.68	No Message	No Message
	ROOF	P8	Bottom	Uniform	16	16	150	0.019 Top Leg 29	-9590.3	33929.3	-9598.3	33904.2	81.4	200	500	0.69	3.88	3.88	3.88	3.88	0.69	3.88	3.88	3.88	0.69	No Message	No Message
ROOF	P8	Top	Uniform	16	16	150	0.019 Top Leg 30	-9511.2	33904.2	-9441.9	33715.5	77.3	200	500	0.67	3.88	3.88	3.88	3.88	0.73	3.88	3.88	3.88	0.73	No Message	No Message	
	P8	Bottom	Uniform	16	16	150	0.019 Top Leg 31	-9411.2	33865.4	-9341.94																	

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150	0.073 Bottom Leg 53	-9558.3	23529.3	-9558.3	23604.2	81.4	200	500	0.92	2.35	3.88	No Message	No Message	No Message	No Message	No Message	No Message	No Message	No Message	No Message	No Message	No Message	No Message	No Message	No Message	No Message	No Message	No Message	No Message	No Message	No Message	No Message	No Message	No Message	No Message	No Message	No Message	No Message	No Message	No Message	No Message	No Message	No Message	No Message	No Message	No Message	No Message	No Message	No Message	No Message	No Message	No Message	No Message	No Message	No Message	No Message	No Message	No Message	No Message	No Message	No Message	No Message	No Message	No Message	No Message	No Message	No Message	No Message	No Message	No Message	No Message	No Message	No Message	No Message	No Message	No Message	No Message	No Message	No Message	No Message	No Message	No Message	No Message	No Message	No Message	No Message	No Message	No Message	No Message	No Message	No Message	No Message	No Message	No Message	No 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16	P8	Bottom	Uniform	16	16	160	0.143 Bottom Leg 17	-9589	385323	-95893	389042	76.2	200	500	3.94	4.45	3.88	7.6	17.4	8.2	17.4	3.8	3.8	3.1 No Message	No Message	No Message
16	P8	Bottom	Uniform	16	16	160	0.143 Bottom Leg 18	-95893	389042	-95119	389637	75.5	200	500	3.88	3.88	3.88	7.2	16.8	7.6	16.8	3.8	3.8	3.8 No Message	No Message	No Message
16	P8	Bottom	Uniform	16	16	160	0.143 Bottom Leg 19	-95119	389637	-94419	387155	87.1	200	500	3.88	3.88	3.88	7.6	19.3	7.8	19.3	3.8	3.8	3.9 No Message	No Message	No Message
16	P8	Bottom	Uniform	16	16	160	0.143 Bottom Leg 20	-94419	387155	-93895	387148	77	200	500	3.88	3.88	3.88	7.6	17.1	7.6	17.1	3.8	3.8	3.8 No Message	No Message	No Message
16	P8	Bottom	Uniform	16	16	160	0.143 Bottom Leg 21	-93895	387148	-93000	387590	268.6	200	500	502.56	3.88	3.88	3.88	32.3	59.7	31.9	59.7	16.2	15.9 No Message	No Message	No Message
16	P8	Bottom	Uniform	16	16	160	0.143 Bottom Leg 22	-93000	387590	-91000	381590	277.1	200.4	500.97	3.88	3.88	3.88	30.9	61.6	30.4	61.6	3.88	3.88	3.7 No Message	No Message	No Message
16	P8	Bottom	Uniform	16	16	160	0.143 Bottom Leg 23	-9496	319527	-93771	315591	85.7	200	500	3.88	3.88	3.88	7.4	19.1	7.5	19.1	3.7	3.7	4.2 No Message	No Message	No Message
16	P8	Bottom	Uniform	16	16	160	0.143 Bottom Leg 24	-93771	315591	-91000	381590	85.7	200	500	3.88	3.88	3.88	7.4	19.1	7.5	19.1	3.7	3.7	4.2 No Message	No Message	No Message
16	P8	Bottom	Uniform	16	16	160	0.143 Bottom Leg 25	-91000	381590	-90000	379590	85.7	200	500	3.88	3.88	3.88	7.4	19.1	7.5	19.1	3.7	3.7	4.2 No Message	No Message	No Message
16	P8	Bottom	Uniform	16	16	160	0.143 Bottom Leg 26	-90000	379590	-89000	378590	85.7	200	500	3.88	3.88	3.88	7.4	19.1	7.5	19.1	3.7	3.7	4.2 No Message	No Message	No Message
16	P8	Bottom	Uniform	16	16	160	0.143 Bottom Leg 27	-89000	378590	-88000	377590	85.7	200	500	3.88	3.88	3.88	7.4	19.1	7.5	19.1	3.7	3.7	4.2 No Message	No Message	No Message
16	P8	Bottom	Uniform	16	16	160	0.143 Bottom Leg 28	-88000	377590	-87000	376590	85.7	200	500	3.88	3.88	3.88	7.4	19.1	7.5	19.1	3.7	3.7	4.2 No Message	No Message	No Message
16	P8	Bottom	Uniform	16	16	160	0.143 Bottom Leg 29	-87000	376590	-86000	375590	85.7	200	500	3.88	3.88	3.88	7.4	19.1	7.5	19.1	3.7	3.7	4.2 No Message	No Message	No Message
16	P8	Bottom	Uniform	16	16	160	0.143 Bottom Leg 30	-86000	375590	-85000	374590	85.7	200	500	3.88	3.88	3.88	7.4	19.1	7.5	19.1	3.7	3.7	4.2 No Message	No Message	No Message
16	P8	Bottom	Uniform	16	16	160	0.143 Bottom Leg 31	-85000	374590	-84000	373590	85.7	200	500	3.88	3.88	3.88	7.4	19.1	7.5	19.1	3.7	3.7	4.2 No Message	No Message	No Message
16	P8	Bottom	Uniform	16	16	160	0.143 Bottom Leg 32	-84000	373590	-83000	372590	85.7	200	500	3.88	3.88	3.88	7.4	19.1	7.5	19.1	3.7	3.7	4.2 No Message	No Message	No Message
16	P8	Bottom	Uniform	16	16	160	0.143 Bottom Leg 33	-83000	372590	-82000	371590	85.7	200	500	3.88	3.88	3.88	7.4	19.1	7.5	19.1	3.7	3.7	4.2 No Message	No Message	No Message
16	P8	Bottom	Uniform	16	16	160	0.143 Bottom Leg 34	-82000	371590	-81000	370590	85.7	200	500	3.88	3.88	3.88	7.4	19.1	7.5	19.1	3.7	3.7	4.2 No Message	No Message	No Message
16	P8	Bottom	Uniform	16	16	160	0.143 Bottom Leg 35	-81000	370590	-80000	369590	85.7	200	500	3.88	3.88	3.88	7.4	19.1	7.5	19.1	3.7	3.7	4.2 No Message	No Message	No Message
16	P8	Bottom	Uniform	16	16	160	0.143 Bottom Leg 36	-80000	369590	-79000	368590	85.7	200	500	3.88	3.88	3.88	7.4	19.1	7.5	19.1	3.7	3.7	4.2 No Message	No Message	No Message
16	P8	Bottom	Uniform	16	16	160	0.143 Bottom Leg 37	-79000	368590	-78000	367590	85.7	200	500	3.88	3.88	3.88	7.4	19.1	7.5	19.1	3.7	3.7	4.2 No Message	No Message	No Message
16	P8	Bottom	Uniform	16	16	160	0.143 Bottom Leg 38	-78000	367590	-77000	366590	85.7	200	500	3.88	3.88	3.88	7.4	19.1	7.5	19.1	3.7	3.7	4.2 No Message	No Message	No Message
16	P8	Bottom	Uniform	16	16	160	0.143 Bottom Leg 39	-77000	366590	-76000	365590	85.7	200	500	3.88	3.88	3.88	7.4	19.1	7.5	19.1	3.7	3.7	4.2 No Message	No Message	No Message
16	P8	Bottom	Uniform	16	16	160	0.143 Bottom Leg 40	-76000	365590	-75000	364590	85.7	200	500	3.88	3.88	3.88	7.4	19.1	7.5	19.1	3.7	3.7	4.2 No Message	No Message	No Message
16	P8	Bottom	Uniform	16	16	160	0.143 Bottom Leg 41	-75000	364590	-74000	363590	85.7	200	500	3.88	3.88	3.88	7.4	19.1	7.5	19.1	3.7	3.7	4.2 No Message	No Message	No Message
16	P8	Bottom	Uniform	16	16	160	0.143 Bottom Leg 42	-74000	363590	-73000	362590	85.7	200	500	3.88	3.88	3.88	7.4	19.1	7.5	19.1	3.7	3.7	4.2 No Message	No Message	No Message
16	P8	Bottom	Uniform	16	16	160	0.143 Bottom Leg 43	-73000	362590	-72000	361590	85.7	200	500	3.88	3.88	3.88	7.4	19.1	7.5	19.1	3.7	3.7	4.2 No Message	No Message	No Message
16	P8	Bottom	Uniform	16	16	160	0.143 Bottom Leg 44	-72000	361590	-71000	360590	85.7	200	500	3.88	3.88	3.88	7.4	19.1	7.5	19.1	3.7	3.7	4.2 No Message	No Message	No Message
16	P8	Bottom	Uniform	16	16	160	0.143 Bottom Leg 45	-71000	360590	-70000	359590	85.7	200	500	3.88	3.88	3.88	7.4	19.1	7.5	19.1	3.7	3.7	4.2 No Message	No Message	No Message
16	P8	Bottom	Uniform	16	16	160	0.143 Bottom Leg 46	-70000	359590	-69000	358590	85.7	200	500	3.88	3.88	3.88	7.4	19.1	7.5	19.1	3.7	3.7	4.2 No Message	No Message	No Message
16	P8	Bottom	Uniform	16	16	160	0.143 Bottom Leg 47	-69000	358590	-68000	357590	85.7	200	500	3.88	3.88	3.88	7.4	19.1	7.5	19.1	3.7	3.7	4.2 No Message	No Message	No Message
16	P8	Bottom	Uniform	16	16	160	0.143 Bottom Leg 48	-68000	357590	-67000	356590	85.7	200	500	3.88	3.88	3.88	7.4	19.1	7.5	19.1	3.7	3.7	4.2 No Message	No Message	No Message
16	P8	Bottom	Uniform	16	16	160	0.143 Bottom Leg 49	-67000	356590	-66000	355590	85.7	200	500	3.88	3.88	3.88	7.4	19.1	7.5	19.1	3.7	3.7	4.2 No Message	No Message	No Message
16	P8	Bottom	Uniform	16	16	160	0.143 Bottom Leg 50	-66000	355590	-65000	354590	85.7	200	500	3.88	3.88	3.88	7.4	19.1	7.5	19.1	3.7	3.7	4.2 No Message	No Message	No Message
16	P8	Bottom	Uniform	16	16	160	0.143 Bottom Leg 51	-65000	354590	-64000	353590	85.7	200	500	3.88	3.88	3.88	7.4	19.1	7.5	19.1	3.7	3.7	4.2 No Message	No Message	No Message
16	P8	Bottom	Uniform	16	16	160	0.143 Bottom Leg 52	-64000	353590	-63000	352590	85.7	200	500	3.88	3.88	3.88	7.4	19.1	7.5	19.1	3.7	3.7	4.2 No Message	No Message	No Message
16	P8	Bottom	Uniform	16	16	160	0.143 Bottom Leg 53	-63000	352590	-62000	351590	85.7	200	500	3.88	3.88	3.88	7.4	19.1	7.5	19.1	3.7	3.7	4.2 No Message	No Message	No Message
16	P8	Bottom	Uniform	16	16	160	0.143 Bottom Leg 54	-62000	351590	-61000	350590	85.7	200	500	3.88	3.88	3.88	7.4	19.1	7.5	19.1	3.7	3.7	4.2 No Message	No Message	No Message
16	P8	Bottom	Uniform	16	16	160	0.143 Bottom Leg 55	-61000	350590	-60000	349590	85.7	200	500	3.88	3.88	3.88	7.4	19.1	7.5	19.1	3.7	3.7	4.2 No Message	No Message	No Message
16	P8	Bottom	Uniform	16	16	160	0.143 Bottom Leg 56	-60000	349590	-59000	348590	85.7	200	500	3.88	3.88	3.88	7.4	19.1	7.5	19.1	3.7	3.7	4.2 No Message	No Message	No Message
16	P8	Bottom	Uniform	16	16	160	0.143 Bottom Leg 57	-59000	348590	-58000	347590	85.7	200	500	3.88	3.88	3.88	7.4	19.1	7.5	19.1	3.7	3.7	4.2 No Message	No Message	No Message
16	P8	Bottom	Uniform	16	16	160	0.143 Bottom Leg 58	-58000	347590	-57000	346590	85.7	200	500	3.88	3.88	3.88	7.4	19.1	7.5	19.1	3.7	3.7	4.2 No Message	No Message	No Message
16	P8	Bottom	Uniform	16	16	160	0.143 Bottom Leg 59	-57000	346590	-56000	345590	85.7	200	500	3.88	3.88	3.88	7.4	19.1	7.5	19.1	3.7	3.7	4.2 No Message	No Message	No Message
16	P8	Bottom	Uniform	16	16	160	0.143 Bottom Leg 60	-56000	345590	-55000	344590	85.7	200	500	3.88	3.88	3.88	7.4	19.1	7.5	19.1	3.7	3.7	4.2 No Message	No Message	No Message
16	P8	Bottom	Uniform	16	16	160	0.143 Bottom Leg 61	-55000	344590	-54000	343590	85.7	200	500	3.88	3.88	3.88	7.4	19.1	7.5	19.1	3.7	3.7	4.2 No Message	No Message	No Message
16	P8	Bottom	Uniform	16	16	160	0.143 Bottom Leg 62	-54000	343590	-53000	342590	85.7	200	500	3.88	3.88	3.88	7.4	19.1	7.5	19.1	3.7	3.7	4.2 No Message	No Message	No Message
16	P8	Bottom	Uniform	16	16	160	0.143 Bottom Leg 63	-53000	342590	-52000	341590	85.7	200	500	3.88	3.88	3.88	7.4	19.1	7.5	19.1	3.7	3.7	4.2 No Message	No Message	No Message
16	P8	Bottom	Uniform	16	16	160	0.143 Bottom Leg 64	-52000	341590	-51000	340590	85.7	200	500	3.88	3.88	3.88	7.4	19.1	7.5	19.1	3.7	3.7	4.2 No Message	No Message	No Message
16	P8	Bottom	Uniform	16	16	160	0.143 Bottom Leg 65	-51000	340590	-50000	339590	85.7	200	500	3.88	3.88	3.88	7.4	19.1	7.5	19.1	3.7	3.7	4.2 No Message	No Message	No Message
16	P8	Bottom	Uniform	16	16	160	0.143 Bottom Leg 66	-50000	339590	-49000	338590	85.7	200	500	3.88	3.88	3.88	7.4	19.1	7.5	19.1	3.7	3.7	4.2 No Message	No Message	No Message
16	P8	Bottom	Uniform	16	16	160	0.143 Bottom Leg 67	-49000	338590	-48000	337590	85.7	200	500	3.88	3.88	3.88	7.4	19.1	7.5	19.1	3.7	3.7	4.2 No Message	No Message	No Message
16	P8	Bottom	Uniform	16	16	160	0.143 Bottom Leg 68	-48000	337590	-47000	336590	85.7	200	500	3.88	3.88	3.88	7.4	19.1	7.5	19.1	3.7	3.7	4.2 No Message	No Message	No Message
16	P8	Bottom	Uniform	16	16	160	0.143 Bottom Leg 69	-47000	336590	-46000	335590	85.7	200	500	3.88	3.88	3.88	7.4	19.1	7.5	19.1	3.7	3.7	4.2 No Message	No Message	No Message
16	P8	Bottom	Uniform	16	16	160	0.143 Bottom Leg 70	-46000</																		

[illegible]

## **Appendix D**

# **SHEAR WALL ANALYSIS AUDITORIUM**





## **Appendix E**

# **SHEAR WALL ANALYSIS SWIMMING**

TABLE: Shear Wall Per Design Summary - AC308-14

Story	Pier Label	Station	Design Type	Edge Rebar	End Rebar	Rebar Spacing	Required Reinf. Percentage	Current Reinf. Percentage	Pier Leg	Leg X1	Leg Y1	Leg X2	Leg Y2	Length	Thickness	Shear Rebar	Compressive Stress Left	Compressive Stress Right	Compressive Stress Limit Left	Compressive Stress Limit Right	C Depth Left	C Limit Left	C Depth Right	C Limit Right	Boundary Zone Left	Boundary Zone Right	Warnings	Errors
13T	P5	Top	Uniform	10	10	250	0.25	0.25	0.32 Top Leg 1	27169.9	32300	9669.9	32300	2500	200	500	0.09	0.14	4	4	4	mm	mm	mm	mm	No Message	No Message	
	P5	Top	Uniform	10	10	250	0.25	0.25	0.32 Top Leg 2	27169.9	32300	9669.9	32300	2500	200	500	0.12	0.24	4	4	4	mm	mm	mm	mm	No Message	No Message	
	P5	Top	Uniform	10	10	250	0.25	0.25	0.32 Top Leg 3	27169.9	32300	9669.9	32300	2500	200	500	0.09	0.14	4	4	4	mm	mm	mm	mm	No Message	No Message	
	P5	Bottom	Uniform	10	10	250	0.25	0.25	0.32 Bottom Leg 1	27169.9	32300	9669.9	32300	2500	200	500	0.12	0.45	4	4	4	mm	mm	mm	mm	No Message	No Message	
13T	P5	Bottom	Uniform	10	10	250	0.25	0.25	0.32 Bottom Leg 2	27169.9	30799.7	32300	1500.3	200	500	4	4	1.1	4	4	4	4	mm	mm	mm	mm	No Message	No Message
	P5	Bottom	Uniform	10	10	250	0.25	0.25	0.32 Bottom Leg 3	27169.9	30799.7	32300	1500.3	200	500	4	4	0.00238	1.9	4	4	4	mm	mm	mm	mm	No Message	No Message